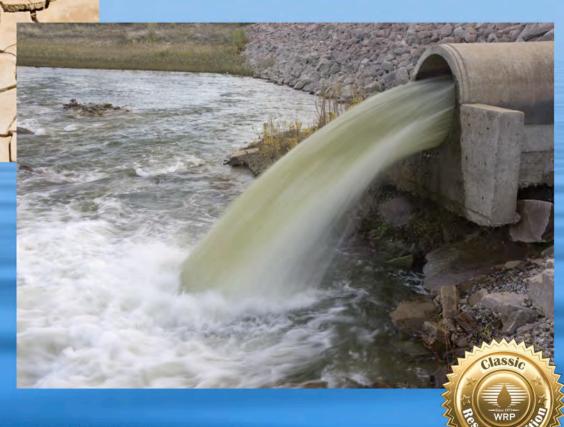
TRANSIENT GROUND WATER HYDRAULICS

by Robert E. Glover





Water Resources Publications, LLC

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Preface

his book has been written to present developments, formulas and methods which will be useful to engineers for making quantitative evaluations of ground water flow. Because, in irrigated country at any rate, transients will be the rule, the main emphasis will be on transient phenomena. The Dupuit-Forchheimer idealization will be the basis for most of these developments. This idealization, which is described in the text, leads to partial differential equations which are nonlinear in form. To be freed from the difficulties imposed by this nonlinearity "the basic differential equations will generally be linearized by neglecting the changes of saturated thickness which accompany transient flow conditions. This procedure will produce formulas that are, to some degree, approximations. However, if these are used skillfully with aquifer properties obtained from field tests and with awareness of their limitations they will yield results as good as can generally be obtained when application has to be made to aquifers where irregularities and nonuniformity are the rule rather than the exception. When this simplification can be used, and this will include the majority of field cases, the computations are freed of the burdensome details which appear when more elaborate treatments are made. It can be pointed out, in addition, that if the criticisms leveled at these useful approximations were to be taken seriously we should find ourselves obligated to discard the great bulk of engineering formulas used so successfully over the past 200 years since a close scrutiny of their bases will reveal short-comings as bad or worse than those outlined above.

Sometimes conditions will require that a closer evaluation be obtained than can be realized by the formulas derived from the linearized equations. For such cases second approximations or formulas derived by special methods will be supplied.

Even though the task of developing working formulas will be adhered to, parallel or alternative procedures' will be described when it appears that useful results will be obtained.

While the treatment of steady state cases will, in general, be confined to those which represent the terminal state to which a transient state being considered converges, some attention will be given to historical developments and to the steady state flow patterns which were obtained by the early workers. It would be impossible to include all of the developments obtained by previous workers in a text of the size of this one but, where material must be omitted, sufficient references will be included to indicate to the user of this text where this other material can be found.

The chapter is made the basis for organization of the material presented herein. A thumb index is provided so that the chapters may be readily located. Figure, formula and table numbers are identified by a chapter number followed by a sequence number. In this fashion Figure 3-2 is the second figure in Chapter 3.

The tables 1 to 15 inclusive have been machine computed and the reproduction has been made directly from the machine readout sheets to insure accuracy. Checks have been made, where needed, to show that the machine program was working as intended.

Data are seldom available which would permit a determination of a probable error and the number of places which would be appropriate for expressing the final result of a computation of ground water movement. In the material presented, the number of places retained has often been chosen to make the development more easily followed or to clarify comparisons. In the final results figures have been retained on the basis of judgment.



Editor's Note to the Third Printing

ransient Ground Water Hydraulics makes quantitative evaluations of ground water flow in regards to irrigation. This book has been cited and referenced in numerous papers, journals, and books continuously for over 30 years. It is still an important addition to any library that is concerned with quantitative ground water flow. WRP is pleased to re-release this book as part of our Classic Resource Edition.

Branka McLaughlin Publisher/Editor

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Chapter 1

Development of ground water use

The knowledge that water could be obtained from the earth and that the water so obtained was generally cool, clear and sweet seems to be very ancient. Wells are mentioned frequently in Genesis indicating that the possibility of obtaining water from the ground was well understood perhaps 3500 years ago. Before modern times, wells appear to have been commonly used to supply domestic and stock water since these important uses could be supplied by muscle power which was then the only means available for raising water from them. In early day America there were many dug wells from which water was lifted by means of a bucket and windlass arrangement. These were followed by hand operated pumps. Windmills were later developed which could operate these pumps by harnessing the force of the winds. Many of these were installed not only at farmsteads but in remote locations to provide water for stock. All of these developments had for their purpose the providing of water for the important domestic and stock water uses. Development of gasoline engines gave a more reliable source of power for farm pumps and eliminated the recurring arduous task of pumping water by hand to fill stock tanks during windless periods. The old "Hit and Miss" farm engine became a common sight on farms and continued in use for many years.

Development of more effective well drilling methods, the use of casings, the extension of power lines through rural areas and the development of improved types of pumps opened up the possibility of pumping for irrigation and other heavy uses such as for municipal water supplies.

These developments went almost unnoticed for some time and then problems began to arise. In the river valleys of the arid west where the practice

of diverting stream flows for irrigation had long been established it came to be realized that pumps could deplete the flow of the stream to the detriment of the surface diverters operations.

Where replenishment of the ground water reservoir comes from infiltration from precipitation, falling water tables were generally observed as pumping for irrigation developed. Whereas something like one acre foot per year would supply the domestic and stock water needs for a farmstead and the infiltration supply could sustain this demand forever, an economically viable irrigation well would need to lift this much, or more, in a day and the possibility of depletion of the resource came to be recognized. If pump irrigation develops in an area and replenishment is from precipitation a sinking of the water table is to be expected. If the comsumptive use is held within the limits imposed by the supply then the changing water table levels represent only a readjustment toward a new stable configuration but if the supply is overencumbered they have a much more serious portent.

Where municipal supplies come from ground water, the increasing demands caused by population increases can threaten a failure of the supply and cause problems of the most serious kind.

To add to the difficulties outlined above a quantitative assessment of ground water flows cannot be made by the simple procedures which suffice for surface water. An operations study for an irrigation system utilizing surface supplies will, for example, make few mathematical demands beyond the ability to add and subtract. To deal with ground water flows in a similar quantitative way will require much more specialized mathematical techniques. Here the hydraulic and geometric properties of the aquifer must be accounted for and partial differential equations of the second order may need to be solved. Fortunately, the results of these

operations can generally be reduced to graphical or tabular form so that the user does not need to deal with the basic mathematical difficulties. It is the purpose of this text to provide such graphs and tables so that the quantitative assessment of ground water flows can be facilitated. It is hoped that these developments will contribute to solutions of some of the serious problems which growing uses of ground water have brought with them.

Occurrence of ground water

Rain and, in some areas, melting snow and ice has supplied over geologic times, percolating waters to build up a body of underground water which exists almost everywhere. Even desert areas receive occasional rains which will contribute deep percolating water to a ground water body. In the more well watered areas of the globe the ground water mound between stream valleys is never completely drained away but, instead, sustains the base flow of the streams so that they continue to flow even though the rains may cease for considerable periods of time.

For those interested in recovering ground water for useful purposes the principal question is not generally the presence of ground water but the presence of sufficient porosity in the formation to sustain a useful flow. For the small quantities of water needed to supply domestic and stock water uses there are vast areas where a well of this capacity can be obtained almost anywhere.

Anything like an exhaustive treatment of productive water bearing formations would be beyond the scope of this work but it will be useful to identify some of the important aquifers of the United States so that some knowledge is acquired concerning the types of formations which may contain water in recoverable amounts (McGuinness).

Coastal sands often yield important supplies of fresh water in spite of being in contact with sea water. The outward flowing fresh water, often replenished from an ample rainfall, holds the saline ocean waters in check (Glover, 1959). The Ghyben-Herzberg rule applies in such cases. This rule holds that for every foot the water table is above sea level there will be 40 feet of fresh water underneath. Extensive aquifers of this type occur along the Atlantic and Gulf coasts.

The Biscayne limestone extends inland from the east coast of Florida. Although it is strong enough to yield drill cores it is nevertheless extremely permeable and is an important source for water supply in the Miami area. Other limestones also yield water in important amounts.

The Dakota sandstone is an extensive and important water bearing formation composed, generally, of a fine sand loosely cemented by iron carbonate. This formation provides an example of a sandstone aquifer.

The Snake Plain aquifer is composed of a series of lava flows. The lavas are extensively cracked presumably due to cooling shrinkage. Permeabilities are surprisingly high. It is an important ground water source in southern and eastern Idaho. The water carrying capabilities of this aquifer are dramatically demonstrated in the "Thousand Springs" area. Here the ground water issues from a cliff in such volumes as to create waterfalls.

The Rathdrum Prairie area in Washington is underlain with an aquifer of exceptional permeability composed of glacial drift.

Many productive aquifers are to be found in alluvial deposits. Perhaps the most extensive of these is the Ogallala formation which extends across parts of Nebraska, Wyoming, Colorado, Kansas, New Mexico and Texas. It is an important source of irrigation water. Alluvial sediments are also widely distributed in river valleys.

Similar to the alluvial deposits are deposits of wind blown sands.

Some of these, as the Navajo sandstone in the Utah, Arizona area have become cemented but in other cases they are represented by the sand dune areas in some of the western states. These sand dune areas possess exceptional infiltration capacities. In some areas infiltration rates are so great that no water erosion forms are to be found since even the heaviest rains produce no runoff. Some of the Nebraska rivers originating in the

Sand Hill area are notable for the uniformity of their flows. This is explainable on the basis that almost none of their flow comes from surface runoff while the bulk of it is base flow coming from ground water.

Crystalline rocks are generally unproductive not because water is absent but largely because permeabilities are so low that no useful flow to a well can be maintained. If the rock is fractured, however, as in a fault zone, then productive wells may sometimes be obtained even in formations of this type. Wells yielding around one gallon per minute can often be obtained in rock if a deep hole is drilled and the rock is fractured by pressures.

Chapter 2

Basic differential equations

The differential equations applicable to the flow of ground water are expressions, in mathematical form, of the budgetary requirement that water volumes must be conserved.

For an element of volume dx dy dz below the water table, for example, there can be no accumulation of water volume and it is then necessary to express the requirement that the net flow of water into the element must be and remain zero.

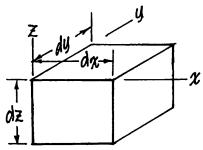


Fig. 2-1 Volume element.

Flows are computed on the basis of Darcy's law which can be expressed in the form

$$q = K i A$$

where q represents the flow through the area A in an aquifer of permeability K under the influence of the gradient i. If h_1 represents the departure of the pressure from a hydrostatic state the flow through the area dy dz in the direction of x will be -K dy dz $\frac{\partial h_1}{\partial x}$ and a similar expression will apply to the flows in the y and z directions. The minus sign is needed because the flow is considered positive in the direction of positive x and if $\frac{\partial h_1}{\partial x}$ is positive the pressure gradient is such as to cause flow in the direction of negative x. The accumulation of flow into the element due to flows in the x direction is $\frac{\partial}{\partial x}$ (-K dy dz $\frac{\partial h_1}{\partial x}$) dx or -K dx dy dz $\frac{\partial^2 h_1}{\partial x^2}$. The condition that there be no accumulation of flow in

the element then takes the form

- K dx dy dz
$$\frac{\partial^2 h_1}{\partial x^2}$$
 - K dx dy dz $\frac{\partial^2 h_1}{\partial y^2}$ - K dx dy dz $\frac{\partial^2 h_1}{\partial z^2}$ = 0

8

or, simply,

$$\frac{\partial^2 h_1}{\partial x^2} + \frac{\partial^2 h_1}{\partial y^2} + \frac{\partial^2 h_1}{\partial z^2} = 0 \tag{2-1}$$

This is known as the Laplace formulation, not because Laplace considered the movement of ground water but because the differential equation obtained is the one which bears his name. Many steady-state solutions have been obtained for it but its use for transient cases has been limited due to the difficulty of dealing with a moving boundary. The formulation is basically sound, however, for both transient and steady state ground water flows. If the moving boundary difficulties could be overcome it would afford a means for overcoming the difficulties caused by nonlinearities which afflict some of the other formulations. Some progress has been made in this direction and more may be expected.

A mathematically advantageous formulation was proposed by Dupuit (1863). He considered an element penetrating the full depth of the aquifer, as shown in the figure below, for flow in the direction of x only

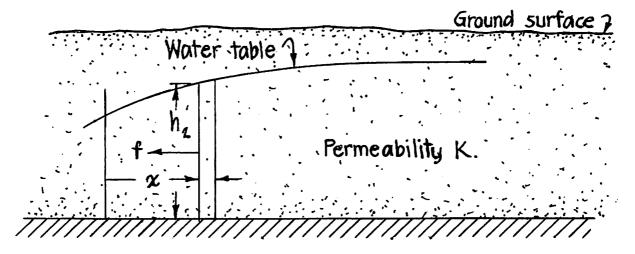


Fig. 2-2 Cross section.

Here the symbol h_2 is used to represent the saturated depth above a level impermeable lower boundary. The flow f through the depth h_2 for a unit width is, if the surface gradient can be considered effective throughout the entire saturated depth, $f = K h_2 \frac{\partial h_2}{\partial x}$. The accumulation of flow in the element during the time dt is $\frac{\partial f}{\partial x} dt$ or $K \frac{\partial}{\partial x} \left(h_2 \frac{\partial h_2}{\partial x}\right) dx dt$. The condition that the accumulation of flow in the element must be consistent with the rate of rise of the water table is

$$K \frac{\partial}{\partial x} (h_2 \frac{\partial h_2}{\partial x}) dx dt = V \frac{\partial h_2}{\partial t} dx dt$$

where V represents the effective voids ratio appropriate for the aquifer.

After simplification the relation becomes:

$$K \frac{\partial}{\partial x} (h_2 \frac{\partial h_2}{\partial x}) = V \frac{\partial h_2}{\partial t}$$

It will be noted at once that this is a differential equation of nonlinear form. If a steady state exists the right hand member will be zero and the formulation will then assume the form:

$$\frac{\partial}{\partial x} \left(h_2 \frac{\partial h_2}{\partial x} \right) = 0$$

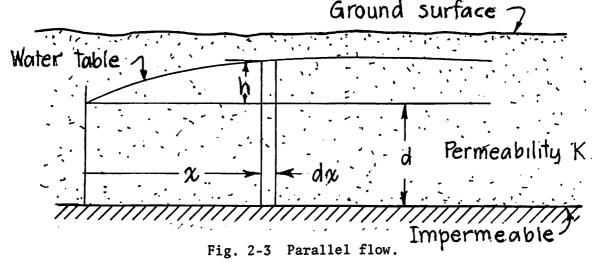
Even though this is nonlinear, solutions are readily obtained. The early workers in the ground water field generally limited their researches to steady state cases.*

Linearization by neglect of small terms

If the transient case be again considered the known intractable nature of nonlinear differential equations will make it desirable to find some way

Boussinesq, 1904 treated the case of ground water flow to streams on a transient basis.

of linearizing them. The simplest way of doing this is to consider small departures h from an initial saturated depth, as shown in the figure below.



By following the reasoning previously used the differential equation for the transient case will take the form:

$$K \frac{\partial}{\partial x} (d + h) \frac{\partial h}{\partial x} = V \frac{\partial h}{\partial t}$$

Linearization can now be obtained, at the expense of approximation, by neglecting h in the term (d+h). This means that the use of any solutions obtained must be restricted to cases where, in fact, h is small compared to d. Experience indicates that this will be a permissible simplification in the great majority of field applications. If the simplification is acceptable the differential equation can be put into the form:

$$\alpha \frac{\partial^2 h}{\partial x^2} = \frac{\partial h}{\partial t}$$
 (2-2)

Where $\alpha = \frac{Kd}{V}$ is the aquifer constant. It is a factor which must be determined by test. Methods of evaluation will be described later in the text. When this simplification is used an important tactical advantage is secured. The differential equation becomes of the type which has been so extensively studied in relation to the conduction of heat in solids. Not only is the differential equation of the same form but the boundary conditions are also

analogous. This means that the wealth of material found in this older discipline becomes immediately available for application in the new field. It will be found that this parallelism extends to the important radially symmetrical cases also. It will appear later that the formulas obtained with this linearization are of simpler form and better adapted to engineering applications than those obtained by any other approach. The many advantages offered by this linearization will support a decision to adopt it as the source of the basic system of formulas presented in this text. In the rare cases, where their limitations hinder their use, other more appropriate solutions will be presented to take their place. When this happens complexities may be expected to appear. In some cases these will take the form of new parameters which must be evaluated and applied. In all cases water quantities will be accounted for without approximation.

The linearization of Werner

In the differential equation

$$K \frac{\partial}{\partial x} (h_2 \frac{\partial h_2}{\partial x}) = V \frac{\partial h_2}{\partial t}$$

The substitution of variable

$$h_2^2 = u$$

is introduced (Werner 1957).

The differential equation is reduced by this means to the form:

$$\frac{K\sqrt{u}}{V} \frac{\partial^2 u}{\partial x^2} = \frac{\partial u}{\partial t}$$

Linearization is now accomplished by replacing $\sqrt{u} = h_2$ by some carefully chosen constant representative value H.

If we now set

$$\alpha_{W} = \frac{KH}{V}$$

The differential equation becomes:

$$\alpha_{W} \frac{\partial^{2} u}{\partial x^{2}} = \frac{\partial u}{\partial t}$$
 (2-3)

Radial symmetry - Laplace type of formulation

Operation of a well can produce a flow of ground water with radial symmetry as shown in the figure below

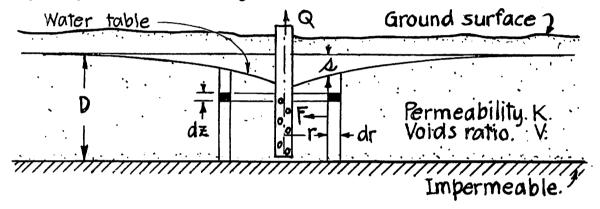


Fig. 2-4 Radial flow.

The flow of ground water into a submerged ring shaped element of volume of the aquifer of cross section dr dz and radius r will be

$$\frac{\partial}{\partial z}(2\pi r K dr \frac{\partial h}{\partial z}) dz + \frac{\partial}{\partial r}(a\pi r K dz \frac{\partial h}{\partial r}) dr = 0$$

In this expression h represents the departure of the pressure from hydrostatic. The continuity condition which prohibits the accumulation of water in the element dictates the requirement that the sum of these two flows must be zero. After the indicated differentiations are performed and the expression is simplified by dividing out common terms it takes the form:

$$\frac{\partial^2 h}{\partial r^2} + \frac{1}{r} \frac{\partial h}{\partial r} + \frac{\partial^2 h}{\partial z^2} = 0 \tag{2-4}$$

This is the Laplace type of formulation where radial symmetry is present.

Radial symmetry - Dupuit-Forchheimer formulation

The flow F through the cylindrical surface of radius r and height (D - s) will be, under the Dupuit-Forchheimer idealization:

$$F = -2\pi r K(D-s) \frac{\partial s}{\partial r}$$

The accumulation of flow in the annulus of dr thickness will be:

$$\frac{\partial \mathbf{F}}{\partial \mathbf{r}} d\mathbf{r} = -2\pi K \frac{\partial}{\partial \mathbf{r}} [\mathbf{r}(D-s) \frac{\partial s}{\partial \mathbf{r}}] d\mathbf{r}$$

If, to avoid nonlinearity, s is neglected as being small compared to D then approximately:

$$\frac{\partial F}{\partial r} dr = -2\pi KD \frac{\partial}{\partial r} (r \frac{\partial s}{\partial r}) dr$$

The condition of continuity is then expressible in the form:

$$\frac{\partial F}{\partial r} dr = -2\pi r V dr \frac{\partial s}{\partial t}$$

or after substitution and rearrangement with:

$$\alpha = \frac{KD}{V}$$

it becomes

$$\alpha \frac{\partial}{\partial r} (r \frac{\partial s}{\partial r}) = r \frac{\partial s}{\partial t}$$

or

$$\alpha(\frac{\partial^2 s}{\partial r^2} + \frac{1}{r} \frac{\partial s}{\partial r}) = \frac{\partial s}{\partial t}$$
 (2-5)

This is the linearized differential equation obtained, for radial symmetry conditions, when s is neglected as being small compared to D.

A similar procedure is described in Chapter XIII of the text by Polubarinova-Kochina.

Comparisons of results obtained by these alternative methods will be found later in the text.

Chapter 2.

Problems

(2-1) For steady state conditions $(\partial s/\partial t) = 0$ and formula 2-5 takes the form:

$$\frac{d^2s}{dr^2} + \frac{1}{r}\frac{ds}{dr} = 0 \qquad \text{or} \qquad \frac{d}{dr}(r \frac{ds}{dr}) = 0$$

derive from this a formula for the draw down s, as a function of r, due to pumping of a well, of radius a, at the rate Q when the water table is maintained at the level D when r = b. Consider s as negligibly small as compared to D.

Answer:

$$s = \frac{Q}{2\pi KD} \log_e(\frac{b}{r})$$

(2-2) Compare this result with that obtained by Dupuit (Rouse and Ince, 1963) and comment on differences or similarities.

(2-3) In the relation
$$F = -2\pi Kr(D-s)\frac{ds}{dr}$$
,

presented in the account of the Dupuit-Forchheimer formulation set F = Q and $h_2 = (D-s)$ to obtain

$$Q = 2\pi Krh \frac{dh_2}{dr}$$

Treat this as a nonlinear differential equation and derive a formula for the variation of saturated depth h_2 with r when the water table is maintained at the depth h_2 = D at r = b.

Answer:

$$D^2 - h_2^2 = \frac{Q}{\pi K} \log_e(\frac{b}{r})$$

(2-4) Compare this result with the second form obtained by Dupuit (Rouse and Ince, 1963) and comment on differences or similarities.

(2-5) By using an idealization of the Dupuit-Forchheimer type, as shown in figure 2-3, derive an expression for the steady state form of the water table where a uniform infiltration rate i is applied to a strip drained by parallel drains spaced a distance L apart. Note that the continuity condition will require that, if h is considered negligibly small compared to d.

$$Kd \frac{dh}{dx} = i(\frac{L}{2} - x)$$

Answer:

$$h = \frac{iL^2}{8Kd} - \frac{i}{2Kd} (\frac{L}{2} - x)^2$$
 At $x = \frac{L}{2}$ $h_m = \frac{iL^2}{8Kd}$

(2-6) With the conditions shown on figure 2-2 and with $f = i(\frac{L}{2} - x)$ express the continuity condition and solve the resulting nonlinear differential equation to obtain the variation of h_2 with respect to x, if $h_2 = d$ when x = 0 and when x = L.

Answer:

$$\frac{h_2^2}{2} - \frac{d^2}{2} = \frac{iL}{2K} (x - \frac{x^2}{L})$$

At $x = \frac{L}{2}$

$$(h_2 - d)_c = d \left[\sqrt{1 + \frac{iL^2}{4Kd^2}} - 1 \right]$$

Approximately, at x = L/2:

$$(h-d)_c = \frac{iL^2}{8Kd}$$
 if $\frac{iL^2}{4Kd^2}$ is small compared to unity.

(2-7) Compare the value obtained from (2-5) for (h+d) with the value obtained from (2-6) for h_2 at the point x = L/2, which is the point midway between drains. (Suggestion: Develop the radical into a series by use of the binomial theorem on the basis that $(iL^2/4Kd^2)$ is small compared to unity).

Answer:

To a first approximation the results of (2-5) and (2-6) agree.

Chapter 3

Well drawing water at the constant rate Q from an aquifer with a free water table

To treat this case it will be sufficient to solve the linearized differential equation:

$$\alpha \left(\frac{\partial^2 s}{\partial r^2} + \frac{1}{r} \frac{\partial s}{\partial r} \right) = \frac{\partial s}{\partial t}$$

Subject to the initial and boundary conditions

$$s = 0$$
 when $t = 0$ for $r > 0$
- $2\pi r KD \frac{\partial s}{\partial r} \rightarrow Q$ as $r \rightarrow 0$

The solution, so obtained, will serve as an effective approximation so long as s is small compared to D.

The Boltzman variable, used in the form:

$$z = \frac{r}{\sqrt{4\alpha t}}$$

may be applied to obtain the desired solution. With:

$$\frac{\partial s}{\partial r} = \frac{\partial s}{\partial z} \sqrt{\frac{1}{4\alpha t}} \qquad \frac{\partial s}{\partial t} = -\frac{\partial s}{\partial z} \frac{2\alpha r}{(4\alpha t)^{3/2}}$$

$$\frac{\partial^2 s}{\partial r^2} = \frac{\partial^2 s}{\partial z^2} \frac{1}{4\alpha t}$$

Substitution and simplification will produce the ordinary differential equation

$$\frac{d^2s}{dz^2} + \left(\frac{1}{z} + 2z\right) \frac{ds}{dz} = 0.$$

Let $p = \frac{ds}{dz}$. Then the equation may be reduced to one of the first order.

$$\frac{dp}{dz} + \left(\frac{1}{z} + 2z\right) p = 0.$$

An integrating factor is:

$$e^{\int (\frac{1}{z} + 2z)dz} = e^{\log_e z + z^2} = ze^{z^2}$$

Then a solution is:

$$pze^{z^2} = C$$

And

$$\frac{ds}{dz} = \frac{Ce^{-z^2}}{z}$$

By integration, if s = 0 when t = 0

$$s = C \int_{\sqrt{4\alpha t}}^{\infty} \frac{e^{-z^2} dz}{z}$$

The flow toward the well is:

$$F = -2\pi KDr \frac{\partial s}{\partial r} = + 2\pi KDCe^{-\frac{r^2}{4\alpha t}}$$

If
$$F o Q$$
 as $r o 0$ $C o (\frac{Q}{2\pi KD})$

Then, finally, the solution sought is; with the variable of integration changed to u to correspond to the usage of Table 1,

$$s = \frac{Q}{2\pi KD} \int_{\left(\mathbf{r}/\sqrt{4\alpha t}\right)}^{\infty} \frac{e^{-u^2} du}{u}$$
 (3-1)

An example to illustrate the use of this formula can be made to serve two purposes. A computation of drawdowns produced by pumping a well can first be made. Then, by considering the computed drawdowns to be observed values, the process of determining aquifer properties from pump test data can be presented. An advantage is conferred by going about the presentation in this way because we will already know what the outcome should be and we will thus be in position to evaluate the effectiveness of the method. The following conditions will be assumed to prevail:

Flow of well Q = 750 gallons per minute

Permeability K = 0.00384 ft/sec D - 66.6 feet

Transmissibility KD = $0.2557 \text{ ft}^2/\text{sec V} = 0.171$

Aquifer constant $\alpha = \frac{KD}{V} = 1.50 \text{ ft}^2/\text{sec}$

In consistent units of feet and seconds:

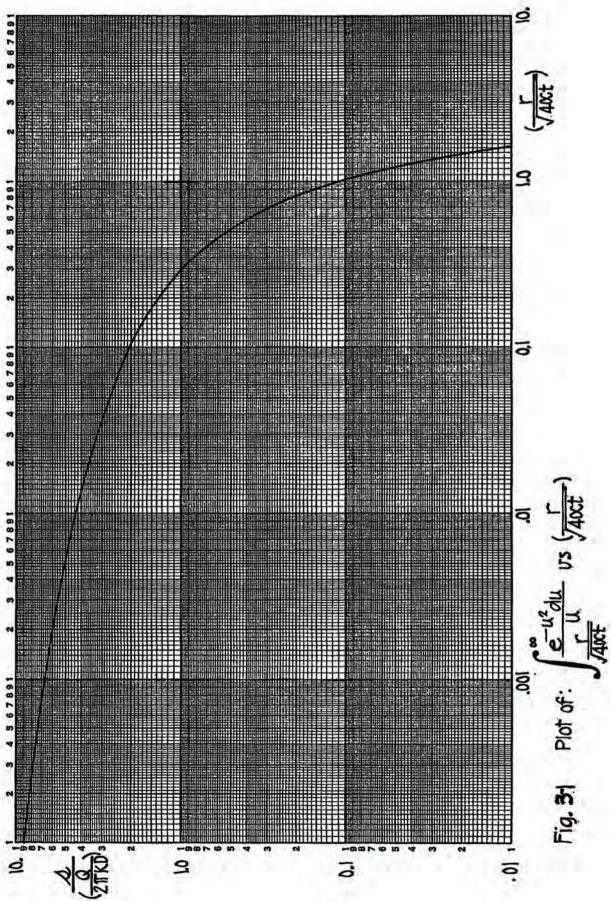
$$Q = 1.6710 \text{ ft}^3/\text{sec}$$
 $(Q/2\pi KD) = 1.0401$

Computations of the drawdowns s for various distances and times are made as shown below. Values of $s/(Q/2\pi KD)$ are obtained from Table 1.

r (feet)						
Time	10	50	100	500	1000	
1 day 86400 sec						
r/√4∝t	0.01389	0.06945	0.1389	0.6945	1.3890	
$s/(Q/2\pi KD)$	3.988	2.381	1.695	0.2909	0.02697	
s (feet)	4.148	2.476	1.763	0.302	0.028	
1 week 604800 sec						
r/√4∝t	0.005250	0.02625	0.05250	0.2625	0.5250	
s/Q/2πKD)	4.9609	3.3518	2.6597	1.0828	0.4846	
s (feet)	5.160	3.486	2.766	1.126	0.504	
1 month 2628000 sec						
r/√4∝t	0.002518	0.01259	0.02518	0.1259	.2518	
$s/(Q/2\pi KD)$	5.6957	4.0871	3.3935	1.7916	1.1217	
s (feet)	5.924	4.251	3.530	1.863	1.167	
4 months 10512000 sec						
r/ 4at	0.001259	0.006296	0.01259	0.06296	0.1259	
$s/(Q/2\pi KD)$	6.3888	4.7793	4.0871	2.4786	1.7916	
s (feet)	6.645	4.971	4.251	2.578	1.863	

Attention may now be turned to the problem of determining aquifer properties from field test data. The computed drawdowns from the above tabulation may now be construed as data obtained from observation wells from which aquifer data are to be derived. The first step is to prepare a plot such as is shown on figure 3-1. To be suitable, the grid must be logarithmic both ways. Such a chart is plotted from data obtained from Table 1.

The test data are plotted on a similar sheet with the observed draw-downs s as ordinates and the quantity (r/\sqrt{t}) as abscissa. The fact that use of the Boltzman variable succeeds in converting the partial differential equation 2-5 to an ordinary differential equation will insure that all of the plotted points will fall on a single line irrespective of



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radius and time so long as these two variables occur in the form (r/\sqrt{t}) . Such a plot is shown in figure 3-2.

The sheet of figure 3-1 is now placed over the plot of figure 3-2 and the two are adjusted, while keeping the axes parallel, until the observed points of figure 3-2 coincide everywhere with the curve of figure 3-1. The position of the cross, at the point 1.0 - 1.0 of figure 3-1 is now marked on figure 3-2.

It may have been noted that, while the parameters of figure 3-1 are pure numbers, the parameters of figure 3-2 are not. Since both scales are logarithmic a shift will represent a multiplication and the missing units can appear in the factor represented by the shift. The following relations are now available:

$$\frac{s}{(Q/2\pi KD)} = 1.00$$
 $s = 1.02$ $\frac{r}{\sqrt{4\alpha t}} = 1.00$ $\frac{r}{\sqrt{t}} = 2.40$ $\frac{Q}{2\pi KD} = 1.04$

From the first of these:

$$\frac{1.02}{\left(\frac{Q}{2\pi KD}\right)} = 0.981 \qquad KD = \frac{Q(0.981)}{(1.02)(2\pi)} = \frac{(1.671)(0.981)}{6.409} = 0.2588 \text{ ft}^2/\text{sec.}$$

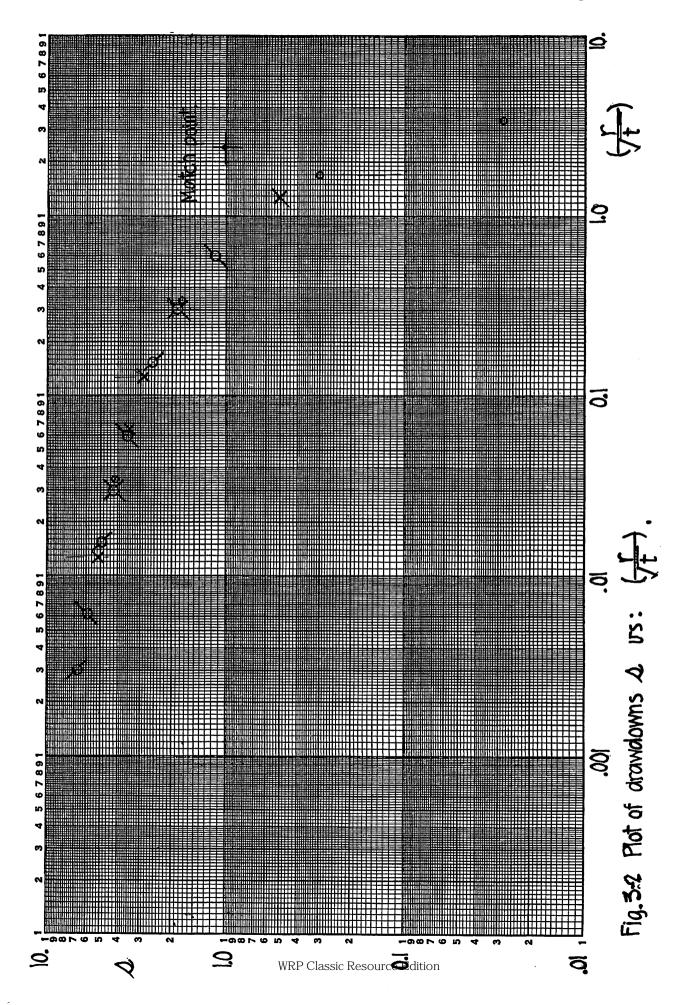
From the second pair of values, by substitution:

$$\frac{2.40}{\sqrt{4\alpha}}$$
 = 0.980 α = 1.499 ft²/sec

The values obtained, 0.2558 and 1.499 should be, as is known, 0.256 and 1.50, respectively. The errors are less than one percent. The effective voids ratio is:

$$V = \frac{KD}{\alpha} = \frac{0.2558}{1.499} = 0.171$$

This figure corresponds to V = 0.171 as used in the original computation.



Some justification for using the point 1.0 - 1.0 of figure 3-1 as a match point needs to be made. The logic of the procedure would be more readily understood if we used a selected one of the observed points of figure 3-2 as the basis of the computation. If the validity of such a procedure be granted, then it may be seen that so long as the observed points of figure 3-2 are adjusted to agree with the curve of figure 3-1, the match point can be shifted to the convenient position 1.0 - 1.0 because the shift represents multiplication by the same factor on both charts. In subsequent work this factor will divide out.

Some scatter is to be expected in a set of points representing field observations because of aquifer irregularities. The presence of impermeable boundaries or of streams where the water level is maintained will also cause irregularities. The "Theory of Aquifer Tests" as treated in U.S.G.S. Water Supply Paper 1536-E of 1962, will supply additional details.

The case where the drawdown s is not everywhere small compared to the initial saturated depth D has been treated by Glover and Bittinger. They obtain a second approximation, based upon Dupuit-Forchheimer concepts, by accounting, approximately, for the restriction of the area available for flow of ground water as a result of drawdown. The relationship obtained is:

$$\frac{y}{\left(\frac{Q}{2\pi KD}\right)} = \frac{1}{\sigma} \left(1 - \sqrt{1 - 2\sigma} \int_{-\sqrt{4\alpha t}}^{\infty} \frac{e^{-u^2} du}{u}\right)$$
 (3-2)

Where:

$$\sigma = \frac{Q}{2\pi KD^2} \tag{3-3}$$

Table 2 has been prepared to facilitate the use of this relationship. An example of its application will be given later in the section on the effect of pumping over an area.

A development of much interest is that of N. S. Boulton who attacks the problem, of estimating drawdowns due to pumping a well, by utilizing the Laplace type of formulation as a basis for deriving formulas.

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Water table maintained at r = b.

Constant pumping rate Q.

For the ultimate steady state, the flow everywhere must be Q, then,

$$-2\pi r KD \frac{ds}{dr} = Q$$

or

$$\frac{ds}{dr} = \frac{-Q}{2\pi KD} \frac{1}{r}$$

By integration, if s = 0 when r = b,

$$s = \frac{Q}{2\pi KD} \log_e(\frac{b}{r})$$

To meet the initial condition that s = 0 for t = 0 for 0 < r < b use a Fourier-Bessel series of the type

$$s_1 = \sum_{n=1}^{n=\infty} A_n J_o(\beta_n r) e^{-\frac{(\beta_n b)^2 \alpha t}{b^2}}$$

Which is a solution of equation 2-5.

The A values are to be obtained from the relation

$$\int_0^b \frac{Q}{2\pi KD} \log_e(\frac{b}{r}) J_o(\beta_n r) r dr = A_n \int_0^b J_o^2(\beta_n r) r dr.$$

or

$$A_{n} = \frac{\int_{0}^{b} \frac{Q}{2\pi KD} \log_{e}(\frac{b}{r}) J_{o}(\beta_{n}r) r dr}{\int_{0}^{b} J_{o}^{2}(\beta_{n}r) r dr}$$

Let

$$w = \beta_n r$$
 $dr = \frac{dw}{\beta_n}$ $r = \frac{w}{\beta_n}$

Then:

$$\int_{0}^{b} J_{o}^{2}(\beta_{n}r) r dr = \frac{1}{\beta_{n}^{2}} \int_{0}^{\beta_{n}b} J_{o}^{2}(w) w dw = \frac{\beta_{n}^{2}b^{2}}{\beta_{n}^{2}2} [J_{1}^{2}(w) + J_{o}^{2}(w)]_{0}^{\beta_{n}b}$$
$$= \frac{b^{2}}{2} [J_{1}^{2}(\beta_{n}b) + J_{o}^{2}(\beta_{n}b)]$$

$$\int_{0}^{b} \frac{Q}{2\pi KD} \log_{e}(\frac{b}{r}) J_{o}(\beta_{n}r) r dr = \frac{Q}{2\pi KD} \int_{0}^{b} \log_{e}(\frac{b}{r}) J_{o}(\beta_{n}r) r dr.$$

By parts, let:

$$u_1 = \log_e(\frac{b}{r}) \qquad dv_1 = r J_o(\beta_n r) dr$$

$$du_1 = -\frac{dr}{r} \qquad v_1 = \frac{r J_1(\beta_n r)}{\beta_n}$$

Then the last of these integrals is to be evaluated in the following way.

Note that the first term vanishes at both limits.

$$\frac{Q}{2\pi KD} \int_{0}^{b} \log_{e}(\frac{b}{r}) J_{o}(\beta_{n}r) r dr = \frac{Q}{2\pi KD} \frac{rJ_{1}(\beta_{n}r)}{\beta_{n}} \log_{e}(\frac{b}{r})$$

$$+ \frac{Q}{2\pi KD} \int_{0}^{rJ_{1}(\beta_{n}r)} \frac{dr}{\beta_{n}r} dr$$

$$= \frac{Q}{2\pi KD} \frac{-J_{o}(\beta_{n}r)}{\beta_{n}^{2}} \Big]_{0}^{b} = \frac{Q}{2\pi KD} \frac{1}{\beta_{n}^{2}} [1 - J_{o}(\beta_{n}b)]$$

And

$$A_{n} = \frac{Q}{2\pi KD} \frac{2[1 - J_{o}(\beta_{n}b)]}{(\beta_{n}b)^{2}[J_{o}^{2}(\beta_{n}b) + J_{1}^{2}(\beta_{n}b)]}$$

Since the boundary conditions will require that $J_o(\beta_n b) = 0$ this expression reduces to:

$$A_{n} = \frac{Q}{2\pi KD} \frac{2}{(\beta_{n}b)^{2} J_{1}^{2}(\beta_{n}b)}$$
 (3-4)

Then the final form of the solution is:

$$s = \frac{Q}{2\pi KD} \left[\log_{e}(\frac{b}{r}) - \sum_{n=1}^{n=\infty} \frac{2J_{o}(\beta_{n}r)e^{-(\beta_{n}b)^{2}}(\frac{\alpha t}{b^{2}})}{(\beta_{n}b)^{2}J_{1}^{2}(\beta_{n}b)} \right]$$
(3-5)

This function may be evaluated by use of Table 6.

Impermeable outer boundary present

The developments described at the beginning of this chapter apply where the outer boundary of the aquifer, or reservoir if the flow is of oil, is so remote that the disturbance created by the well has not had time to reach the outer boundary. All real aquifers are of finite extent, however, and if the removal of water or oil continues long enough the effect of the outer boundary must become apparent. If the actual case can be idealized as being circular, of outer radius b, with a well at the center in the form of a line sink flowing at the rate Q, then the required solution of the differential equation;

$$\alpha_1 \left(\frac{\partial^2 y}{\partial r^2} + \frac{1}{r} \frac{\partial y}{\partial r} \right) = \frac{\partial y}{\partial t}$$

is:

$$\frac{y}{(\frac{Q}{2\pi KD})} = \left[\log_{e}(\frac{b}{r}) + \frac{r^{2}}{2b^{2}} - \frac{3}{4} + \frac{2\alpha_{1}t}{b^{2}} - \sum_{n=1}^{n=\infty} \frac{2J_{o}(\beta_{n}r)e^{-(\beta_{n}b)^{2}}(\frac{\alpha_{1}t}{b^{2}})}{(\beta_{n}b)^{2}J_{o}^{2}(\beta_{n}b)}\right]$$

(3-6)

Table 7 contains values of $\frac{y}{\left(\frac{Q}{2-VD}\right)}$ for values of $\left(\frac{\alpha_1 t}{b^2}\right)$. The postulates

used here are the same as for the flowing artesian well development. assumed that the fluid occupies interstices in a porous bed sandwiched between impermeable members and that a pressure is originally present. the original conditions the assumption is that a part of the weight of the overburden is carried by this fluid pressure and that when this pressure is relieved by operation of a well the load that it originally carried is shifted to the grain structure which is compressed as a result. The quantity $\,{
m V}_{1}\,\,$ then represents the quantity of fluid squeezed out of the interstices of a column of the permeable bed having a unit horizontal area and a height D when the pressure on the column is increased by the pressure equivalent to that exerted by a one foot depth of water.

Example

A case of oil flow will be considered. Bottom hole oil pressures will be expressed in equivalent feet of water in conformity with the usages adopted herein. Laboratory determinations of permeability will be expressed in terms of the permeability for water and the permeabilities for other fluids obtained by applying a ratio of viscosities. Then the permeability for oil will be

$$K_{O} = \frac{K_{W} \mu_{W}}{\mu_{O}}$$

In this manner the units required will be reduced to two; a length, which will be expressed in feet and a time unit which will be expressed in days.

In the first part of the example pressure drawdowns will be computed for a set of assumed well test conditions and in the second part the method of estimating the reservoir properties from the test data will be shown.

As a basis for the computations assume that a=7.5 feet, b=750 feet, D=11.0 feet, $\mu_0=1.25$ centipoises. Bottom hole pressure with the well shut in is 1500. $1b/in^2$. From laboratory tests on a sample of the oil sand it has been determined that the porosity ϕ , representing the ratio of the volume of oil in the sample to the gross volume of the sample, is 0.12 and that the compressibility C_t , expressed as the reduction in volume under a pressure of one pound per square inch, to the initial volume is 18.5×10^{-6} . The permeability K_w for water at a temperature such that the viscosity μ_w is 1.0 centipoise is found to be 0.150 ft/day. Then,

$$K_o = \frac{(0.150)(1.00)}{1.250} = 0.120(\frac{ft}{day})$$
 . $K_o D = (0.120)(11.0) = 1.320(\frac{ft^2}{day})$

Bottom hole pressure is (1500)(2.307) = 3460. feet of water.

$$C_{w} = 18.5(10)^{-6}/2.307 = 8.0 \times 10^{-6} \text{ per foot of water.}$$

$$V_1 = (0.12)(8.0)(10)^{-6}(11.0) = 10.560(10)^{-6}$$
 (Dimensionless)
 $\alpha_1 = \frac{K_0 D}{V_1} = \frac{(1.320)(10)^6}{10.560} = 125000.(\frac{ft^2}{day})$
 $b^2 = 750^2 = 562500. ft^2$

Flow rate; 400 barrels per day or 2246 cubic feet per day. A barrel, as used here, is 42 gallons or 5.615 cubic feet.

$$\frac{Q}{2\pi K_0 D} = \frac{2246.}{(6.283)(1.320)} = 270.812 \text{ feet.}$$
 The effective radius is 7.5 feet. Then $(\frac{a}{b}) = 0.010$.

Computation of the pressure drawdown at the well is made in the manner shown below. Note that:

$$\frac{\alpha_1 t}{a^2} = \frac{\alpha_1 t}{b^2} \frac{b^2}{a^2} . \qquad (\frac{\alpha_1}{b^2}) = \frac{125000}{562500} = 0.222222$$

Table 1 Computation of pressure reduction at a = 7.5 ft.

Time (minutes)	Time (Days)	$(\frac{\alpha_1^t}{b^2})$	$(\frac{\underline{y}^*}{2\pi K_0}D)$	$(\frac{4\alpha_1t}{b^2})$	$(\frac{a}{4\alpha_1 t})$	$\int_{0}^{\infty} \frac{e^{-u^{2}} du}{u}^{*}$ $(\frac{a}{\sqrt{4\alpha_{1}t}})$	y (feet)
0	0	0		0	∞	0	0
10	.00694	.001542		.006168	.1273	1.780	482.0
20	.01389	.003087		.012348	.0902	2.120	574.1
30	.02083	.004629		.018516	.0735	2.324	629.4
60	.04167	.009260		.037040	.0520	2.669	722.8
90	.06250	.013889	2.842	.055556	.0424	2.871	777.5
120	.08333	.018518	3.002				813.0
150	.10417	.023149	3.116				843.8
180	.12500	.027778	3.210				869.3
240	.16667	.037037	3.358				909.4
360	.25000	.055556	3.561				964.6
720	.50000	.111111	3.910				1058.9
1080	.75000	.166667	4.115				1143.4
1440	1.00000	.222222	4.266				1155.3
1800	1.25000	.277778	4.395				1190.2
	1.50000	.333333	4.515				1222.7
2520	1.75000	.388889	4.628				1253.3
2880	2.00000	.44444	4.742				1284.2

From Table 7.

From Table 1.

A reference to Table 7 will show that the disturbance caused by production does not reach the outer boundary at (r/b) = 1 until $(\alpha_1 t/b^2) = 0.02$. For values of $(\alpha_1 t/b^2)$ smaller than this the appropriate values of $y/(Q/2\pi K_0 D)$ can be obtained from Table 1, as shown above. Note that in this case

$$\frac{\frac{y}{(\frac{Q}{2\pi K_0 D})}}{\frac{r_w}{\sqrt{4\alpha_1 t}}} = \int_{-\infty}^{\infty} \frac{e^{-u^2} du}{u}$$

Determination of reservoir properties from drawdown test data.

As a first step, prepare a master chart from Tables 7 and 1 showing curves for $y/(Q/2\pi K_0D)$ versus (α_1t/b^2) for a series of ratios of (r/b) as shown on figure 3-3. Plot the drawdowns y from the above table versus t, treating them as test values, as shown on figure 3-4. Then match the observed y versus t curve against one of the curves of the master chart. It will be found to fit with the curve for (r/b) = 0.01. A selected data point and the corresponding match point on the master chart are indicated by crosses. Corresponding to the selected data point y = 1058.9 is the point

$$\frac{y}{(\frac{Q}{2\pi K_0 D})} = 4.0.$$

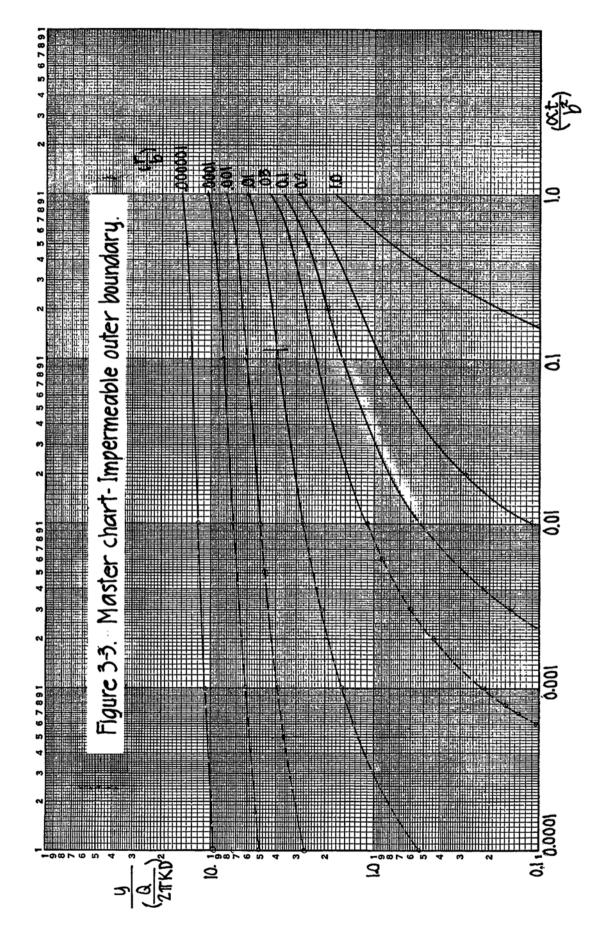
Since y and Q are both known quantities this can be solved for K_{0}^{D} then

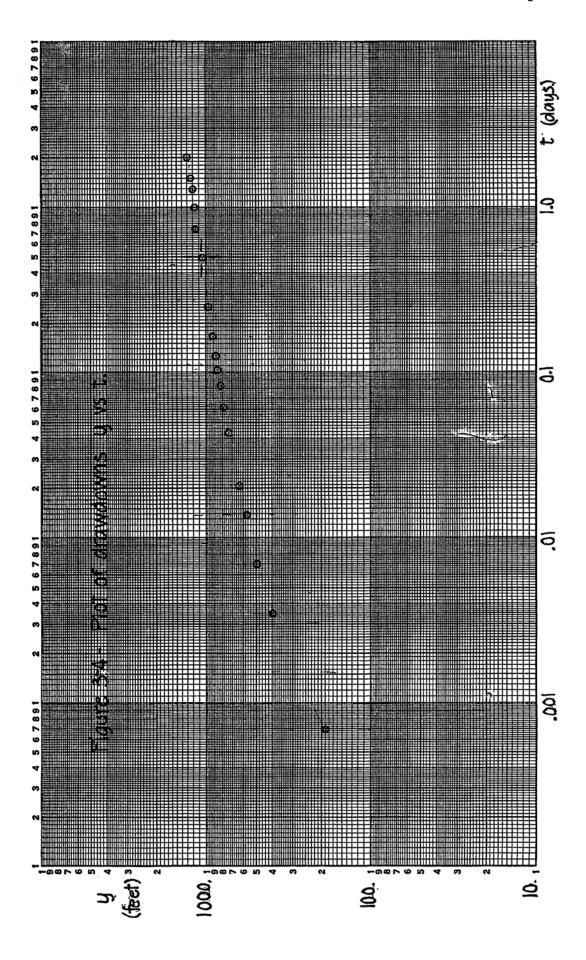
$$K_0D = \frac{4.0Q}{2\pi y} = \frac{(4.0)(2246)}{(6.283)(1058.9)} = 1.350 \text{ (ft}^2/\text{day)}$$

This should be 1.320 ft²/day as this is the value we began with.

$$\alpha_1 = \frac{K_0 D}{V_1} = \frac{(1.350)(10)^6}{10.560} = 127840. (ft^2/day)$$

We know this should be 125000 ft²/day.





From the corresponding abscissas;

t = 0.500 days and
$$(\alpha_1 t/b^2) = 0.113$$
. Then,
$$b^2 = \frac{\alpha_1 t}{0.113} = \frac{(127840)(0.500)}{0.113} = 565663$$
.
$$b = 752$$
. feet

We know this should be 750 feet. Since the (r/b) ratio was found to be .01 the effective well radius is

$$a = (752)(.01) = 7.52 \text{ feet}$$

Then by this process of curve matching we have recovered the reservoir properties.

Based upon the recovered properties we can infer that the whole volume of oil in the reservoir is

$$\pi b^2 D \phi = \pi (565663) (11.0) (0.12) = 2346053 \text{ ft}^3$$
or 418000 barrels

Chapter 3

Problems

- (3-1) Farmers A and B live on adjoining farms and both have wells used to supply irrigation water. Their wells are 2500 feet apart and fully penetrate a saturated thickness of 60 feet. The aquifer constant is $\alpha = 0.75 \text{ ft}^2/\text{sec}$. How much lowering of the water table will A's well cause at B's well if he lifts 1.2 ft $^3/\text{sec}$ for a period of four months? The permeability is .0025 ft/sec and the effective voids ratio is 0.2. Answer: 0.783 feet.
- (3-2) If A's well is gravel packed out to a radius of 1.25 feet what will be drawdown at the outside of the gravel pack after four months of pumping under the conditions described in (1)?

Answer: 11.46 feet.

(3-3) For purposes of checking an analog design a supplemental panel is set up with analog components to represent a circular area of five mile radius. The outer components are connected to the ground wire to represent a maintained water table elevation there. An input lead is connected to the node point at the center of the panel to represent a well pumped at a constant rate of $10.0~\rm ft^3/\rm sec$. The analog is designed to represent the conditions $D = 200~\rm ft$, $K = 0.001~\rm ft/\rm sec$, V = 0.20. By using Table 6 compute the drawdowns which should be shown by the analog test panel at the end of one year of continuous pumping. Node points are at 0.5 mile intervals.

Answer:

Radius (Miles)	Drawdown (feet)
0	
0.5	9.421
1.0	4.528
1.5	2.233
2.0	1.063
2.5	0.478

Answer: (3-3) continued

Radius	Drawdown
(Miles)	(feet)
3.0	0.201
3.5	0.078
4.0	0.028
4.5	0.009
5.0	0

(3-4) The Fox Hills formation in the Denver area is a sandstone member confined between the Pierre Shale below and the Laramie formation above. It outcrops in a roughly elliptical zone which is about 110 miles across in the North-South direction and 70 miles across in the East-West direction.

The Fox Hills sandstone within the outcrop has the form of a synclinal fold and at Denver the top of this member is about 1500 feet below ground level. The bed is filled with water under pressure and in an earlier era flowing artesian wells were obtained from it. If it be assumed that the total flow of the early wells was 10,000 gallons per minute and that the pressure was originally 200 feet of water at Denver estimate how long would be required for the wells to cease to flow if the wells were distributed over a circle nine miles in diameter. Assume that the aquifer properties are $KD = 0.002 \text{ ft}^2/\text{sec}$, V = 0.0004, $\alpha = 5.0 \text{ ft}^2/\text{sec}$ and that the recharge rate is small compared to the well flow.

Answer: 0.910 years.

Suggestion: If Table 7 does not have the needed value, but does show that there would have been no drawdown at the outer boundary for greater times, then the case can be considered as an infinitely extended aquifer and a solution can be obtained by use of Table 1.

(3-5) Convert the data of Problem (3-1) to units of meters and seconds and solve for a drawdown in meters. Convert this drawdown back to feet and compare with the result obtained for Problem (3-1).

Answer: 0.239 meters or 0.784 feet.

Chapter 4.

The flowing artesian well

The processes of uplift, erosion and deposition have been important factors in producing the sedimentary formations which are prominent features of the earth as we know it today. Operation of the geologic forces which produced these changes was often erratic and it has happened that a permeable bed, such as a sandstone, has been sandwiched in betweem impermeable strata such as shale. If subsequent changes warp these strata into a synclinal fold with the permeable bed exposed at the surface along an outcrop then the conditions shown in figure 4-1 can become a reality. Recharge then occurs

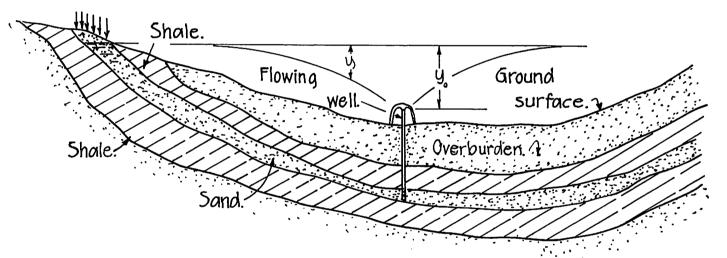


Fig. 4-1 Section showing an artesian stratum. as a result of precipitation along the outcrop and the permeable member contains water under pressure. If a well is now sunk to the permeable bed, as shown, a flowing well can be obtained.

The combination of conditions needed to produce such a flowing well are not of common occurrence but erratic geologic processes have sometimes produced them. The Fox Hills formation below the Denver area is an example. Here the formation occupies an elongated saucer shaped depression confined between the Laramie formation above and the Pierre Shale below. Outcrops are to be found in the Greeley area to the north, in the plains area to the east

and in the Colorado Springs area to the south. To the west of Denver the Fox Hills formation outcrops along the eastern face of the foothills. In earlier times flowing wells could be obtained but the replenishment rate is slow and now wells tapping this formation commonly must be pumped if water is to be obtained. Another example is to be found in the Artesia area of New Mexico.

Because recharge rates are typically low it is easy to overencumber the supply and some areas which once produced flowing wells must now be pumped if water is to be obtained. So long, however, as artesian wells are obtainable it will be important to have an analytical treatment by which the changes of pressure they produce and the rates of flow they can maintain can be estimated.

A treatment can be based upon the idealization that the hydraulic properties of a water bearing stratum can be specified by a permeability K, a thickness D and a yield factor V_1 . This latter factor will be much smaller than would be found for a free water table case because under artesian conditions there is no unwatering of the aquifer. The water which flows from the well comes from an expansion of the water due to relief of pressure and from compression of the granular material of the aquifer. It may be noted in passing that the compression possible in a granular material, by application of pressure, is much greater than could be produced in an equal volume of solid material of the same kind. Before the pressures are disturbed, much of the weight of the overburden is sustained by them. When the water pressures are relieved this load is transferred to the grain structure and compression follows.

If we let y represent the pressure reduction in the aquifer, expressed in equivalent feet of water, then the flow toward the well at the radius r

will be:

$$F = -2\pi r KD \frac{\partial y}{\partial r} .$$

The continuity condition is:

$$\frac{\partial F}{\partial r} drdt = -2\pi r dr V_1 \frac{\partial y}{\partial t} dt$$
.

or

$$\frac{\partial F}{\partial r} = -2\pi r V_1 \frac{\partial y}{\partial r}$$

Then, by substitution and rearrangement

$$-2\pi KD \left(r \frac{\partial^2 y}{\partial r^2} + \frac{\partial y}{\partial r}\right) = -2\pi r V_1 \frac{\partial y}{\partial t}$$

Ιf

$$\alpha_1 = \frac{KD}{V_1}$$

The relation can be put into the form:

$$\alpha_1 \left(\frac{\partial^2 y}{\partial r^2} + \frac{1}{r} \frac{\partial y}{\partial r} \right) = \frac{\partial y}{\partial t}$$

This relation is of the same form as was obtained for the case of the well drawing water from an aquifer with a free water table but, in the present case, no concession needs to be made to avoid nonlinearities and the treatment is exact.

In the previous case the well flow was taken to be constant and the drawdown at the well varied with time. In the present case the pressure drawdown at the well is a constant amount y_0 and the discharge of the well varies with time. A similar idealization is often appropriate where a flowing well is producing oil from an extensive oil sand. The differential

equation 2-5 is then to be solved subject to the conditions

$$y = y_0$$
 at $r = a$ for $t > 0$

$$y = 0$$
 when $t = 0$ for $r > a$

The required solution has been obtained by a number of investigators. Among them are Nicholson, Smith, Goldstein and Carslaw and Jaeger. A summary of these investigations as well as references to original sources may be found in Carslaw and Jaeger, 1947. The solution so obtained is in the form of an infinite integral. Evaluation is difficult. An alternative approach to a solution for the case where the outer boundary is infinitely remote is to work in terms of a finite outer radius b. This solution will behave as an infinitely remote outer boundary case until the disturbance produced by flow from the well of radius a reaches the outer boundary b. By using a sequence of increasingly remote outer boundaries it is possible to compute with a limited number of terms of the series solution and to extend the outer boundary to as remote a location as may be desired.

The required solution is:

$$y = y_0 \left[1 - \sum_{n=1}^{n=\infty} A_n U_0(\beta_n r) e^{-\frac{k^2(\beta_n b)^2}{4} (\frac{4\alpha t}{a^2})} \right]$$
 (4-1)

This solution meets the condition that

$$\frac{\partial y}{\partial r} = 0$$
 when $r = b$.

The A_n values are to be computed from the expression:

$$A_{n} = \frac{\frac{2k}{(\beta_{n}b)} U_{o}' (\beta_{n}a)}{[U_{o}(\beta_{n}b)]^{2} - k^{2}[U_{o}'(\beta_{n}a)]^{2}}$$
(4-2)

Where

$$U_o(\beta_n r) = J_o(\beta_n a) Y_o(\beta_n r) - Y_o(\beta_n a) J_o(\beta_n r)$$
(4-3)

and

$$k = (\frac{a}{b}) \tag{4-4}$$

The $(\beta_n b)$ values are to be obtained as roots of the equation:

$$U_O'(\beta_n b) = 0 (4-5)$$

In these expressions:

$$U_o^{\dagger}(\beta_n \mathbf{r}) = \frac{dU_o(\beta_n \mathbf{r})}{d(\beta_n \mathbf{r})}$$

$$J_o'(\beta_n r) = \frac{dJ_o(\beta_n r)}{d(\beta_n r)} = -J_1(\beta_n r)$$

$$Y_o(\beta_n r) = \frac{dY_o(\beta_n r)}{d(\beta_n r)} = -Y_1(\beta_n r)$$

The flow toward the well at the radius r is:

$$F = 2\pi r KD \frac{\partial y}{\partial r} = 2\pi KDy_0 \sum_{n=1}^{n=\infty} A_n (\beta_n r) U_0' (\beta_n r) e^{-\frac{k^2 (\beta_n b)^2}{4} (\frac{4\alpha_1 t}{a^2})}$$
(4-6)

If the flow at the radius a is

$$Q = 2\pi KDy_0 G\left(\frac{\sqrt{4\alpha_1 t}}{a}\right)$$
 (4-7)

Then:

$$G\left(\frac{\sqrt{4\alpha_1 t}}{a}\right) = \sum_{n=1}^{n=\infty} A_n (\beta_n a) U_0' (\beta_n a) e^{-\frac{k^2 (\beta_n b)^2}{4} \left(\frac{4\alpha_1 t}{a^2}\right)}$$
(4-8)

Values of (y/y_0) can be obtained from Table 3 and of $G(\sqrt{4\alpha t}/a)$ from Table 4.

Values outside the tables

It is quite possible that a need might arise to deal with values which are beyond the scope of the tables. Approximate procedures can be outlined to take care of such cases. For computation of pressure drawdowns the following expression will be useful:

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$$y \approx y_0 \frac{\int_0^{\infty} \frac{e^{-u^2} du}{u}}{\int_0^{\infty} \frac{e^{-u^2} du}{u}}$$
 if $(\frac{\sqrt{4\alpha_1 t}}{a}) > 1000$. (4-9)

This expression is based upon the concept that the flow from an artesian well becomes almost steady after the well has been flowing for some time. The formula for drawdowns produced by a well of constant flow is then used and the pressure drawdown y at the radius r is estimated by applying the ratio of the drawdown at the radius r to the drawdown at the radius r to the known drawdown r at the radius r.

A comparison of approximate and tabular values is shown in the following table:

Table 4-1 Comparison of approximate and tabular values of (y/y_0) for an artesian well.

$$\frac{(\frac{\mathbf{r}}{a})}{(\frac{\mathbf{r}}{\sqrt{4\alpha_{1}t}})} \quad \frac{\text{Tabular}}{\text{values of}} \quad \int_{(y/y_{0})}^{\infty} \frac{e^{-u^{2}} du}{u} \quad \text{Values of} \quad (\frac{\sqrt{4\alpha_{1}t}}{a})$$

$$\frac{1.00}{10.00} \quad 0.001 \quad 1.0000 \quad 6.6192 \quad 1.0000 \quad 1000$$

$$10.00 \quad 0.010 \quad 0.6557 \quad 4.3166 \quad 0.6521 \quad 1000$$

$$100.00 \quad 0.10 \quad 0.3118 \quad 2.0190 \quad 0.3050 \quad 1000$$

$$1000.00 \quad 1.00 \quad 0.0188 \quad 0.1097 \quad 0.0165 \quad 1000$$

$$1000.00 \quad 1.00 \quad 0.0188 \quad 0.1097 \quad 0.0165 \quad 1000$$

$$1000.00 \quad 1.00 \quad 0.0188 \quad 0.1097 \quad 0.0165 \quad 1000$$

$$1000.00 \quad 1.00 \quad 0.0188 \quad 0.1097 \quad 0.0165 \quad 1000$$

$$1000.00 \quad 1.00 \quad 0.0188 \quad 0.1097 \quad 0.0165 \quad 1000$$

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$$1000.00 \quad 1.00 \quad 0.0188 \quad 0.1097 \quad 0.0165 \quad 1000$$

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$$1000.00 \quad 0.0188 \quad 0.1097 \quad 0.0165 \quad 1000$$

$$1000.00 \quad 0.0188 \quad 0.1097 \quad 0.0165 \quad 1000$$

$$1000.00 \quad 0.0188 \quad 0.1097 \quad 0.0165 \quad 1000$$

Values in the third and fifth columns should be compared. It should be expected that the approximation would improve as times grow longer.

Since it can be shown that, on the basis used for development of formula 4-9,

$$y = y_0 G(\frac{\sqrt{4\alpha_1 t}}{a}) \int_0^\infty \frac{e^{-u^2} du}{u} \qquad \text{if } (\frac{\sqrt{4\alpha_1 t}}{a}) > 1000. \tag{4-10}$$

a comparison of formulas 4-9 and 4-10 would indicate that an approximation of the following type could be obtained:

$$G(\frac{\sqrt{4\alpha_1 t}}{a}) \cong -\frac{1}{\int_{-\infty}^{\infty} \frac{e^{-u^2} du}{u}}$$
 if $(\frac{\sqrt{4\alpha_1 t}}{a}) > 1000$. (4-11)

A comparison is shown below. In this table only the last two values would be admissable under the restriction imposed on formula 4-11.

Table 4-2 Comparison of approximate and tabular values for $G(\frac{\sqrt{4\alpha_1}t}{a})$

$(\frac{\sqrt{4\alpha_1 t}}{a})$	$G(\frac{\sqrt{4\alpha_1 t}}{a})$	(<u>a</u>)	$\int_{0}^{\infty} \frac{e^{-u^2} du}{u}$	$(1/\int_{-u}^{\infty} \frac{e^{-u^2} du}{u})$
	(tabular)	√4α ₁ t	$(\frac{a}{\sqrt{4\alpha_1 t}})$	$(\frac{a}{\sqrt{4\alpha_1 t}})$
100	.22585	0.010000	4.3166	0.2317
200	.19593	0.005000	5.0097	0.1996
300	.18177	0.003333	5.4151	0.1846
400	.17288	0.002500	5.7029	0.1753
500	.16655	0.002000	5.9260	0.1687
600	.16171	0.001667	6.1086	0.1637
700	.15783	0.001428	6.2630	0.1596
800	.15461	0.001250	6.3960	0.1563
900	.15188	0.001111	6.5137	0.1535
1000	.14952	0.001000	6.6192	0.1510
10000	.11146	0.000100	8.9217	0.1121

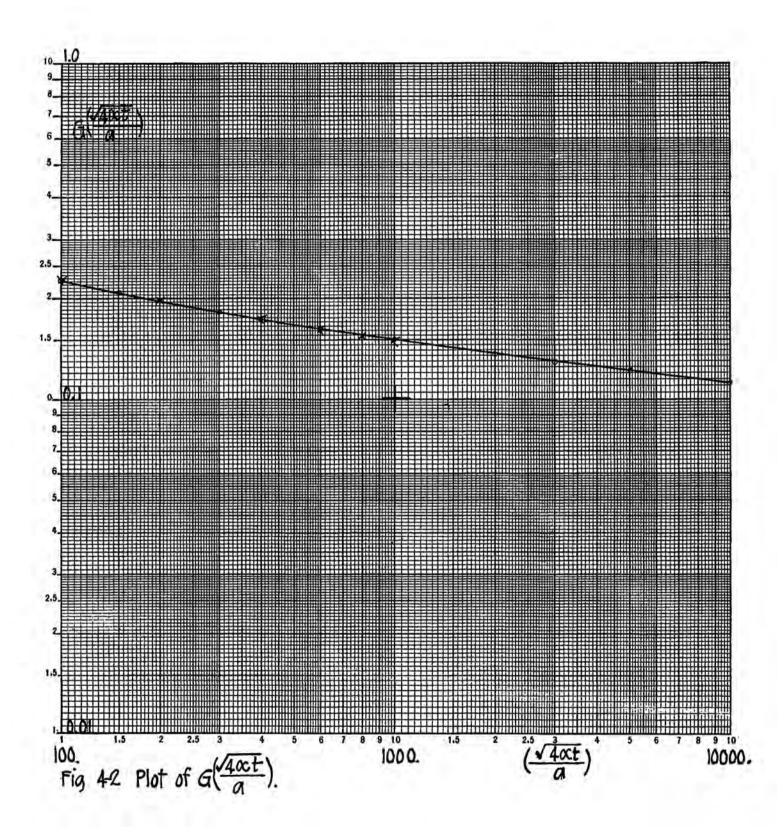
Values in the second and fifth columns should be compared. At the limit of tabular values the error is about one-half percent. The approximation should improve for values of $(\sqrt{4\alpha_1 t}/a)$ greater than 1000.

Example

Data furnished through the courtesy of Mr. Stan Lohman, of the U.S. Geological Survey, will first be used to make a determination of aquifer properties. The aquifer properties so obtained will then be used to make an estimate of the drawdowns produced by flows at the test well on pressures at an adjacent well. A description of these well tests will be found in the paper by Jacob and Lohman.

A plot of a portion of the $G(\frac{\sqrt{4\alpha_1 t}}{a})$ function is first made on logarithmic cross section paper as shown on figure 4-2. The test data are reduced as shown in table 4-3, and a plot of Q vs (\sqrt{t}/a) is then made on logarithmic paper, as shown on figure 4-3. The observed data are then matched to the type curve data by superimposing the plots and shifting them while keeping the axes parallel. When a satisfactory match is obtained the point 0.1 - 1000 of the type curve is marked on the test data plot. These points are marked on the charts.

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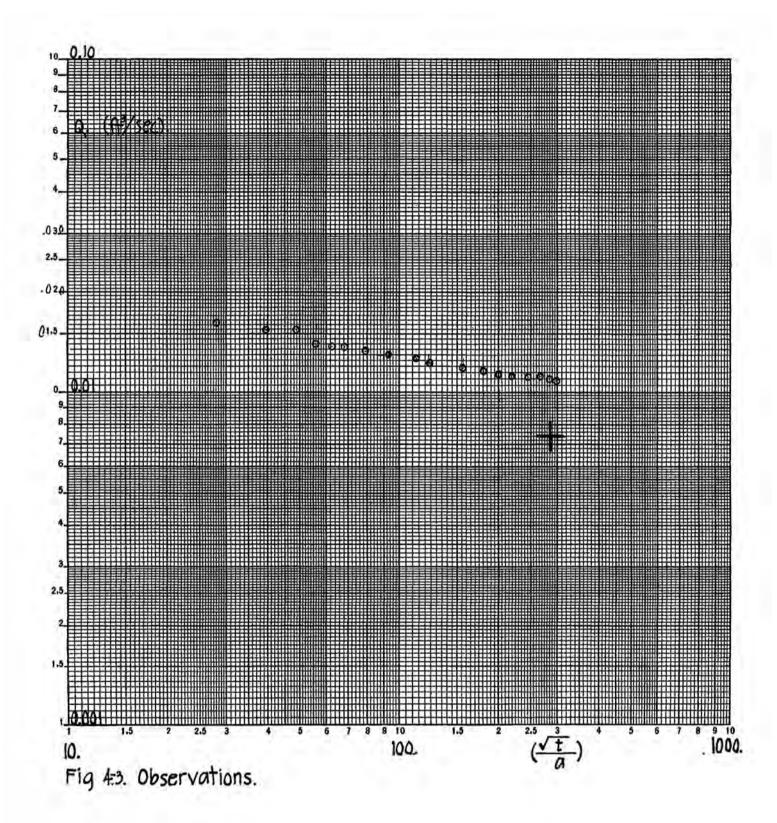


Table 4-3 Artesian well test.

Time of observation	Rate of flow	Time since flow began minutes	Rate of flow $(\frac{\text{ft}^3}{\text{sec}})$	Time since flow began (seconds)	$(\sqrt{\frac{t}{a}})$
10:29 A.M.	_	0		0	0
10:30	7.28	1.	.01622	60	28.07
10:31	6.94	2.	.01546	120	39.69
10:32	6.88	3.	.01533	180	48.61
10:33	6.28	4.	.01400	240	56.13
10:34	6.22	5.	.01386	300	62.76
10:35	6.22	6.	.01386	360	68.75
10:37	5.95	8.	.01326	480	79.38
10:40	5.85	11.	.01303	660	93.08
10.45	5.66	16.	.01261	960	112.26
10:50	5.50	21.	.01225	1260	128.61
10:55	5.34	26.	.01190	1560	143.10
11:00	5.34	31.	.01190	1860	156.26
$11:10^{1/2}$	5.22	41.5	.01163	2490	180.80
11:20	5.14	51.	.01145	3060	200.42
11:30	5.11	61.	.01138	3660	219.20
11:45	5.05	76.	.01125	4560	244.66
12:00N	5.00	91	.01114	5460	267.72
12:12 P.M.	4.92	103	.01096	6180	284.83
12:22	4.88	113	.01087	6780	298.34

Shutoff pressure, $y_0 = 92.33$ feet of water.

Well radius 0.276 feet.

Penetration 60 feet.

Thickness of aquifer, D = 100 feet (Estimated from thicknesses at the location of nearby fully penetrating wells).

From chart matching, on the basis that the well is fully penetrating.

$$(\frac{Q}{2\pi KDy_0}) = 0.100$$
 $Q = 0.007400$ ft³/sec. KD = 0.00012755 ft²/sec., $(\frac{\sqrt{4\alpha_1 t}}{a}) = 1000$. $(\frac{\sqrt{t}}{a}) = 285$. $\alpha = 3.076$ ft²/sec = $(\frac{KD}{V_1})$

Then

K = 0.0000012755 ft/sec. $KD = (0.00012755)(86400) = 11.04 \text{ feet}^2 \text{ per day.}$ Lohman's value is 11.7 feet² per day.

Total production volume

If the pressure reduction is:

$$y = y_0 \left[1 - \sum_{n=1}^{n=\infty} A_n U_0(\beta_n r) e^{-\frac{k^2(\beta_n b)^2}{4} (\frac{4\alpha_1 t}{a^2})} \right]$$

The whole production volume out to time t is:

$$P = V_1 \int_a^b 2\pi ry dr = 2\pi V_1 \int_a^b ry dr$$

For the first term, with $k = (\frac{a}{b})$

$$2\pi V_1 y_0 \int_a^b r dr = 2\pi y_0 V_1(\frac{b^2 - a^2}{2}) = 8 y_0 KDt(\frac{a^2}{4\alpha_1 t})(\frac{b^2 - a^2}{2a^2})$$

Since

$$\int_{a}^{b} r U_{o}(\beta_{n} r) dr = -\frac{1}{\beta_{n}^{2}} \left[(\beta_{n} b) U_{o}^{'}(\beta_{n} b) - (\beta_{n} a) U_{o}^{'}(\beta_{n} a) \right] \quad \text{and} \quad$$

because, in this case, $U_0'(\beta_n b) = 0$ this integral reduces to $\frac{(\beta_n a)U_0'(\beta_n a)}{\beta_n^2}$

Then for the second term:

$$2\pi y_{0}V_{1}\int_{a}^{b}A_{n}rU_{0}(\beta_{n}r)e^{-\frac{k^{2}(\beta_{n}b)^{2}}{4}(\frac{4\alpha_{1}t}{a^{2}})}dr = 8\pi KDy_{0}t(\frac{a^{2}}{4\alpha t})\sum_{n=1}^{n=\infty}\frac{A_{n}U_{0}'(\beta_{n}a)}{(\beta_{n}a)}e^{-\frac{(\beta_{n}a)^{2}}{4}(\frac{4\alpha_{1}t}{a^{2}})}$$

Since
$$k = (\frac{a}{b})$$
 and $V_1 = (\frac{KD}{\alpha})$

Finally, the whole production volume is:

$$P = 8\pi KDy_{o}t(\frac{a^{2}}{4\alpha_{1}t})\left[\frac{(b^{2}-a^{2})}{2a^{2}} - \sum_{n=1}^{n=\infty} \frac{A_{n}U_{o}(\beta_{n}a)e}{(\beta_{n}a)} - \frac{(\beta_{n}a)^{2}4\alpha_{1}t}{4}(\frac{1}{a^{2}})\right]$$

or if

$$H(\frac{a^{2}}{4\alpha_{1}t}) = (\frac{a^{2}}{4\alpha_{1}t}) \left[\frac{(b^{2}-a^{2})}{2a^{2}} - \sum_{n=1}^{\infty} \frac{A_{n}U_{o}(\beta_{n}a)e^{-\frac{(\beta_{n}a)^{2}}{4}(\frac{4\alpha_{1}t}{a^{2}})}}{(\beta_{n}a)} \right]$$

$$P = 8\pi KDy_{o}t \cdot H. \tag{4-12}$$

Values of the function H may be found in Table 5. These values have been computed from the relations shown in equations 4-12 above. Values are given for the range of $\frac{\sqrt{4\alpha_1 t}}{a}$ from 3 to 10,000. When an application requires values of H beyond the range of this table the approximate formulas described in the following paragraphs may be used. The graph of figure 4-4 has been prepared using these approximate formulas to extend the range to

$$(\frac{\sqrt{4\alpha_1 t}}{2}) = 1,000,000.$$

Extension of the H function table

Beyond the argument $(\sqrt{4\alpha_1 t}/a) = 1000$, the approximate expression

$$G(\frac{\sqrt{4\alpha_1 t}}{a}) \cong \frac{1}{\int_{0}^{\infty} \frac{e^{-u^2} du}{u}}$$

$$(\frac{a}{\sqrt{4\alpha_1 t}})$$

has been shown to hold with an error of one percent or less. In this range the quantity $(a/\sqrt{4\alpha_1 t})$ will have a value of 0.001 or less. A series development is given in USBR Monograph 31 of the form:

$$\int_{x}^{\infty} \frac{e^{-u^{2}} du}{u} = -0.288608 - \log_{e} x + \frac{x^{2}}{2} - \frac{x^{4}}{2!4} + \frac{x^{6}}{3!6} - \dots$$

In the region: $(\frac{a}{\sqrt{4\alpha_1 t}})$ < 0.001 the approximation

$$\int_{-\infty}^{\infty} \frac{e^{-u^2} du}{u} = -0.288608 - \log_{e} x.$$
 (Approximate for x < 0.001)

would represent the integral with an accuracy of less than one unit in the sixth decimal place. Then

$$G(\frac{\sqrt{4\alpha_1 t}}{a}) = \frac{1}{-0.288608 - \log_e(\frac{a}{\sqrt{4\alpha_1 t}})} . \quad (Approximate for (\frac{\sqrt{4\alpha_1 t}}{a}) > 1000 .$$

If the constant term is treated as a logarithm $-0.288608 = \log_e 0.749306$, then the approximate expression for G can be put in the form

$$G(\frac{\sqrt{4\alpha_1 t}}{a}) \cong \frac{1}{\log_e m(\frac{\sqrt{4\alpha_1 t}}{a})} \text{ or } G(\frac{\sqrt{4\alpha_1 t}}{a}) \cong \frac{2}{\log_e m^2(\frac{4\alpha_1 t}{a^2})}$$

Where m = 0.749307 and $m^2 = 0.561461$. $(1/m^2) = 1.78107$. The total production can then be expressed as:

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$$P = 2\pi KDy_0 \int_0^t G(\frac{\sqrt{4\alpha_1 t}}{a}) dt$$

or:

$$P = 4\pi KDy_{o} \left[\int_{0}^{t} \frac{dt}{\log_{e} m^{2} \left(\frac{4\alpha_{1} t}{a^{2}}\right)} + C_{1} \right]$$

Let

$$x = m^2 (\frac{4\alpha_1 t}{a^2})$$
 $dx = \frac{m^2 4\alpha_1 dt}{a^2}$ $dt = \frac{a^2 dx}{4\alpha_1 m^2}$

Then, by substitution:

$$P = \frac{4\pi KDy_0 a^2}{m^2 + \alpha_1} \left[\int_0^x \frac{dx}{\log_e x} + C_2 \right] \qquad \text{if } \left(\frac{\sqrt{4\alpha_1 t}}{a} \right) > 1000.$$

and:

$$P \approx \frac{4\pi KDy_0 t}{m^2} \left(\frac{a^2}{4\alpha_1 t}\right) \left[\int_0^x \frac{dx}{\log_e x} + C_2 \right]$$
$$\approx 7.124290 \ \pi KDy_0 t \left(\frac{a^2}{4\alpha_1 t}\right) \left[\int_0^x \frac{dx}{\log_e x} + C_2 \right]$$

Where C_1 and C_2 represent constants of integration.

This can be put in the form:

$$P = 8\pi KDy_0 t$$
 0.890536 $(\frac{a^2}{4\alpha_1 t}) \left[\int_0^x \frac{dx}{\log_e x} + \epsilon_2 \right]$ (4-13)

so that

$$H \approx 0.890536 \left(\frac{a^2}{4\alpha_1^t}\right) \left[\int_0^x \frac{dx}{\log_e x} + C_2 \right]$$
 (4-14)

The integral which appears here is known as the logarithmic integral. Its value in terms of the exponential integral is given below in the example. Example

To show the method used for evaluating the logarithmic integral.

Suppose
$$(\frac{\sqrt{4\alpha t}}{a^2}) = 1000$$
. $\frac{4\alpha t}{a^2} = 1000000$.

$$x = m^2(\frac{4\alpha t}{a^2}) = 561460.$$
 $\log_e 561460. = 13.238293$

$$\int_{0}^{x} \frac{dx}{\log_{e} x} = Ei(\log_{e} x) = 46270.$$

Where:

$$Ei(z) = \int_{-\infty}^{z} \frac{e^{+u} du}{u}.$$

is a tabulated function (Jahnke and Emde, 1945--Dwight, 1958, Department of Commerce, 1966).

An evaluation of the constant C_2 now needs to be made. When $(\sqrt{4\alpha_1t/a})$ = 1000 the tabular value of H is 0.040671 and the value obtained from the approximation is 0.041205. Based upon these values C_2 = -0.000534. When $(\sqrt{4\alpha_1t/a})$ = 10000 the tabular value is H = 0.029616 and the corresponding value obtained from the approximate expression based upon the logarithmic integral is H = 0.029794. On this basis C_2 = -0.000178. A comparison shows that the approximation value is coming closer to the tabular value as $(\sqrt{4\alpha_1t/a})$ increases and that there is only about 0.6 percent difference at the end of

the table where $(\sqrt{4\alpha_1 t/a}) = 10~000$. Then for values of $(\sqrt{4\alpha_1 t/a})$ beyond the table it would be reasonable to take $C_2 = 0$ and use the relation:

$$H = 0.890536 \left(\frac{a^2}{4\alpha_1 t}\right) \int_0^x \frac{dx}{\log_e x}$$
 (4-15)

It should be realized that the production function H is based upon developments which imply that the reservoir is of infinite extent. Ultimately the disturbances caused by production of the well will reach the outer boundary of any actual finite reservoir and the production will then fall below that estimated by use of the H function.

Example of application

Estimate the total production of the Artesian test well of Table 4-3 if it flowed unchecked for one year

 $KD = 0.00012756 \text{ ft}^2/\text{sec}$ $\alpha = 3.078 \text{ ft}^2/\text{sec}$ t = 31536000 sec a = 0.276 ft

$$(\frac{\sqrt{4\alpha t}}{a}) = \frac{19698}{0.276} = 71370.$$

This is beyond the H table range. From the graph of figure 4-4

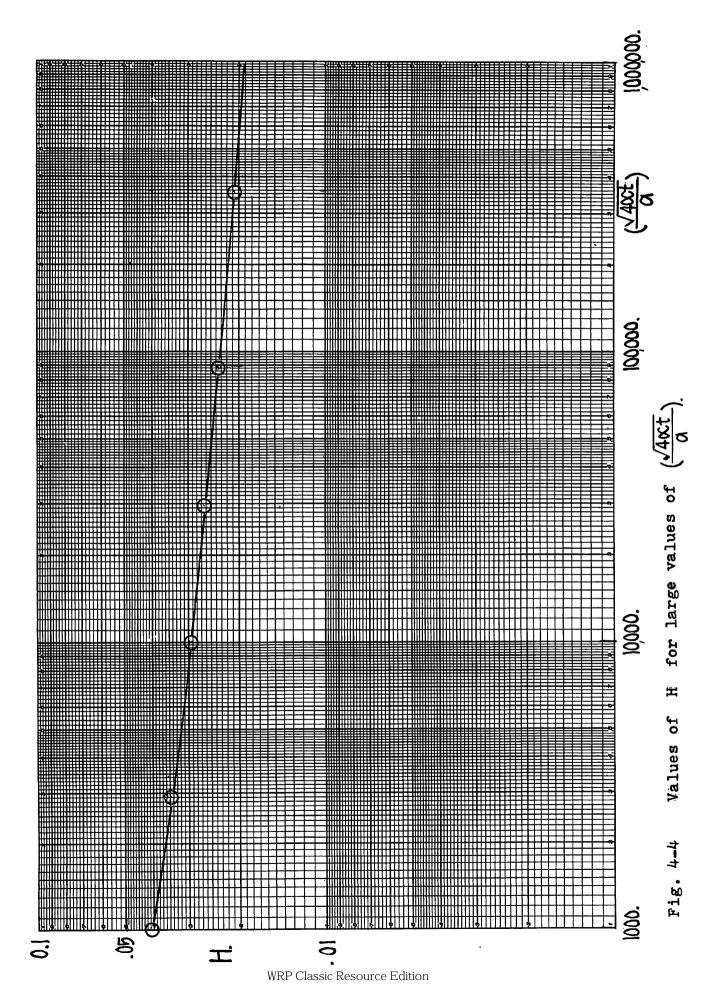
 $H = .0240 8\pi KD = .003206 8\pi KDy_0 t = (003206)(92.33)(31536000) = 9335000.$

 $P = 9335000(.0240) = 224000.ft^3$ or 5.14 acre feet.

As a rough check, the well would have produced

$$\frac{(5.00)(31536000)}{448.8}$$
 = 351300. ft³ or 8.06 acre feet.

if the flow observed at noon of the day of the test had been sustained.



Chapter 4.

Problems.

(4-1) For the Fox Hills aquifer as described for Problem (3-4) compute the pressure drawdown at (r/a) = 1000 to (r/a) = 10000 at the end of the first day of operation if $y_0 = 200$ feet and the diameter of the well casing is six inches.

Answer:

r (feet)	$\frac{\mathbf{r}}{\mathbf{a}}$	$\frac{y}{y}_{o}$	y (feet)	r	$\frac{\mathbf{r}}{\mathbf{a}}$	$\frac{y}{y}_{o}$	y (feet)
0.25	1	1.0000	200.0	1500	6000	.009006	1.80
250	1000	0.1725	34.5	1750	7000	.004547	.91
500	2000	0.09444	18.9	2000	8000	.002178	.44
750	3000	0.05422	10.8	2250	9000	.000985	.20
1000	4000	0.03074	6.15	2500	10000	.000420	.08
1250	5000	0.01702	3.40				

(4-2) Estimate the flow of the well at the end of each of the first twelve hours of operation.

Answer:

Time (hours)	$(\frac{\sqrt{4\alpha_1 t}}{a})$	$G(\frac{\sqrt{4\alpha_1 t}}{a})$	Q (ft ³ /sec)	Time (hours)	$(\frac{\sqrt{4\alpha_1 t}}{a})$	$G(\frac{\sqrt{4\alpha_1 t}}{a})$	Q (ft ³ /sec)
0				7	2840	0.1295	0.325
1	1073	0.1480	0.372	8	3036	0.1284	0.323
2	1518	0.1408	0.354	9	3220	0.1274	0.320
3	1859	0.1370	0.344	10	3394	0.1266	0.318
4	2149	0.1343	0.338	11	3560	0.1258	0.316
5	2497	0.1317	0.331	12	3718	0.1252	0.315
6	2629	0.1308	0.329				

(4-3) Estimate the total flow of the well for the first 12 hours of operation.

Answer:

$$P = 14560 \text{ ft}^3$$

(4-4) Check the result obtained for (4-3) by use of formula 4-14.

Answer:

(4-5) Compare this total flow with what would have been produced if the well had flowed for the full twelve hours at the rate it was flowing at the end of the first hour.

Answer:

16070 ft³

Chapter 5

Well with a semi-permeable bed overlying the aquifer

A cross section through the well is shown in figure 5-1.

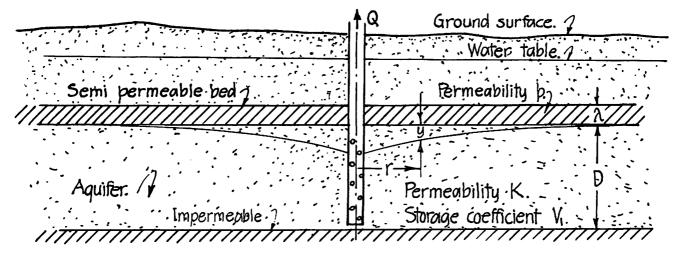


Fig. 5-1 Well drawing water from an aquifer overlain by a semi-permeable bed.

To treat this case it will be needful to modify equation 2-5 of Chapter 2 to account for the seepage of water through the semi-permeable bed. It is assumed that before pumping begins there is a hydrostatic pressure distribution which is continuous from the water table downward through the semi-permeable bed and the aquifer to the impermeable barrier. When pumping begins, withdrawal of water from the aquifer reduces the pressure at the bottom of the semi-permeable bed and causes a downward seepage of water through it. The pressure reduction at the bottom of the semi-permeable bed is represented by y . The downward flow, per unit of horizontal area is then (py/λ) since the downward gradient is (y/λ) . It is supposed that the water table is maintained at its original level above the semi-permeable bed and that movement through the semi-permeable bed is in the vertical direction only. Unwatering of the aquifer does not occur. The storage coefficient V_1 is of the artesian type since the yield of water from the aquifer is produced by expansion of the water and compression of the aquifer by the pressure reduction y . In regard

to the compression of the aquifer it may be noted that the weight of the overburden is originally supported, in part, by the hydrostatic pressure and the remainder by contact pressures between grains. When the pressure below the semi-permeable bed is reduced the load originally carried by it is transferred to the aquifer grain structure to cause it to be compressed. This compression causes the aquifer to yield water. The storage coefficient V_1 is given the subscript 1 to indicate that the yield of water is produced by pressure change and not by drainage.

The flow through a cylindrical shell of radius r and height D is, by Darcy's law $F = -2\pi r K D \frac{\partial y}{\partial r}$. The minus sign is appropriate where F is considered positive if toward the well and y is taken to be positive for a pressure reduction. The continuity condition is then:

$$-\frac{\partial}{\partial \mathbf{r}}(2\pi\mathbf{r}KD \frac{\partial \mathbf{y}}{\partial \mathbf{r}}) d\mathbf{r} dt + 2\pi\mathbf{r} \frac{\mathbf{p}\mathbf{y}}{\lambda} d\mathbf{r} dt = -2\pi\mathbf{r}V_1 \frac{\partial \mathbf{y}}{\partial t} d\mathbf{r} dt .$$

After simplification and rearrangement this becomes, with α_1 = (KD/V₁)

$$\alpha_1 \left(\frac{\partial^2 y}{\partial r^2} + \frac{1}{r} \frac{\partial y}{\partial r} \right) = \frac{\partial y}{\partial t} + \frac{py}{V_1 \lambda}$$
 (5-1)

A solution of this differential equation which meets the conditions y = 0 when t = 0 for r > 0

$$-2\pi r KD \frac{\partial y}{\partial r} \rightarrow Q$$
 as $r \rightarrow 0$ for $t > 0$

is:

$$y = \frac{Q}{2\pi KD} \int_{\frac{\mathbf{r}}{\sqrt{4\alpha t}}}^{\infty} \frac{e^{-u^2 - \frac{m^2}{u^2}} du}{u}$$
 (5-2)*

This problem has been treated by Hantush and Jacob in terms of the integral $\int_{-y}^{\infty} \frac{e^{-y-\frac{r^2}{4B^2y}}}{y} dy$. See Trans. Amer. Geophysical Union, Vol. 36, 1955. $\frac{r^2S}{4Tt}$

where

$$m = \frac{\mathbf{r}}{2} \sqrt{\frac{\mathbf{p}}{\lambda KD}}$$

Values of the integral which appear in equation (5-2) may be found in table 14.

Values outside the table

Since the table covers a finite range of values something needs to be said about the procedure to be used when a point falls outside the tabular range.

In the integrand of formula 5-2 the factor e^{-u^2} is unity when u=0 and decreases as u increases reaching the value 0.0001234 when u=3.0 the factor $e^{-\frac{m^2}{u^2}}$ is zero when u is zero and rises toward unity as u increases. The factor 1/u reaches an infinite value when u=0 and decreases toward zero as u increases. The integrand, however, always approaches zero as u approaches zero if m>0. The integral is, therefore, always finite so long as m>0. The integral from zero to infinity can be evaluated and has the value

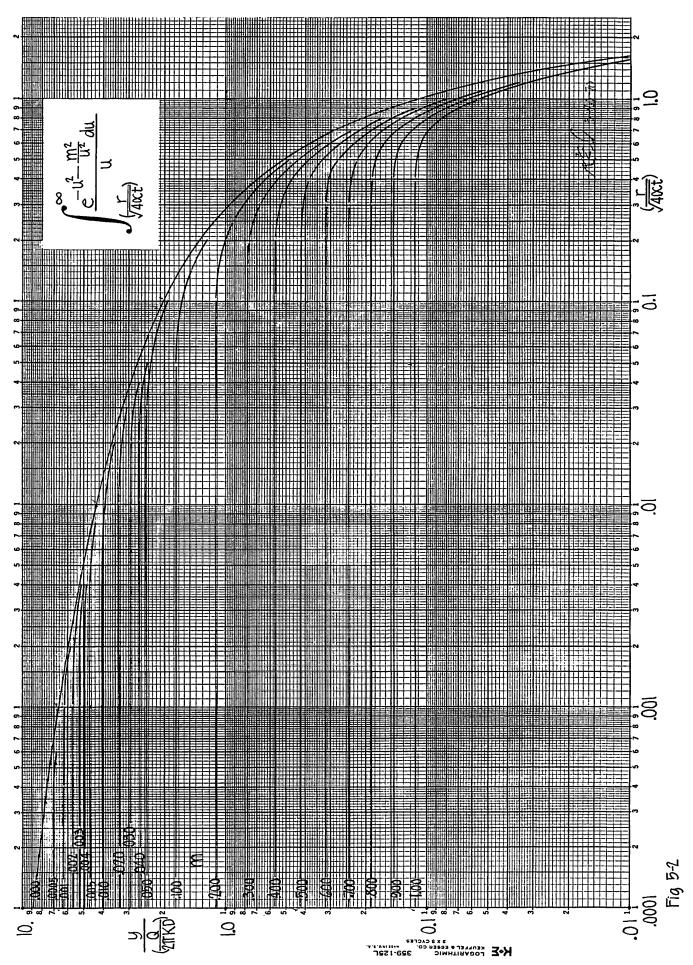
$$\int_{0}^{\infty} \frac{e^{-u^2 - \frac{m^2}{u^2}}}{u} = K_0(2m). \qquad (5-3)$$

This case has an ultimate steady state which is realized when the pressure reduction below the semi-permeable bed becomes great enough to produce a seepage through it sufficient to supply the flow of the well. When the steady state is reached $(\partial y/\partial t)$ becomes zero and the differential equation 5-1 takes the form

$$\frac{\mathrm{d}^2 y}{\mathrm{d}r^2} + \frac{1}{r} \frac{\mathrm{d}y}{\mathrm{d}r} - \frac{py}{\alpha_1 \lambda V_1} = 0. \tag{5-4}$$

Note that

$$(p/\alpha_1 \lambda V_1) = (p/\lambda KD)$$



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The substitutions:

$$\mu = \frac{y}{\left(\frac{Q}{2\pi KD}\right)}$$
 and $\rho = r\sqrt{\frac{p}{\lambda KD}}$

will reduce the above differential equation to the form:

$$\frac{d^2u}{d\rho^2} + \frac{1}{\rho} \frac{du}{d\rho} - \mu = 0 . ag{5-5}$$

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A solution, subject to the requirement that $\mu \to 0$ as $\rho \to \infty$ is:

$$\mu = K_{O}(\rho)$$

or

$$\mu = K_{\Omega}(2m)$$

or

$$y = (\frac{Q}{2\pi KD}) K_Q(2m)$$
 (5-6)

This is the ultimate steady state. The quantity K_0 is the modified Bessel's function of the second kind. It has been extensively tabulated. Approximate values can be read from the chart of figure 5-2. This result is in complete agreement with the value given for the integral in formula 5-2. A reference to figure 5-2 will show that, for a given value of r, the plots become horizontal when time increases sufficiently. This horizontal portion indicates that the ultimate steady state has been attained. The value of u corresponding to the horizontal portion of the plots will be found to be in agreement with that given by formula 5-2. It will hold this value even though the value of $(r/\sqrt{4\alpha_1 t})$ is less than the smallest value shown on figure 5-2.

There will be some values in the region where $(r/\sqrt{4\alpha_1 t}) < .001$ and m < .005, however, which cannot be treated in this way. In this region the factor e^{-u^2} is between 0.999999 and unity. To a close approximation, therefore, it will be possible to replace the integral of formula 5-2 with

$$\mu = K_0(2m) - \int_0^a \frac{e^{-\frac{m^2}{u^2}} du}{u}$$
 (5-7)

Where a represents the limit $(\frac{r}{\sqrt{4\alpha_1 t}})$. (Valid if $(\frac{r}{\sqrt{4\alpha_1 t}})$ < 0.001).

The variable substitution (m/u) = v will put this into the form

$$\mu = K_{O}(2m) - \int_{(\frac{m}{a})}^{\infty} \frac{e^{-v^{2}}dv}{v}$$

$$(Valid if m < .005.$$

$$(\frac{r}{\sqrt{4\alpha_{1}t}}) < .001).$$

As an example suppose m = 0.001, a = 0.01, $\frac{m}{a} = 0.1$, $K_0(2m) = 6.3305$

$$\int_{0.1}^{\infty} \frac{e^{-v^2} dv}{v} = 2.01896$$
 (From Table 1)

Then

$$\frac{y}{(\frac{Q}{Q})} = \mu = 6.33055 - 2.01896 = 4.31159$$

The corresponding tabular value is 4.31158.

A similar approximation can be made where the (m/u) values are small enough to make $e^{-\frac{m^2}{12}} = 1$. Here the approximation is:

$$\mu = \int_{0}^{\infty} \frac{e^{-u^2} du}{u}$$

Evaluation can be made with the help of Table 1. The open circles on figure 5-2 show approximations obtained in this way.

Example

The following example will illustrate the use of these formulas. An aquifer of 125 feet thickness having a permeability K of 0.0004 ft/sec and a storage coefficient of $V_1 = 0.0009$ is overlain by a bed of glacial till

having a thickness of 32 feet and a permeability p, for vertical flow, of 35(10)⁻⁹ ft/sec. A well penetrating the full thickness of the aquifer is pumped at the rate of 0.25 ft³/sec. It is desired to compute the pressure drawdown, as a function of the radius, at the end of 24 hours of pumping. It is also desired to estimate the final steady state drawdowns and to estimate the time required to establish this steady state.

Solution Solution

With:

$$K = 0.00040 ft/sec$$

$$D = 125 ft KD = 0.050 ft^2/sec$$

$$V_1 = 0.0009 (dimensionless) \alpha_1 = 55.556 ft^2/sec$$

$$p = (35.0)(10)^{-9} ft/sec$$

$$\lambda = 32. ft$$

$$Q = 0.250 ft^3/sec t = 86400 sec (24 hours)$$

$$(\frac{Q}{2\pi KD}) = \frac{0.250}{(6.2832)(0.050)} = 0.795$$

$$\sqrt{\frac{p}{\lambda KD}} = \sqrt{\frac{35(10)^{-9}}{(32)(.050)}} = \sqrt{(2.19)(10)^{-8}} = 0.0001480$$

The follo	wing radii will be	used:	∞2 m ²	
Radius (feet)	m	$(\frac{r}{\sqrt{4\alpha_1 t}})$	$\int_{\frac{\mathbf{r}}{\sqrt{4\alpha_1 t}}}^{\frac{-\mathbf{u}^2 - \mathbf{u}^2}{\mathbf{u}^2} d\mathbf{u}} d\mathbf{u}$	у
10	0.00074	0.002282	5.864	4.66
50	0.00370	0.011410	4.157	3.30
100	0.00740	0.022820	3.452	2.77
500	0.03700	0.114103	1.860	1.48
1000	0.07400	0.228206	1.171	0.93
5000	0.37000	1.141031	0.0623	0.05
10000	0.74000	2.282062	0.0004	0.00
$\sqrt{4\alpha_1 t} =$	$\sqrt{(4)(5\ 5.555)(864)}$	$\overline{100}$) = 4382		

Computations were made by use of Table 14 and double interpolation was used to obtain the y values. The detail of the computations for the first two radii are shown below. For m=0 the table cannot be used the these values were obtained by the methods described previously.

For
$$m = 0.00074$$
 $\frac{r}{\sqrt{4\alpha_1 t}} = 0.002282$ $m = 0.002282$ 0.003 $\mu = 5.864$ $0 = 5.9260$ 0.002282 0.003 $\mu = 5.864$ 0.0074 0.0074 0.0074 0.0074 0.0074 0.0074 0.0074 0.0075 0

The ultimate steady state will be substantially attained when η = 4 and η is given by the relation:

$$\eta = t(\frac{p}{\lambda V_1})^*$$

then

$$t = \frac{4\lambda V_1}{p} = \frac{(4)(32)(.0009)}{35(10)^{-9}} = 3291400. \text{ sec or } 38 \text{ days.}$$

An independent development is presented in Bureau of Reclamation Monograph 31. This formula is obtained from this source.

The ultimate steady state drawdowns may be computed by use of formula 5-6.

$$y = \left(\frac{Q}{2\pi KD}\right) K_{O}(2m)$$

The computation can be made as follows:

Radius (feet)	m	K _o (2m)	у
10	0.00074	6.632028	5.272
50	0.00370	5.022289	3.992
100	0.00740	4.329351	3.441
500	0.0370	2.724716	2.166
1000	0.0740	2.043074	1.624
5000	0.370	0.620173	0.492
10000	0.740	0.219434	0.174

As a second example we will estimate the pressure drawdown at the radii 10 , 100 , and 1000 feet for a succession of times. We will use readings from the graph of figure 5-2 and make the computations with a slide rule. The data for the example are as follows:

The quantities (r/\sqrt{t}) are added for purposes to be described later.

Table 5-1.

Table 5-1.							
For	r = 10 feet		m	$m = \frac{10}{2} (0.5000) (10)^{-3} = 0.00250$			
T	ime	Time (seconds)	$(\frac{\mathbf{r}}{\sqrt{4\alpha_1 t}})$	$\frac{y^*}{(\frac{Q}{2\pi KD})}$	y (feet)	$(\frac{\mathbf{r}}{\sqrt{\mathbf{t}}})$	
1	min	60	0.09128	2.10	3.21	1.290	
10	min	600	0.02887	3.28	5.01	0.408	
1	hr	3600	0.01178	4.18	6.38	0.167	
6	hr	21600	0.00482	5.02	7.66	0.068	
.12	hr	43200	0.00340	5.20	7.94	0.048	
24	hr	86400	0.00240	5.40	8.25	0.034	
5	days	432000	0.00108	5.46	8.33	0.015	
10	days	864000	0.00076	5.50	8.40	0.011	
For $r = 100$ feet $m = 0.0250$							
T	ime	Time (seconds)	$(\frac{\mathbf{r}}{\sqrt{4\alpha_1 t}})$	$(\frac{y^*}{\frac{Q}{2\pi KD}})$	y (feet)	$(\frac{\mathbf{r}}{\sqrt{t}})$	
1	min	60	0.9128	0.150	0.229	12.90	
10	min	600	0.2889	1.00	1.528	4.08	
1	hr	3600	0.1178	1.87	2.86	1.67	
6	hr	21600	0.0482	2.70	4.13	0.680	
12	hr	43200	0.0340	2.90	4.44	0.481	
24	hr	86400	0.0240	3.10	4.74	0.340	
5	days	432000	0.0108	3.30	5.04	0.152	
10	days	864000	0.0076	3.30	5.04	0.108	
For	For $r = 1000$ feet $m = 0.250$						
T	ime	Time (seconds)	$(\frac{\mathbf{r}}{\sqrt{4\alpha_1 t}})$	$\frac{y^*}{(\frac{Q}{2\pi KD})}$	y (feet)	$(\frac{\mathbf{r}}{\sqrt{t}})$	
1	min	60	9.128				
10	min	600	2.887				
1	hr	3600	1.178	0.060	0.092	16.67	
6	hr	21600	0.482	0.470	0.718	6.680	
12	hr	43200	0.340	0.710	1.085	4.82	
24	hr	86400	0.240	0.830	1.270	3.40	
5	days	432000	0.108	0.920	1.408	1.52	
10	days	864000	0.076	0.920	1.408	1.08	

^{*}Read from the chart of Fig. 5-2.

Determination of aquifer properties

Aquifer properties can be determined from test data by a curve matching procedure such as has been previously described. For this purpose the data of Table 5-1 are plotted on a chart with logarithmic scales for both ordinate and abscissa. Such a chart is shown on figure 5-3. Observed pressure drawdowns y from test data are then plotted against corresponding values of (r/\sqrt{t}) on an identical grid. This second chart is then placed over the master chart of figure 5-2 and adjusted, while keeping the axes parallel, until a fit is obtained. The position of the index of the master chart is then marked on the test data chart. The aquifer properties can then be obtained by the process described in the following example.

It will be advantageous to use the computed drawdowns of the second example as test data because we will then know that results should be obtained and thereby gain some insight as to the effectiveness of the method. Plots of y versus (r/\sqrt{t}) are shown on figure 5-3. When this is superposed on the master chart and adjusted to a satisfactory fit the position of the master chart index is indicated by a cross.

For the master chart index

$$\frac{y}{\left(\frac{Q}{2\pi KD}\right)} = 1.00 \qquad \left(\frac{r}{\sqrt{4\alpha_1 t}}\right) = 1.00$$

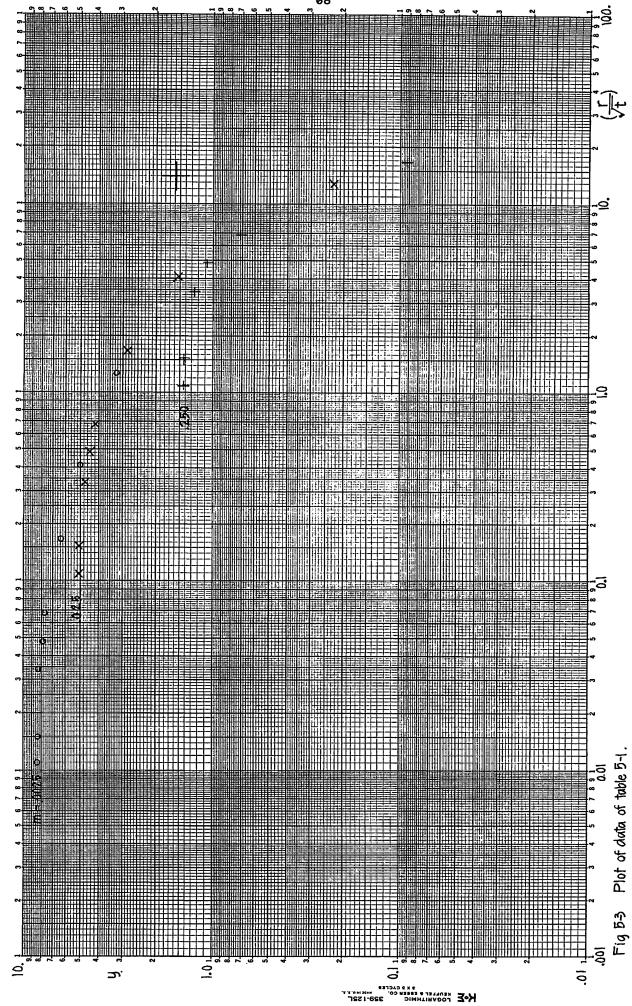
For the test data chart index

$$y = 1.58$$
 $(\frac{r}{\sqrt{t}}) = 14.0$

From the first of these

KD =
$$\frac{(1.00)(Q)}{2 \pi y}$$
 = $\frac{(1.00)(0.24)}{(6.2832)(1.58)}$ = 0.0242 (Should be 0.025)
K = $(.0242)/(40)$ = 0.000605 (Should be 0.000625)





From the second set of values

$$\sqrt{4\alpha_1} = (\frac{r}{\sqrt{t}})$$
. Then, $\sqrt{4\alpha_1} = 14.0$
 $\alpha_1 = \frac{14.0^2}{4} = 49.0$ (Should be 50)

For r = 100 ft

$$m = 0.0250$$

$$m = \frac{r}{2} \sqrt{\frac{p}{\lambda KD}}$$
 or $\frac{p}{\lambda} = \frac{4m^2 KD}{r^2}$

then

$$\frac{p}{\lambda} = \frac{(4)(000625)(.0243)}{(10000)} = 6.075(10)^{-9}$$

$$p = (6.075)(10)^{-9}(20) = 121.5(10)^{-9} \text{ (Should be } 125.(10)^{-9} \text{)}.$$

Then all of the aquifer properties have been recovered to an approximation which is close enough for engineering purposes.

Comments

Because there is generally no way to maintain the water table level in the upper bed, drawdowns must occur there to supply the leakage through the semi-permeable bed and it is a fair question, therefore, as to how long the solution described can maintain its validity. In actual applications the solution has worked very well. The explanation seems to be that the pressure drawdowns spread rapidly due to the artesian type values of V_1 and the flow of the well is then drawn from a large area and the drawdown of the water table is very slow. Good correlation of test results is the rule and it will be found that when a semi-permeable bed overlies the aquifer this development will produce correlations while formulas ignoring the presence of the semi-permeable member will not produce correlation.

Applications can be made to situations where the upper member has a low permeability as compared to the lower member and the water table is in the upper member. In such cases λ can represent the saturated thickness of that part of the upper member which is below the water table.

Chapter 5

Problems

(5-1) A permeable water bearing sand having a thickness of 40 feet is overlain by a glacial till having a saturated thickness of 20 feet. A well is sunk to the bottom of the water bearing sand and gravel packed to an effective diameter of two feet. The aquifer properties are:

$$K = 0.002 \text{ ft/sec}$$
 $KD = 0.080 \text{ ft}^2/\text{sec}$ $V_1 = 0.0005$
 $p = 180 \times 10^{-9} \text{ ft/sec}$

If the well is pumped at the rate of 0.250 $\rm ft^3/sec$ what will be the pressure drawdown y below the semi-permeable till bed, after 24 hours of pumping, at the radii $\, r = 1.0 \, \rm ft$, 10 ft, 100 ft and 1000 ft? Answer:

radius (feet)	y (feet)
1.0	3.98
10.0	2.81
100.0	1.72
1000.0	0.61

(Suggestion: for m = 0 revert to Table 1)

(5-2) What will be the ultimate steady state drawdown?

Answer: From formula 5-6

radius (feet)	y (feet)
1.0	4.04
10.0	2.89
100.0	1.75
1000.0	0.63

(5-3) Approximately how long will it take to establish the ultimate steady state?

Answer: About 5.7 days, based upon the point from Table 14 identified by $m = 0.200 \frac{r}{\sqrt{4\alpha t}} = 0.06$. This relation is solved for t using r = 1000 ft. This value must be accepted as an estimate because the point when stability is reached is difficult to recognize.

Chapter 6

Bank storage

This chapter will be devoted to consideration of those cases where flow is in one direction only. The first case to be treated will be that of a reservoir, with permeable banks, which has remained filled for a long time and is then drawn down as shown in the figure below. It is desired to determine the configuration of the water table in the banks for any time subsequent to the drawdown and to estimate the rate of return flow from them and the total amount of the return flow. To illustrate the various possible approaches this case will be attacked by several methods and a final comparison of results will be made. As a first approach a solution of the differential equation 2-2

$$\alpha \frac{\partial^2 h_1}{\partial x^2} = \frac{\partial h_1}{\partial t}$$

Subject to the requirements

$$h_1 = H_0$$
 for $x > 0$ when $t = 0$
 $h_1 = 0$ for $x = 0$ for $t > 0$

can be sought. The required solution is:

$$h_1 = H_0 \frac{2}{\sqrt{\pi}} \int_0^{(\frac{x}{\sqrt{4\alpha t}})} e^{-u^2} du$$
 (6-1)

The flow of ground water at x = 0 is:

$$F = + KD \left(\frac{\partial h_1}{\partial x}\right)_{x=0}$$

But since, as will be shown later,

$$\frac{\partial h_1}{\partial x} = H_0 \frac{2}{\sqrt{\pi}} \frac{e^{-(\frac{x^2}{4\alpha t})}}{\sqrt{4\alpha t}}$$

And

$$\left(\frac{\partial h_1}{\partial x}\right)_{x=0} = H_0 \frac{2}{\sqrt{\pi}} \frac{1}{\sqrt{4\alpha t}}$$

Then

$$F = \frac{2 H_0 KD}{\sqrt{4\pi\alpha t}}$$
 (6-2)

The total amount of return flow is obtained by integrating this expression with respect to t from 0 to t. The result is:

$$q_{o} = H_{o} V \sqrt{\frac{4\alpha t}{\pi}}$$
 (6-3)

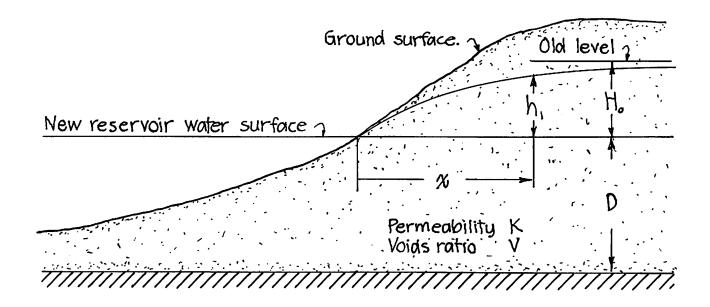


Fig. 6-1 Bank storage.

Differentiation of the integral

In order to check the validity of the expression for h_1 and to compute the derivatives needed for flow determination it is necessary to differentiate the integral which appears in the right hand member of the solution. To find the derivative let the integral be represented by the area under the curve shown below

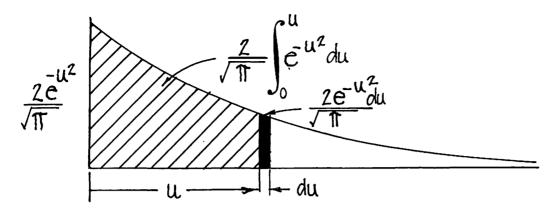


Fig. 6-2 Integral and derivative.

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$$I = \frac{2}{\sqrt{\pi}} \int_0^u e^{-u^2} du$$

is represented by the singly shaded area then the increment of $\, \, I \,$ as $\, \, u \,$ increases from $\, u \,$ to $\, u \,$ + du $\,$ is

$$dI = \frac{2 e^{-u^2}}{\sqrt{\pi}} du$$

or,

$$\frac{\mathrm{dI}}{\mathrm{du}} = \frac{2 \, \mathrm{e}^{-\mathrm{u}^2}}{\sqrt{\pi}} \tag{6-4}$$

The upper limit of the integral is:

$$u_1 = (\frac{x}{\sqrt{4\alpha t}})$$

To get $\frac{\partial h_1}{\partial x}$ we note that:

$$\frac{\partial h_1}{\partial x} = \frac{\partial h_1}{\partial u} \frac{\partial u}{\partial x}$$

Then

$$\frac{\partial h_1}{\partial x} = H_0 \frac{\partial I}{\partial u} \frac{\partial u}{\partial x} = H_0 \frac{2}{\sqrt{\pi}} \frac{e^{-(\frac{x^2}{4\alpha t})}}{\sqrt{4\alpha t}}$$
 (6-5)

This is the derivative used for estimating the flow.

Check of the validity of the solution

By differentiation with respect to x

$$\frac{\partial^2 h_1}{\partial x^2} = -H_0 \frac{2 e^{-\left(\frac{x^2}{4\alpha t}\right)}}{\sqrt{\pi} \sqrt{4\alpha t}} \frac{2x}{4\alpha t} = \frac{-H_0 4e^{-\left(\frac{x^2}{4\alpha t}\right)}}{\sqrt{\pi} (4\alpha t)^{\frac{3}{2}}}$$

In the above work the partial derivative of $\, u \,$ with respect to $\, x \,$ has been evaluated as:

$$\frac{\partial u_1}{\partial x} = \frac{1}{\sqrt{4\alpha + 1}}$$

In a similar manner

$$\frac{\partial h_1}{\partial t} = H_0 \frac{\partial I}{\partial u} \frac{\partial u}{\partial t} = H_0 \frac{2 \frac{e^{-(\frac{x^2}{4\alpha t})}}{\sqrt{\pi}}}{\sqrt{\pi}} \frac{\partial u}{\partial t} = -H_0 \frac{2}{\sqrt{\pi}} \frac{e^{-(\frac{x^2}{4\alpha t})}}{2(4\alpha t)^{\frac{3}{2}}} = -H_0 \frac{4\alpha}{\sqrt{\pi}} \frac{e^{-(\frac{x^2}{4\alpha t})}}{(4\alpha t)^{\frac{3}{2}}}$$

Where

$$\frac{\partial u}{\partial t} = \frac{-x4\alpha}{2(4\alpha t)^{3/2}}$$

Then $\alpha \frac{\partial^2 h_1}{\partial x^2} = \frac{\partial h_1}{\partial t}$ and the differential equation is satisfied.

For the first boundary condition

$$h_1 = H_0$$
 for $x > 0$ when $t = 0$
 $h_1 \rightarrow H_0 \frac{2}{\sqrt{\pi}} \int_0^\infty e^{-u^2} du = H_0$ when $x \rightarrow \infty$.

Since

$$\frac{2}{\sqrt{\pi}} \int_{0}^{\infty} e^{-u^{2}} du = 1.00$$

For the second condition

$$h_1 = 0$$
 for $x = 0$ for $t > 0$

We have

$$h_1 = H_0 \frac{2}{\sqrt{\pi}} \int_0^0 e^{-u^2} du = 0$$

Then the expression satisfies both the differential equation, and the boundary and initial conditions. The solution we have is therefore unique (Cohen, 1933). This means that although we might possibly find another solution differing outwardly in form from this one it must, nevertheless, yield the same numerical values as this one for a specified value of the parameter $(\frac{X}{\sqrt{4\alpha t}})$.

Application of Werner's idealization

A solution of Werner's differential equation 2-3 satisfying the condition:

$$h_2 = H_0$$
 for $x > 0$ when $t = 0$
 $h_2 = 0$ for $x = 0$ for $t > 0$

is

$$u = H_0^2 \frac{2}{\sqrt{\pi}} \int_0^{\frac{x}{\sqrt{4\alpha t}}} e^{-u^2} du$$

Then since $u = h_2^2$

$$h_2 = H_0 \sqrt{\frac{2}{\sqrt{\pi}} \int_0^{\sqrt{4\alpha t}} e^{-u^2} du}$$
 (6-6)

This solution is of limited application because the condition $h_2 = 0$ for x = 0 is one of complete drawdown at x = 0. Treatment of lesser drawdowns runs into the difficulty of meeting the boundary condition:

$$h_2 = constant for x = 0 for t > 0.$$

For the complete drawdown case, however, the solution obtained is a close approximation if $\alpha_W = \frac{K \ H_O}{2V}$. From this idealization we have obtained a treatment of the extreme case where the water table is drawn down to the barrier at the origin. This case can be considered to be out of reach for the previous development since the drawdown is not small when compared to the original saturated depth.

It will be of interest to compute the rate of ground water flow at the origin.

The flow at x at the time t is:

$$f = K h_2 \left(\frac{\partial h_2}{\partial x}\right)_{x = 0}$$

But

$$\frac{\partial h_2}{\partial x} = \frac{\frac{2}{\sqrt{\pi}} \frac{e^{-\left(\frac{x^2}{4\alpha t}\right)}}{\sqrt{4\alpha t}}}{2\sqrt{\sqrt{\pi}} \int_{0}^{\frac{x}{\sqrt{4\alpha t}}} e^{-u^2} du}$$

Then

$$f = \frac{KH_0^2 \frac{2}{\sqrt{\pi}} \frac{e^{-(\frac{x^2}{4\alpha t})}}{e^{-u^2}du} \frac{2}{\sqrt{\pi}} \int_0^{(\frac{x}{\sqrt{4\alpha t}})} e^{-u^2}du}{2\sqrt{\frac{2}{\sqrt{\pi}}} \int_0^{(\frac{x}{\sqrt{4\alpha t}})} e^{-u^2}du}$$

or

$$f = \frac{K H_0^2 e^{-\frac{x^2}{4\alpha t}}}{\sqrt{\pi} \sqrt{4\alpha t}}$$
 (6-7)

As x approaches zero the flow approaches

$$F_2 = \frac{K H_o^2}{\sqrt{4\pi\alpha_w t}}$$
 (6-8)

A comparison with the previous result will show that this flow is one-half what would have been computed on the basis of the first development but the variation with respect to time remains the same.

Developments of Haushild and Kruse (1962)

These authors employed two methods for extending the range of the drawdown conditions which could be treated. A second approximation development was made by the method of Picard (Agnew, 1942) but a more satisfactory treatment was obtained by utilizing physical concepts such as the following: The flows obtained from the first approximation 6-1 must be a good approximation to the true flows because it is only near the origin that drawdowns become large enough to make the validity of the first approximation questionable. Even in this area it can be expected that gross discrepancies will be absent because the solution must show the right drawdown at the origin where the drawdown is a maximum. Then it is to be expected that a much improved approximation could be obtained if the water table profile were computed on the basis of the first approximation flows but with the true saturated thickness accounted for. These concepts lead to the following formulation

$$K (d+h_3) \frac{\partial h_3}{\partial x} = KD \frac{\partial h_1}{\partial x}$$

Where d represents the saturated depth at the new water surface level. In this expression the right hand member represents the flow, as obtained from the first approximation. The left hand member represents the flow as computed from the Dupuit-Forchheimer idealization in its nonlinear form.

$$\frac{(d + h_3)^2}{2} = D h_1 + C_1$$

Ιf

By integration:

$$h_3 = 0 \quad \text{when} \quad x = 0$$

$$C_1 = \frac{d^2}{2}$$

Then

$$(d + h_3)^2 = d^2 + 2 Dh_1$$

or, after rearrangement:

$$h_3 = \sqrt{2 D h_1 + d^2} - d. \tag{6-9}$$

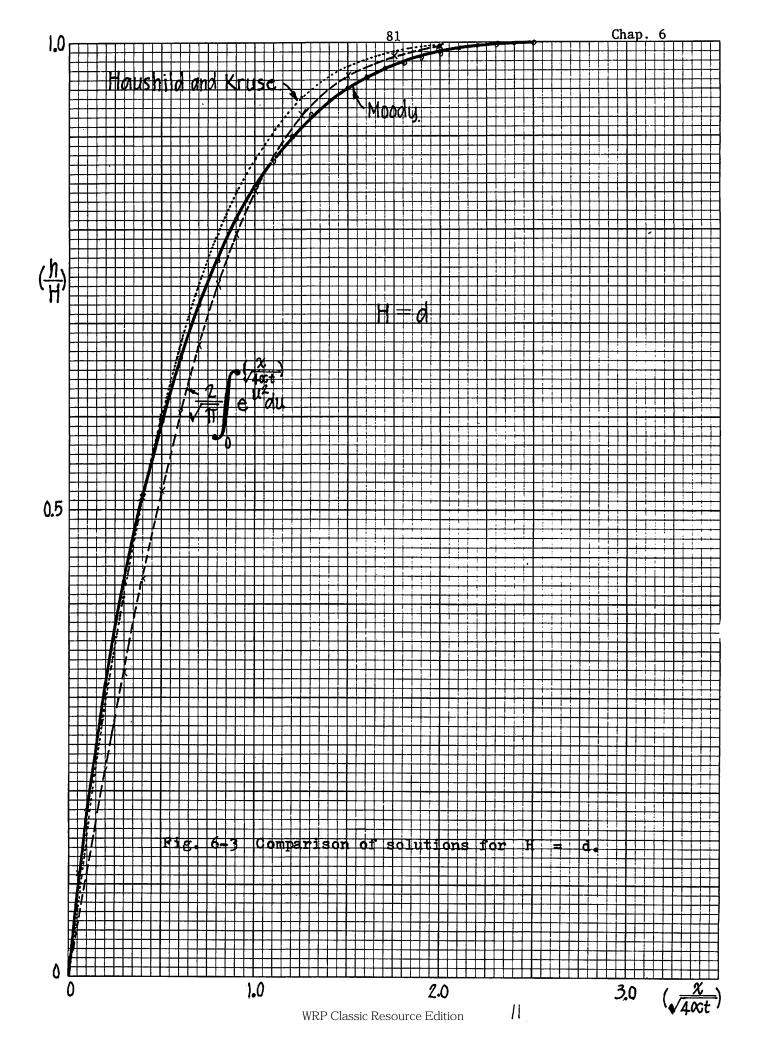
In their work they use:

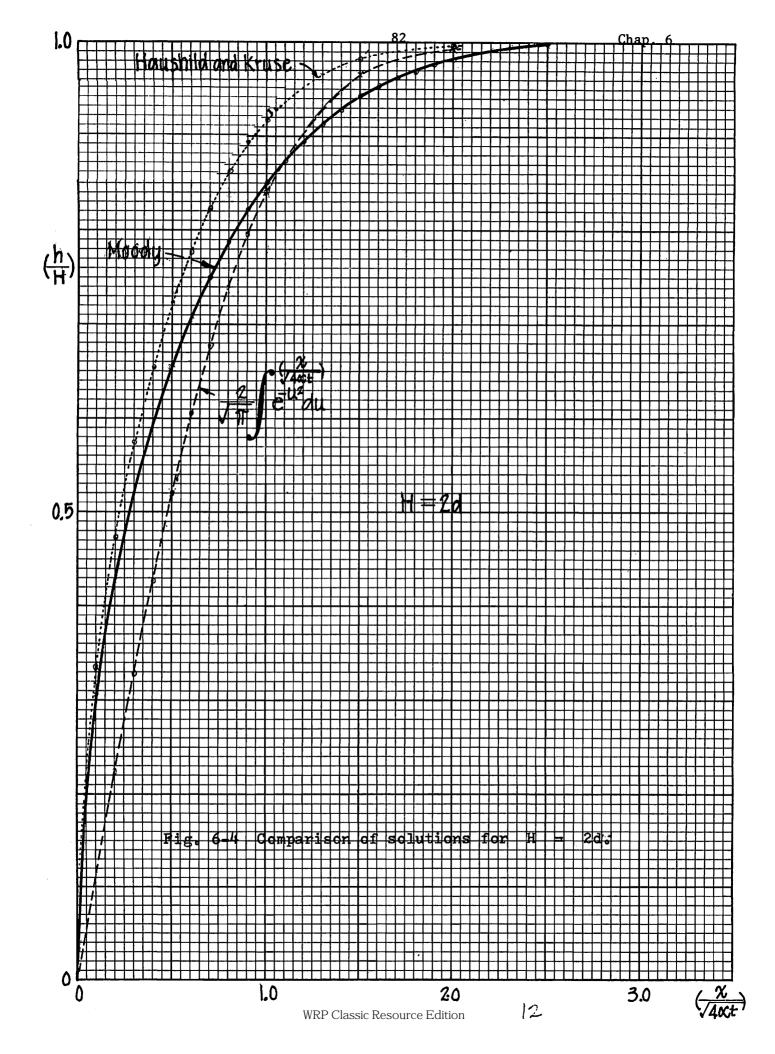
$$D_1 = (d + \frac{H_0}{2}) \tag{6-10}$$

The development of Moody

In a discussion of Paper 3317 by Haushild and Kruse, ASCE Transactions, Vol. 127, Part I, 1962, Mr. W. T. Moody developed a solution of the nonlinear partial differential equation of ground water flow from bank storage when the Dupuit-Forchheimer assumptions hold. He followed the general method described by J. Crank, in "The Mathematics of Diffusion," Oxford, 1957, pages 149-152. An iterative procedure was used to yield an essentially exact solution of the differential equation:

$$\alpha \frac{\partial^2 h}{\partial x^2} + \frac{\alpha}{D} (\frac{\partial h}{\partial x})^2 = \frac{\partial h}{\partial t} - \frac{\alpha h}{D} \frac{\partial^2 h}{\partial x^2}$$





This is Haushild and Kruse's equation (6). The solution thus developed is included on figures 6-3 and 6-4 for comparison purposes.

Comparisons

When drawdown is complete, d=0, and it will be found that the Werner and Haushild and Kruse solutions become identical. Furthermore with the Haushild and Kruse choice of $D_1=d+\frac{H_0}{2}$, when d=0, reduces to $D_1=\frac{H_0}{2}$. then the flow computed by formula 6-2 becomes identical with that obtained from the Werner idealization. No choice has yet been made for the value of D to use in the solution obtained from the Werner linearization but it would be reasonable to make the same choice as Haushild and Kruse. If this is done these developments are brought into complete accord. Haushild and Kruse checked their formula for complete drawdown against laboratory test data and found a very good agreement. Analytical developments and laboratory test data are then brought into harmony.

Comparisons with Moody's results are shown on figures 6-3 and 6-4. It is surprising how well the first approximation solution holds up even when applied to drawdowns which could well be considered excessive as judged from the approximations introduced to linearize the differential equation from which it was derived.

Values of the function

$$\frac{2}{\sqrt{\pi}} \int_{0}^{x} e^{-u^2} du$$

can be found in Table 8.

Chapter 6

Problems

(6-1) A reservoir built in a sandy area has an average length of 20,000 feet, has been filled for some time, and is then drawn down 10 feet. What will be the rate of return flow from 40,000 feet of bank, at the end of the first month following drawdown, if the aquifer properties are?

KD = 3,000,000 ft²/year V = 0.15

$$\alpha$$
 = 20,000,000 ft²/year

Answer: 16.6 ft³/sec.

(6-2) What will be the total return when one month has elapsed since drawdown? Answer: 2006 acre feet.

(6-3) If the water level in the reservoir stands, on the average, 40 feet higher than it did before the reservoir was built what will be the leakage rate five years after construction?

Answer: 8.59 ft³/sec.

(6-4) What would be the accumulated leakage loss at this time?

Answer: 62165 acre feet.

(6-5) If a severe drought caused the reservoir to be emptied, after this ground water storage had been accumulated, would a part then return to the reservoir to supplement the flow obtainable from surface storage?

Answer: Yes.

Chapter 7

Line source

Canal leakage penetrating to the water table can raise a ground water ridge from which ground water can flow both ways as shown in the figure below

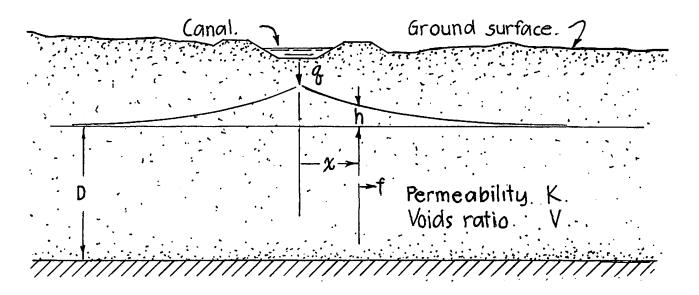


Fig. 7-1 Line source

If it is assumed that conditions are sufficiently uniform to cause a parallel flow the basic differential equation to be satisfied for a first approximation treatment is:

$$\alpha \frac{\partial^2 h}{\partial x^2} = \frac{\partial h}{\partial t}$$

This is to be solved subject to the boundary conditions

q = 2 f at x = 0 for t > 0

h = 0 when t = 0 for x > 0

A solution satisfying the differential equation and the boundary conditions is:

$$h = \frac{q\sqrt{4\pi\alpha t}}{2\pi KD} \left(\frac{x}{\sqrt{4\alpha t}}\right) \int_{0}^{\infty} \frac{e^{-u^{2}} du}{u^{2}}$$

$$\left(\frac{x}{\sqrt{4\alpha t}}\right)$$
(7-1)

Values of this integral may be obtained from Table 9. At x = 0 this becomes:

$$(h)_{o} = \frac{q\sqrt{4\pi\alpha t}}{2\pi KD}$$
 (7-2)

It will be of interest to compute the flow f passing between planes a unit distance apart. This flow is, to a first approximation:

$$f = -KD \frac{\partial h}{\partial x} = -\frac{q\sqrt{\pi}}{2\pi} \left[-\frac{\sqrt{4\alpha t} e^{-\frac{x^2}{4\alpha t}}}{x} + \int_{-\frac{x^2}{\sqrt{4\alpha t}}}^{\infty} \frac{e^{-u^2} du}{u^2} \right]$$

To obtain this result the procedure for differentiating an integral has been followed. This was described previously. In the present case, however, the variation is in the lower limit which introduces a negative sign. An evaluation of the integral is needed. To obtain this integrate by parts with

$$u_1 = e^{-u^2}$$
 $dv_1 = \frac{du}{u^2}$
 $du_1 = -2ue^{-u^2}du$
 $v_1 = -\frac{1}{u}$

Then since

$$\int u_1 dv_1 = u_1 v_1 - \int v_1 du_1$$

$$\int_{0}^{\infty} \frac{e^{-u^{2}} du}{u^{2}} = -\frac{e^{-u^{2}}}{u} - \int_{0}^{\infty} \frac{2ue^{-u^{2}} du}{u} \Big]_{0}^{\infty}$$

$$= \frac{\sqrt{4\alpha t}}{x} e^{-(\frac{x^{2}}{4\alpha t})} - \sqrt{\pi} + \sqrt{\pi} \frac{2}{\sqrt{\pi}} \int_{0}^{(\frac{x}{\sqrt{4\alpha t}})} e^{-u^{2}} du$$

In obtaining this result use has been made of the relation

$$\frac{2}{\sqrt{\pi}} \int_{0}^{\infty} e^{-u^2} du = 1.0$$

Then by substitution

$$f = -\frac{q\sqrt{\pi}}{2\pi} \left[-\frac{\sqrt{4\alpha t} e^{-(\frac{x^2}{4\alpha t})}}{x} + \frac{\sqrt{4\alpha t} e^{-(\frac{x^2}{4\alpha t})}}{x} - \sqrt{\pi} + \sqrt{\pi} \frac{2}{\sqrt{\pi}} \int_{0}^{(\frac{x}{\sqrt{4\alpha t}})} e^{-u^2} du \right]$$

or
$$f = \frac{q}{2} \left[1 - \frac{2}{\sqrt{\pi}} \int_{0}^{\left(\frac{x}{\sqrt{4\alpha t}}\right)} e^{-u^{2}} du \right]$$
(7-3)

Values of the integral shown here may be obtained from Table 8. When $x \to 0$ $f \to \frac{q}{2}$ as it should. It will appear later that a formula of this general type will hold for a point source, a line source of finite length or a line source of infinite length.

Chapter 7

Problems

(7-1) A canal constructed in alluvial sediments in a river valley leaks at the rate of one cubic foot per second per mile of length. If water is run in the canal during an irrigating season lasting six months how high a mound will the leakage create under the canal?

Aquifer properties are: K = 0.0040 ft/sec D = 60 ft.

$$KD = 0.240 \text{ ft}^2/\text{sec}$$
 $V = 0.160 \quad \alpha = 1.50 \text{ ft}^2/\text{sec}$

Answer: 2.16 feet.

(7-2) What rise of the water table is to be expected, at this time, at a distance of one quarter of a mile from the canal?

Answer: 1.68 feet.

(7-3) What will be the ground water flow, per mile of canal, at the quarter mile distance at this time?

Answer: 0.424 cubic feet per second per mile.

Chapter 8

Parallel drains

In areas where natural drainage is inadequate, irrigation will cause the water table to rise progressively until the land becomes water logged. To improve the drainage, parallel drains can be installed. These may take the form of drainage canals or of tile drains laid in a trench and back filled. The latter arrangement has the advantage that the installation of drains does not take any land out of production.

First approximation solution

A solution of the differential equation 2-2

$$\alpha \frac{\partial^2 h}{\partial x^2} = \frac{\partial h}{\partial t}$$

Subject to the conditions

h = 0 when x = 0 for t > 0

h = 0 when x = L for t > 0

h = H when t = 0 for 0 < x < L

is

$$h = \frac{4H}{\pi} \sum_{n=1,3,5,...} \frac{e^{-n^2\pi^2(\frac{\alpha t}{L^2})}}{e^{-n}} \sin(\frac{n\pi x}{L})$$

$$(8-1)$$

A cross section normal to the drains is shown in figure 8-1. When $x=\frac{L}{2}$ this expression takes the form

$$h_{c} = \frac{4H}{\pi} \sum_{n=1,3,5...}^{n=\infty} \frac{e^{-n^{2}\pi^{2}(\frac{\alpha t}{L^{2}})}}{e^{-n^{2}\pi^{2}(\frac{\alpha t}{L^{2}})}} \sin(\frac{n\pi}{2})$$
 (8-2)

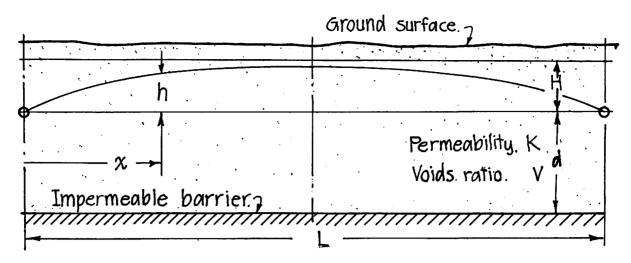


Fig. 8-1 Cross section normal to the line of drains.

Values of this function are listed in Table 10. These values find use in the technically important task of selecting a drain spacing to fit a specified set of field conditions. This is because the most difficult point to drain is midway between the drains. If this point can be drained then every other point will be drained also.

The flow to a drain from one side is

$$Kd \left(\frac{\partial h}{\partial x}\right)_{x=0} = \frac{4KdH}{L} \sum_{n=1,3,5...}^{n=\infty} e^{-n^2 \pi^2 \left(\frac{\alpha \tau}{L^2}\right)}$$
(8-3)

It may be noted in passing that this function has a singularity at t = 0. This value must be disregarded as the infinite gradient obtained from the above formula conflicts with the requirement that, for validity, the gradient must be small compared to unity. It will be shown later that there is a local resistance due to the convergence of the flow approaching a tile drain which limits the flow rate to a finite value.

Another quantity of importance is the fractional part of the drainable volume remaining at the time $\,t\,$. This is obtained from the relation

$$p = \frac{1}{HL} \int_{0}^{L} h \, dx = \frac{8}{\pi^{2}} \sum_{n=1,3,5...}^{n=\infty} \frac{e^{-n^{2}\pi^{2}(\frac{\alpha t}{L^{2}})}}{e^{n^{2}}}$$
(8-4)

Values of p are obtainable from Table 11.

Application of Werner's method

The first approximation solution is also a solution of Werner's differential equation. The boundary and initial conditions are also appropriate but since h_2 is measured from the barrier the case represented is one where the drain is on the barrier. Then

$$\frac{h_2}{H} = \sqrt{\frac{4}{\pi}} \sum_{n=1,3,5...}^{n=\infty} \frac{e^{-n^2 \pi^2 (\frac{\alpha t}{L})}}{e^{n}} \sin(\frac{n\pi x}{L})$$
 (8-5)

The development of Boussinesq

A transient state drainage treatment has been contributed by J. Boussinesq (Boussinesq 1904). He used concepts very similar to those employed by Dupuit in that he assumed the surface gradient to apply throughout the saturated depth. In our notation the condition of continuity would take the form

$$\frac{\partial}{\partial x} \left(Kh_2 \frac{\partial h_2}{\partial x} \right) = V \frac{\partial h_2}{\partial t}$$
 (8-6)

The development applies where the drains are on the barrier at a distance L apart. The distance x is measured from one of the drains toward the other. The drainable depth h_2 has the value H at $x = \frac{L}{2}$ when the time t = 0. Let

$$U = \frac{h_2}{H}$$

$$\xi = \frac{x}{L}$$

$$\eta = \frac{KH}{VL^2} t$$

Then the differential equation takes the form

$$\frac{\partial}{\partial \xi} \left(U \frac{\partial U}{\partial \xi} \right) = \frac{\partial U}{\partial \eta} \tag{8-7}$$

A possible type of solution is

$$U = WY$$

Where W is a function of ξ only and Y is a function of η only. Substitution of this product into the differential equation permits a separation of the variables and yields two ordinary differential equations one in W and the other in Y. In the case of Y the relation is

$$\frac{1}{Y^2} \frac{dY}{d\eta} = -C$$

And a solution satisfying the conditions Y = 1 when $\eta = 0$ is

$$Y = \frac{1}{C\eta + 1}$$

The differential equation for W is of the nonlinear form

$$\frac{d^2W}{d\xi^2} + \frac{1}{W} \left(\frac{dW}{d\xi}\right)^2 = -C$$

This can be reduced to a first order differential equation by the substitutions

$$\frac{dW}{d\xi} = p \qquad \qquad \frac{d^2W}{d\xi^2} = p \quad \frac{dp}{dW}$$

After substitution the above differential equation becomes

$$p \frac{dp}{dW} + \frac{p^2}{W} = -C$$

Where it may be noted that W has now become the independent variable. A further substitution

$$v = p^2 \frac{dv}{dW} = 2p \frac{dp}{dW}$$

reduces it to the linear form

$$\frac{dv}{dW} + \frac{2v}{W} = -2C$$

A solution is

$$vW^2 = -\frac{2CW^3}{3} + C_2$$

or

$$p^2 = -\frac{2CW}{3} + \frac{C_2}{W^2}$$

If p = 0 when W = 1 $C_2 = \frac{2C}{3}$ and

$$\frac{W}{\sqrt{1-W^3}} \quad \frac{dW}{d\xi} = \sqrt{\frac{2C}{3}}$$

By integration subject to the condition that W=0 when $\xi=0$.

$$\int_{0}^{W} \frac{WdW}{\sqrt{1-W^{3}}} = \sqrt{\frac{2C}{3}} \, \xi \tag{8-8}$$

When $\xi = \frac{1}{2}$ W = 1 then

$$\int_0^1 \frac{\text{WdW}}{\sqrt{1 - \text{W}^3}} = \sqrt{\frac{\text{C}}{6}}$$

The integral of this relation can be evaluated with the aid of the Beta and Gamma functions (Osgood 1933, p. 485). The evaluation is

$$\int_{0}^{1} \frac{WdW}{\sqrt{1-W^{3}}} = \frac{\Gamma(\frac{1}{2}) \Gamma(\frac{2}{3})}{3\Gamma(\frac{7}{6})} = 0.86237$$

then

C = (6)
$$(0.86237)^2 = 4.46209$$
 $\sqrt{\frac{2C}{3}} = 1.72474$

The following table is reproduced through the courtesy of Mr. W. T. Moody.

ξ	W
0.0	0.0
0.005	0.412
0.10	0.575
0.15	0.692
0.20	0.782
0.25	0.853
0.30	0.908
0.35	0.949
0.40	0.978
0.45	0.994
0.50	1.000

Finally

$$\frac{h}{H} = \frac{W}{4.46 \left(\frac{KHt}{VL^2}\right) + 1}$$
 (8-9)

This development is of value because it gives indications as to what drainage performance is to be expected when the drains are near the barrier. The first approximation solution gives little guidance in such cases. It may be noted that the initial condition of a uniform drainable depth is not met by the Boussinesq development. It is interesting to note also that the pattern of decrease is not of a descending exponential type but here takes an algebraic form.

The Method of Brooks

A second approximation solution which remains valid when the drainable depth is not negligibly small compared to the saturated depth below the drains was obtained by Brooks 1963 by application of the Pioncare, Lighthill, Kuo method. Good results were obtained where drainable depth were as great as the saturated depth below the drains (H/d=1.0). He also developed a second approximation of the type described by Haushild and Kruse, 1962. This formula can be expressed in our notation as

$$h_1 = -D_a + \sqrt{D_a^2 + 2D_a h_0 + (\frac{H}{2})^2}$$
 (8-10)

Where $D_a = (d + \frac{H}{2})$ and h_0 comes from the first approximation. It is obtained by computing the drainable depths on a nonlinear basis based upon the flows obtained from the first approximation. The original paper may need to be consulted. His origin is placed midway of the original drainable depth.

The Method of Dumm, Tapp and Moody

This procedure was developed at the U.S. Bureau of Reclamation* to provide an orderly approach to the problem of determining drain spacings. It was recognized that there would be applications where the drainable depth would not be small when compared to the barrier depth. It was also understood that where the drainable depth is not small, in the above sense, the basic differential equation would be nonlinear in form which would mean that the principle of superposition would not apply since the sum of two solutions is then not a solution. As a consequence of this concept the superposition of uniform increments of drainable depth originating in uniform irrigations was abandoned in favor of a drainage pattern representing observed configurations after a number of irrigations had been made. Computations of drainage progress were begun anew with each irrigation. The increment of added drainable depth being added to the depth obtained from the preceding calculation. Computations are made for the point midway between drains. The pattern chosen to represent the data obtained from field observations is

$$h = 8H \left(\frac{x}{L} - \frac{3x^2}{L^2} + \frac{4x^3}{L^3} - \frac{2x^4}{L^4}\right)$$
 (8-11)

A first approximation solution having this initial configuration is given as

$$h_{1} = \frac{192H}{\pi^{5}} \sum_{m=0}^{m=\infty} \frac{[(2m+1)^{2} \pi^{2} - 8] e}{(2m+1)^{5}} \sin \frac{(2m+1)\pi x}{L}$$
(8-12)

At the point midway between drains this takes the form

^{*}See also USBR Eng. Monograph 31, 1966.

$$\frac{h_1}{(\frac{1}{H})} = \frac{192}{\pi^3} \sum_{n=1,3,5...}^{n=\infty} \frac{(-1)^{\frac{n-1}{2}} (n^2 - \frac{8}{\pi^2}) e^{-n^2\pi^2(\frac{\alpha t}{L^2})}}{n^5}$$
(8-13)

The initial drainable depth at the point midway between drains will here be represented by H_m . The quantity D_m is expressible in the notation of this volume as $D_m = (d + H_m)$ and the aquifer constant is, correspondingly, $\alpha_m = \frac{KD}{V}$. The quantity q_m represents the rate of discharge to unit length of drains from the space between two drains, or, if a single drain is considered, it represents the flow to unit length of drain, from both sides. The quantity W_m represents the total flow to a unit length of drain from both sides. The quantity W_m represents the total flow to a unit length of drain from both sides. The quantity W_m represents the drainable depth midway between drains at the time t.

For the case where the drains are on the barrier Boussinesq's solution is used.

The intermediate case was treated by comparison with field tests. It was found that the first approximation formula could be used providing an aquifer constant of the form:

$$\alpha = \frac{K D_a}{V}$$
 was used, where $D_a = (d + \frac{H}{2})$.

Moody's development

A development by Moody 1966 does away with the need to make such a choice. He used a computer to solve the nonlinear differential equation for a series of drain positions ranging from a location near the water table to a location on the barrier. He produced a table giving the maximum water table height, the

discharge to the drains and the volume of water removed. This is in dimensionless form and covers the entire range of possible drain positions between the water table and the barrier. The table is reproduced here as Table 8-1 through the courtesy of Mr. W. T. Moody and of the U. S. Bureau of Reclamation.*

The procedures described have been presented to the Profession in a series of papers and much comment has been received both favorable and unfavorable. The authors have made changes to meet the unfavorable comments and have correlated their methods with field data from Australia, Canada and the United States. The method has also been correlated with the results of laboratory studies. There is also accumulating the experience with field installations for which drain spacings have been selected by application of the method. The experience with such installations is understood to have been satisfactory. So far as this writer is aware, this method is the most carefully worked out, has received the most searching scrutiny, and has been more extensively tested against laboratory and field data than any method proposed for determining the spacing of drains which will, on the one hand, provide satisfactory drainage and on the other hand avoid the excessive expenditures incurred when the drains are spaced closer than necessary. Some valuable byproducts were obtained in the period of development, which has now covered 12 years and a study of the original papers is therefore recommended to those who become seriously involved in the task of selecting drain spacings. One of these is the need to account for the local resistance to flow near the drain. This problem will be dealt with later in the text.

From unpublished USBR data. For a Graphical Presentation see ASCE Paper No. 4835, by William T. Moody, on "Nonlinear Differential Equation of Drain Spacing,"
Journal of the Irrigation and Drainage Division, June 1966, pp. 1-9, inclusive.

Table 8-1

Values of Dimensionless Parameters for Water Surface Height(hm/Hm)Flow Rate (qmL/KDmHm) and Volume Drained (Wm/MLHm) for Given Parameters of Time ($_{\rm m}$ t/L²). For notation see page 98.

9*0 := m	hm qmL wm Hm KDmHm VLHm	1. 6.9757 0.	1.0000 6.9831 0.0014 0.9999 6.9852 0.0028 0.9999 6.9816 0.0042 0.9998 6.9745 0.0056	0.9997 6.9634 0.0070	0.9996 6.9496 0.0084 0.9995 6.9331 0.0098 0.9994 6.9146 0.0112 0.9992 6.8942 0.0125	0.9991 6.8724 0.0139 0.9967 6.6097 0.0274 0.9930 6.3289 0.0403 0.9880 6.0630 0.0527	0.9821 5.8192 0.0646	0.9753 5.5979 0.0760 0.9678 5.3971 0.0870 0.9597 5.2146 0.0976 0.9512 5.0481 0.1079	0.9424 4.8954 0.1178 0.8472 3.8409 0.2041 0.7566 3.1944 0.2741 0.6766 2.7178 0.3330
л в 0.7	hm qmL Wm Hm KDmHm VLHm	1. 5.7254 0.	1.0000 5.7710 0.0012 0.9999 5.8103 0.0023 0.9999 5.8419 0.0035 0.9998 5.8679 0.0047	0.9997 5.8882 0.0058	0.9996 5.9040 0.0070 0.9995 5.9155 0.0082 0.9994 5.9234 0.0094 0.9992 5.9280 0.0106	0.9991 5.9297 0.0118 0.9967 5.8462 0.0236 0.9930 5.6788 0.0351 0.9882 5.4908 0.0463	0.9824 5.3042 0.0570	0.9758 5.1267 0.0675 0.9685 4.9609 0.0776 0.9608 4.8070 0.0873 0.9528 4.6644 0.0968	0.9444 4.5321 0.1060 0.8557 3.5911 0.1863 0.7721 3.0019 0.2519 0.6984 2.5663 0.3074
m 0.8	h qmL Wm Hm KDmHm VLHm	1. 4.4752 0.	1.0000 4,5506 0.0009 0.9999 4.6199 0.0018 0.9999 4.6809 0.0028 0.9998 4.7355 0.0037	0.9997 4.7835 0.0047	0.9996 4.8259 0.0056 0.9995 4.8632 0.0066 0.9994 4.8959 0.0076 0.9992 4.9242 0.0085	0.9991 4.9487 0.0095 0.9967 5.0459 0.0196 0.9931 4.9969 0.0296 0.9883 4.8910 0.0395	0.9826 4.7644 0.0492	0.9762 4.6328 0.0586 0.9693 4.5034 0.0677 0.9619 4.3791 0.0766 0.9542 4.2612 0.0852	0.9462 4.1499 0.0936 0.8637 3.3255 0.1677 0.7868 2.7936 0.2285 0.7191 2.3981 0.2803
6.0 u m	$\begin{array}{ccc} h_m & q_m L & ^W_m \\ \hline H_m & KD_m H_m & VLH_m \end{array}$	1. 3.2249 0.	1.0000 3.3219 0.0007 0.9999 3.4138 0.0013 0.9999 3.4980 0.0020 0.9998 3.5763 0.0027	0.9997 3.6482 0.0035	0.9996 3.7144 0.0042 0.9995 3.7752 0.0050 0.9994 3.8310 0.0057 0.9992 3.8820 0.0065	0.9991 3.9286 0.0073 0.9967 4.2107 0.0155 0.9931 4.2880 0.0240 0.9884 4.2695 0.0325	0.9829 4.2061 0.0410	0.9766 4.1224 0.0494 0.9699 4.0306 0.0575 0.9628 3.9368 0.0655 0.9555 3.8441 0.0733	0.9480 3.7542 0.0809 0.8711 3.0466 0.1483 0.8006 2.5708 0.2042 0.7387 2.2134 0.2519
ы 1.0	hm qmi Wm Hm KDmHm VIHm	1. 1.9746 0.	1.0000 2.0845 0.0004 0.9999 2.1915 0.0008 0.9999 2.2928 0.0013 0.9998 2.3898 0.0018	0.9997 2.4815 0.0023	0.9996 2.5687 0.0028 0.9995 2.6508 0.0033 0.9994 2.7282 0.0038 0.9993 2.8011 0.0044	0.9991 2.8695 0.0049 0.9968 3.3459 0.0112 0.9932 3.5618 0.0182 0.9885 3.6382 0.0254	0.9831 3.6422 0.0327	0.9770 3.6084 0.0399 0.9706 3.5551 0.0471 0.9638 3.4921 0.0541 0.9567 3.4248 0.0610	0.9496 3.3563 0.0678 0.8779 2.7630 0.1287 0.8133 2.3404 0.1795 0.7569 2.0182 0.2229
	& t	0.	0.0002 0.0004 0.0006 0.0008	0.0010	0.0012 0.0014 0.0016 0.0018	0.0020 0.0040 0.0060 0.0080	0.0100	0.0120 0.0140 0.0160 0.0180	0.0200 0.0400 0.0600 0.0800

Table 8-1--Continued

	п = 1.0		0.0 m		-	₩ 0.8			m = 0.7			9.0 = 8	
Km t	h_{m} q_{mL} h_{m} V_{LH}	m Hm Hm	qmL KDmHm V	Wm hm I		q _m r KD _m H _m	W _m VLH _m	E H	q _m L KDmH _m	$\frac{q_mL}{KD_mH_m} \frac{W_m}{VLH_m} \frac{h_m}{H_m} \frac{q_mL}{KD_mH_m} \frac{W_m}{VLH_m} \frac{h_m}{H_m}$	h H	qml Wm KDmHm VIHm	W _m VIH _m
0.1000	0.7076 1.7613 0.2607		0.6845 1.9296 0.	0.2933	0.6598 2.0848 0.3250	2.0848 (0.3250	0.6338	0,6338 2,2214 0,3551	0.3551	0.6069	0.6069 2.3401 0.3834	0.3834
0.1200 0.1400 0.1600 0.1800	0.6643 1.5514 0.2937 0.6260 1.3772 0.3230 0.5918 1.2309 0.3490 0.5612 1.1067 0.3723		0.6369 1.6982 0. 0.5947 1.5061 0. 0.5572 1.3446 0. 0.5235 1.2076 0.	0.3295 0.3614 0.3899 0.4154	0.6077 0.5617 0.5207 0.4841	1.8291 0.3641 1.6168 0.3985 1.4384 0.4290 1.2869 0.4562	3641 3985 4290 4562	0.5773 1.9398 0.5274 1.7059 0.4833 1.5093 0.4439 1.3423	0.5773 1.9398 0.3967 0.5274 1.7059 0.4330 0.4833 1.5093 0.4651 0.4439 1.3423 0.4936	0.3967 0.4330 0.4651 0.4936	0.5460 0.4926 0.4455 0.4038	0.5460 2.0315 0.4270 0.4926 1.7750 0.4650 0.4455 1.5595 0.4983 0.4038 1.3767 0.5276	0.4270 0.4650 0.4983 0.5276
0.2000 0.4000 0.6000	0.5336 1.0004 0.3934 0.3576 0.4494 0.5275 0.2689 0.2541 0.5951 0.2155 0.1632 0.6358		1.0903 0.4811 0.2650 0.1644	0.4383 0.5835 0.6550 0.6969	0.4511 0.2450 0.1467 0.0923	1.1571 (0.4839 (0.2481 (0.1413 (0.4806 0.6314 0.7011 0.7388	0.4087 0.1941 0.1000 0.0534	0.4087 1.1994 0.5190 0.1941 0.4637 0.6705 0.1000 0.2146 0.7345 0.0534 0.1081 0.7654	0.5190 0.6705 0.7345 0.7654	0.3668 0.1501 0.0654 0.0292	0.3668 1.2205 0.5536 0.1501 0.4274 0.7019 0.0654 0.1738 0.7578 0.0292 0.0752 0.7813	0.5536 0.7019 0.7578 0.7813
1.0000	0.1798 0.1136 0.6630		0.1106 0.1099 0.	0.7238	0.0596 0.0855 0.7610	0.0855	0.7610	0.0290	0.0290 0.0569 0.7813	0.7813	0.0132	0.0132 0.0335 0.7916	0.7916
1.2000 1.4000 1.6000 1.8000	0.1542 0.0836 0.6825 0.1350 0.0641 0.6971 0.1200 0.0507 0.7085 0.1081 0.0411 0.7176		0.0771 0.0561 0.0419 0.0319	0.7422 0.7554 0.7651 0.7725	0.0391 0.0537 0.7746 0.0259 0.0345 0.7833 0.0173 0.0226 0.7889 0.0116 0.0149 0.7926	0.0391 0.0537 0.7746 0.0259 0.0345 0.7833 0.0173 0.0226 0.7889 0.0116 0.0149 0.7926	0.7746 0.7833 0.7889 0.7926	0.0159 0.0088 0.0048 0.0027	0.0159 0.0306 0.7898 0.0088 0.0167 0.7944 0.0048 0.0092 0.7969 0.0027 0.0051 0.7983	0.7898 0.7944 0.7969 0.7983	0.0060 0.0027 0.0012 0.0006	0.0060 0.0151 0.796 0.0027 0.0068 0.798 0.0012 0.0031 0.799 0.0006 0.0014 0.799	0.7962 0.7983 0.7992 0.7996
2.0000	0.0983 0.0340 0.7251		0.0333 0.0246 0.	0.7781	0.0078 0.0099 0.7950	0.0099		0.0015	0.0015 0.0028 0.7991	1.7991	0.0003	0.0003 0.0006 0.7998	0.7998

Table 8-1--Continued

m = 0.1	$\frac{h_m}{H_m} \frac{q_m L}{K D_m H_m} \frac{w_m}{V L H_m}$	13.2271 0.	1.0000 12.9280 0.0026 0.9999 12.6428 0.0052 0.9999 12.3808 0.0077 0.9998 12.1482 0.0101	.9997 11.9319 0.0125	0.9996 11.7363 0.0149 0.9995 11.5527 0.0172 0.9994 11.3842 0.0195 0.9992 11.2247 0.0218	0.9991 11.0764 0.0240 0.9966 9.9327 0.0449 0.9927 9.1404 0.0640 0.9873 8.5315 0.0816	.9805 8.0385 0.0982	0.9725 7.6264 0.1138 0.9633 7.2738 0.1287 0.9530 6.9671 0.1430 0.9420 6.6966 0.1566	0.9302 6.4553 0.1698 0.7964 4.8964 0.2814 0.6679 3.9695 0.3695 0.5579 3.2730 0.4417
m = 0.2	qmL wm KDmHm VLHm	11.9768 0. 1.	11.7538 0.0024 1 11.5389 0.0047 0 11.3389 0.0070 0 11.1584 0.0092 0	9997 10.9887 0.0115 0.9	. 9996 10.8331 0.0136 0.9 . 9995 10.6858 0.0158 0.9 . 9994 10.5489 0.0179 0.9	10.2961 0.0221 9.3247 0.0416 8.6277 0.0596 8.0819 0.0763	9809 7.6345 0.0920 0.9	7.2570 0.1068 0 6.9321 0.1210 0 6.6480 0.1346 0 6.3964 0.1476 0	6.1713 0.1602 4.7061 0.2672 3.8357 0.3521 3.1860 0.4221
п п 0.3	GL Wm hm KDH VIJH Hm	10.7265 0. 1.	10.5724 0.0021 1.0000 10.4217 0.0042 0.9999 10.2786 0.0063 0.9999 10.1470 0.0084 0.9998	.0214 0.0104 0.	9.9043 0.0124 0.99 9.7920 0.0143 0.99 9.6863 0.0163 0.99 9.5845 0.0182 0.99	9.4879 0.0201 0.9991 8.6914 0.0382 0.9967 8.0932 0.0550 0.9928 7.6129 0.0707 0.9875	.2129 0.0855 0.	6.8718 0.0996 0.9731 6.5758 0.1130 0.9643 6.3152 0.1259 0.9546 6.0834 0.1383 0.9441	.8752 0.1503 0.5066 0.2525 0.6920 0.3340 0.0875 0.4015
F	h H	1. 10	1.0000 10 0.9999 10 0.9999 10	0.9997 10	0.9996 0.9995 0.9994 0.9992	0.9991 0.9967 0.9928 0.9876	0.9812 7	0.9737 6 0.9652 6 0.9560 6	0.9356 5 0.8184 4 0.7053 3 0.6068 3
m = 0.4	$\frac{h_m}{H_m} = \frac{q_m L}{KD_m H_m} = \frac{w_m}{VLH_m}$	1. 9.4762 0.	1.0000 9.3837 0.0019 0.9999 9.2905 0.0038 0.9999 9.1994 0.0056 0.9998 9.1132 0.0074	0.9997 9.0289 0.0093	0.9996 8.9484 0.0111 0.9995 8.8698 0.0128 0.9994 8.7943 0.0146 0.9992 8.7206 0.0164	0.9991 8.6494 0.0181 0.9967 8.0298 0.0347 0.9929 7.5339 0.0503 0.9878 7.1220 0.0649	0.9815 6.7716 0.0788	0.9742 6.4686 0.0921 0.9661 6.2027 0.1047 0.9573 5.9669 0.1169 0.9479 5.7558 0.1286	0.9380 5.5651 0.1399 0.8285 4.2969 0.2371 0.7231 3.5380 0.3150 0.6307 2.9771 0.3799
B 0.5	$\frac{h_m}{H_m} \frac{q_m L}{KD_m H_m} \frac{w_m}{VT_n H_m}$	1. 8.2260 0.	1.0000 8.1873 0.0017 0.9999 8.1452 0.0033 0.9999 8.1006 0.0049 0.9998 8.0559 0.0065	0.9997 8.0100 0.0081	0.9996 7.9640 0.0097 0.9995 7.9174 0.0113 0.9994 7.8710 0.0129 0.9992 7.8245 0.0145	0.9991 7.7784 0.0160 0.9967 7.3369 0.0311 0.9929 6.9468 0.0454 0.9879 6.6062 0.0590	0.9818 6.3079 0.0719	0.9748 6.0447 0.0842 0.9670 5.8106 0.0961 0.9586 5.6008 0.1075 0.9496 5.4113 0.1185	0.9403 5.2392 0.1291 0.8382 4.0756 0.2210 0.7402 3.3725 0.2956 0.6540 2.8542 0.3571
	Kmt Lant	0.	0.0002 0.0004 0.0006 0.0008	0.0010	0.0012 0.0014 0.0016 0.0018	0.0020 0.0040 0.0060 0.0080	0.0100	0.0120 0.0140 0.0160 0.0180	0.0200 0.0400 0.0600 0.0800

Table 8-1--Continued

	E	m = 0.5		+	m = 0.4			m = 0.3			m = 0.2			m = 0.1	
Km t	hm qmL Wm hm Hm KDmHm VLHm Hm	qmI JmHm V	LH'm		$\begin{array}{ccc} q_m L & \textbf{W}_m & \textbf{h}_m \\ KD_m H_m & VLH_m & H_m \end{array}$	W W VLH M	h H	$\frac{q_mL}{KD_mH_m} \frac{w_m}{VLH_m}$		r H	qmL Wm KDmHm VLHm	Wm VLHm	H H H	q _m L KDmH _m	Wm VLHm
0.1000	0.5792 2.4427 0.4099	4427 0.		0.5510	0.5510 2.5306 0.4348	,4348	0.5225	2.6046 0.4583	.4583	0.4940	2.6652 0.4804		0.4658	0.4658 2.7128 0.5014	.5014
0.1200	0.5142 2.1061 0.4553 0.4576 1.8263 0.4945	1061 0.4 8263 0.4		0.4823	2.1649 (1.8611 (.4817	0.4506	2.2087 0.5063 1.8803 0.5471	.5063	0.4194	2.2381 0.5293 1.8846 0.5704		0.3890	2.2537 0 1.8750 0	.5509
0.1600 0.1800	0.4081 1.5914 0.5287 0.3646 1.3926 0.5584	5914 0.3		0.3716	0.3716 1.6065 0.5564 0.3269 1.3915 0.5864	5564	0.3365	1.6058 0.5819 1.3750 0.6116	.6116	0.3031	1.5904 0.6051 1.3446 0.6344			1.5617 0.6263 1.3020 0.6549	.6263
0.2000	0.3263 1.2231 0.5846	2231 0.		0.2880	1.2090 (6123	0.2523	1.1801 0.6371	.6371	0.2196	1.1385 0.6592			1.0862 0	.6787
0008.0	0.0414 0.1331 0.7735 0.0415 0.0485 0.7902	1531 0.1		0.0256 (0.0256 0.0971 0.7837 0.0078 0.0295 0.7950	7837 7950	0.0018 0.0039	0.0682 0.7901 0.0682 0.7901 0.0171 0.7975	.7901	0.0092	0.0464 0.7941 0.0096 0.7988		0.0054 0.0009	0.0307 0.7965 0.0052 0.7994	.7965
1.0000	1.0000 0.0057 0.0180 0.7964	0180 0.		0.0024 (0.0024 0.0090 0.7985 0.0010	.7985	0.0010	0.0043 0.7994	.7994	0.0004	0.0020 0.7997		0.0002	0.0009 0.7999	.7999

Moody's development is based upon an initial configuration given by the relation $h=H \left[1-\left(\frac{2x}{L}\right)^4\right]$ vrigin of coordinates is at midspan. The following notation annlies to his development m His origin of coordinates is at midspan. The following notation applies to his development.

(d + H)

represents the initial drainage depth, at midspan, at time zero. the drainable depth, at midspan, at the time t.

H = m D m $H = H = \frac{H}{d + H}$ E

the rate of discharge from the area between two drains, per unit length along the drains. the total quantity of water removed from the area between two drains, per unit length ŧ1 o^E ₃ E

distance from midspan measured horizontally toward a drain. along the drains, up to the time ×E

Table 8-1--Continued

m = 0

ocm t	h _m	q _m L	W _m
L ²	$\frac{m}{H_{m}}$	K D _m H _m	V L Hm
·o.	1.	16.	0.
0.001	0.9998	12.9402	0.0139
0.002	0.9992	11.8719	0.0263
0.003	0.9983	11.1239	0.0377
0.004	0.9969	10.5388	0.0486
0.005	0.9952	10.0560	0.0589
0.006	0.9931	9.6447	0.0687
0.007	0.9906	9.2866	0.0782
0.008	0.9877	8.9698	0.0873
0.009	0.9845	8.6862	0.0961
0.010	0.9808	8.4298	0.1047
0.020	0.9279	6.7266	0.1795
0.030	0.8579	5.7562	0.2415
0.040	0.7844	5.0790	0.2955
0.050	0.7137	4.5459	0.3436
0.060	0.6478	4.0957	0.3867
0.070	0.5874	3.7014	0.4257
0.080	0.5324	3.3496	0.4609
0.090	0.4825	3.0332	0.4928
0.100	0.4372	2.7475	0.5217
0.200	0.1629	1.0238	0.6963
0.300	0.0607	0.3816	0.7613
0.400	0.0226	0.1422	0.7856
0.500	0.0084	0.0530	0.7946
0.600	0.0031	0.0198	0.7980
0.700	0.0012	0.0074	0.7993
0.800	0.0004	0.0027	0.7997
0.900	0.0002	0.0010	0.7999
1.000	0.0001	0.0004	0.8000

Parallel drains from the Laplace standpoint*

Consider the expression

$$p = (d-y) + \sum_{n=1}^{n=m} A_n \cosh (\frac{n\pi y}{L}) \sin (\frac{n\pi x}{L})$$
 (8-14)

where p represents a pressure. It is measured in feet of water and represents a departure from the pressures appropriate to a static state. This is a solution of the Laplace differential equation.

$$\frac{\partial^2 p}{\partial x^2} + \frac{\partial^2 p}{\partial y^2} = 0$$

which meets the requirement that $\frac{\partial p}{\partial y} = 0$ when y = 0. The coordinate y is measured upward from the barrier. The quantities A_n are to be chosen to meet the initial conditions that

$$p = H$$
 when $y = d$ for $0 < x < L$

The term (d-y) represents the hydrostatic pressure which would be present if the water table were at the level of the water surface maintained in the drains. The quantity p represents a pressure in feet of water and the terms under the summation sign represent the additional pressures present when the water table is above the level of the drains. The pressure p will be zero when

$$0 = (d-y) + \sum_{1}^{m} A_{n} \cosh \left(\frac{n\pi y}{L}\right) \sin \left(\frac{n\pi x}{L}\right) .$$

This presentation follows closely that of the paper on "Parallel Drains from the Laplace Standpoint," by Robert E. Glover, which appeared in the Journal of the American Water Resources Association, Vol. 8, No. 1, February 1972, pp. 50-54 inclusive. The development is presented here through the courtesy of the A.W.R.A.

If the quantity n has an upper limit m , which implies a finite number of terms in the series, and $\frac{n\pi y}{L}$ is everywhere small compared to unity; then

$$\cosh \left(\frac{n\pi y}{L}\right) = 1$$

$$y_0 = d + \sum_{1}^{m} A_n \sin(\frac{n\pi x}{L})$$
.

This expression represents a water table profile. If a uniform increment of depth H reaches the water table, due to deep percolation from a uniform application of irrigation water, then the pressure imposed at drain level by the water table profile can be represented initially by the expression

$$p_0 = \frac{4H}{\pi} \sum_{n=1,3,5,...}^{n=\infty} \frac{1}{n} \sin \left(\frac{n\pi x}{L}\right) . \tag{8-15}$$

If the series is terminated at the mth term then the expression can represent approximately a uniform increment of depth H. The relationships considered up to this point do not involve the element of time. This factor can now be introduced.

After the increment is applied, water will flow to the drains and the water table will begin to sink. The flows to each drain accounted for by the individual terms of the series will be:

$$K \int_{0}^{d} \left(\frac{\partial p}{\partial x}\right)_{0} dy = A_{n} K \frac{n\pi}{L} \int_{0}^{d} \cosh \left(\frac{n\pi y}{L}\right) dy = A_{n} K \sinh \left(\frac{n\pi y}{L}\right) \Big]_{0}^{d}$$

$$= A_{n} K \sinh \left(\frac{n\pi d}{L}\right)$$

The volume above the line y = d is, approximately, for each term

$$S_{n} \stackrel{\cong}{=} \int_{0}^{L} (y-d) dx = A_{n} \cosh \left(\frac{n\pi d}{L}\right) \int_{0}^{L} \sin \left(\frac{n\pi x}{L}\right) dx \stackrel{\cong}{=} 2 A_{n} \frac{L}{n\pi} \cosh \left(\frac{n\pi d}{L}\right).$$

The continuity condition for each term is, since there is flow out at x=0 and at x=L

$$V \frac{\partial S_n}{\partial t} = 2 K \int_0^d (\frac{\partial p}{\partial x})_0 dy$$

In this expression t represents time. By substitution:

$$\frac{dA_n}{dt} \frac{2LV}{n\pi} \cosh \left(\frac{n\pi d}{L}\right) = 2 A_n K \sinh \left(\frac{n\pi d}{L}\right)$$

or if

$$\beta = \frac{Kn\pi}{LV} \quad \tanh \quad (\frac{n\pi d}{L})$$

$$\frac{dA_n}{dt} + \beta A_n = 0$$

This is a differential equation whose solution is:

$$A_n = B_n e^{-\beta t}$$
.

Where the $\mbox{\ensuremath{B}}_n$ quantities are new constants. Then all of the requirements described previously will be met to a close approximation if

$$p = \frac{4H}{\pi} \sum_{n=1,3,5...}^{n=m} \frac{e^{-\beta t}}{n} \sin \left(\frac{n\pi x}{L}\right) \frac{\cosh \left(\frac{n\pi y}{L}\right)}{\cosh \left(\frac{n\pi d}{L}\right)} + (d-y)$$
 (8-16)

This expression remains an exact solution of equation 1.

When

$$\frac{n\pi d}{L} <<1 \qquad \qquad tanh \ (\frac{n\pi d}{L}) \ \tilde{=} \ \frac{n\pi d}{L}$$

and with

$$\beta = \frac{\alpha n^2 \pi^2}{r^2} \qquad \alpha = \frac{Kd}{V}$$

Then, approximately,

$$h \approx \frac{4H}{\pi} \sum_{n=1,3,5...}^{n=m} \frac{e^{-n^2\pi^2} \left(\frac{\alpha t}{L^2}\right)}{e} \sin \left(\frac{n\pi x}{L}\right) + (d-y) .$$

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The formula derived from the Dupuit-Forchheimer idealization is then recovered but with the important exception that here

$$\alpha = \frac{Kd}{V} \tag{8-17}$$

If the solution is limited to three terms and $\frac{\pi y}{L} < 0.1$ everywhere then the Dupuit-Forchheimer and Laplace solutions become essentially identical in form and an approximation to the initial condition is obtained which is close enough for practical purposes. Since the higher ordered terms vanish rapidly it is permissible to use Table 10 for computation of the remaining drainable depth at x = L/2 except that here the aquifer constant must be of the form $\alpha = Kd/V$. Examples

Use of the first approximation solution and the Laplace type solution will be illustrated by use of an example adapted from Dumm's 1964 paper. The given data are:

Depth from ground surface to barrier 30 ft
Depth from ground surface to drain 8 ft

Drainable depth produced by an irrigation 0.46 ft

Permeability K = 10 ft/dayEffective voids ratio 0.18Drain spacing L = 1450 ft

We will compute the remaining drainable depth at the point midway between drains at the end of successive seven day periods. Values for three months, six months, and one year are added. The differences between the values obtained by the first approximation and the Laplace type solution are due only to the difference in the α values. These differences are small. The drainable depth at the center does not respond immediately to the action of the drains. The part remaining, however, responds very quickly due to drainage taking place in the immediate neighborhood of the drain.

Computation by first approximation

K = 10 ft/day
$$D_a = (d + \frac{H}{2}) = (22 + \frac{0.46}{2}) = 22.23$$
 ft
V = 0.18 $\alpha = \frac{KD_a}{V} = \frac{(10)(22.23)}{0.18} = 1235 \frac{ft^2}{day}$ L = 1450 ft
 $\frac{\alpha}{L^2} = \frac{1235}{1450^2} = 0.0005874 \frac{1}{day}$ H = 0.46 ft

Time Days	$\frac{\alpha t}{L^2}$	$\frac{h_c}{H}$	h _с	p	Remarks
0	0	1.000	0.460	1.000	Values of $\frac{h_c}{H}$ can be
7	0.004112	1.000	0.460	0.855	read from Table 10.
14	0.008224	1.000	0.460	0.795	Values of p can be
21	0.012335	0.997	0.459	0.749	read from Table 11.
28	0.016447	0.988	0.454	0.711	
35	0.020559	0.973	0.448	0.676	
42	0.024671	0.951	0.473	0.646	
49	0.028782	0.926	0.426	0.617	
56	0.032894	0.897	0.413	0.591	
63	0.037006	0.868	0.399	0.566	

Time Days	αt L ²	h _C H	h _c	p	Remarks
70	0.041118	0.838	0.385	0.543	
77	0.045229	0.807	0.371	0.520	
84	0.049341	0.777	0.357	0.479	
91	0.053453	0.748	0.344	0.499	Three months
182	0.106906	0.443	0.204	0.282	Six months
365	0.214400	0.153	0.070	0.098	One year

Computation by the Laplace type solution

$$\alpha = \frac{Kd}{V} = \frac{(10)(22)}{0.18} = 1222 \text{ (ft}^2/\text{day)} (\alpha/L^2) = 0.0005813$$

Time Days	$\frac{\alpha t}{L^2}$	$\frac{h}{C}$	^h c	p	Remarks
0	0	1.000	0.460	1.000	
7	0.004069	1.000	0.460	0.845	
14	0.008138	1.000	0.460	0.796	
21	0.012207	0.997	0.459	0.751	
28	0.016276	0.989	0.455	0.712	
35	0.020346	0.974	0.448	0.678	
42	0.024417	0.953	0.438	0.647	
49	0.028484	0.928	0.427	0.619	
56	0.032553	0.900	0.414	0.593	
63	0.036622	0.871	0.401	0.568	
70	0.040691	0.841	0.387	0.545	
77	0.044760	0.811	0.373	0.523	
84	0.048829	0.781	0.359	0.502	
91	0.052898	0.752	0.346	0.482	Three months
182	0.105797	0.448	0.206	0.285	Six months
365	0.212174	0.157	0.072	0.100	One year

Note:
$$\frac{\pi y_m}{L} = \frac{(3.1416) (22.46)}{1450} = 0.0487$$

These two examples yield closely similar results because the drainable depth is small compared to the saturated depth below the drains. It will be

profitable to now consider a somewhat extreme case where the drainable depth is nearly equal to the saturated depth below the drains and to again compare the results of computations made by several methods.

As an example of a case where the drains are located about midway between the water table and the barrier, data from a field installation supplied by Mr. Ray Winger of the Bureau of Reclamation will be used. The data are:

Depth of barrier below ground surface 16 ft

Drain depth 9 ft

Permeability 1.4 (ft/day)

Effective voids ratio 0.093

Maximum allowable water table height 3 ft
below ground surface or 6 ft above the drains

Drain spacing 510 ft

Computation of the drainable depth midway between drains by the method of Moody.

$$D_{\rm m} = (16 - 3) = 13 \text{ ft.}$$
 $\alpha_{\rm m} = \frac{(1.4)(13)}{0.093} = 195.7(\text{ft}^2/\text{day}).$ $m = \frac{6}{13} = 0.462.$

Time Days	$(\frac{\alpha_{m}t}{L^{2}})$	$(\frac{h_m}{H_m})$	h _m
0	0	1.000	6.00
20	.0150	.960	5.76
40	.0301	.882	5.29
60	.0451	.809	4.85
80	.0602	.736	4.42
100	.0752	.666	4.00
120	.0903	.606	3.64
140	.1053	.556	3.34
160	.1204	.506	3.04
180	.1354	.463	2.78
270	.2031	.328	1.97
365	.2746	.227	1.36

Time
$$(\frac{\alpha_m t}{L^2})$$
 $(\frac{h}{H_m})$ h_m

42 0.7584 0.015 0.09

49 0.8848 0.008 0.05

One day is 24 hours.
$$\frac{\alpha_m t_1}{L^2} = \frac{(195.7)(24)}{510^2} = \frac{4696.8}{260100} = 0.018057$$

Computation by first approximation method.

$$D_a = (d + \frac{H_o}{2}) = (7 + \frac{6}{2}) = 10 \text{ ft}$$
 $\alpha = \frac{KD_a}{V} = \frac{(1.4)(10)}{0.093} = 150.5 \text{ (ft}^2/\text{day)}.$

$$\frac{\alpha t_1}{L^2} = \frac{(150.54)(24)}{510^2} = 0.01389$$

Time Days	$(\frac{\alpha t}{L^2})$	$\left(\frac{\frac{h_c}{H_o}}{\frac{h_c}{H_o}}\right)^*$	^h c
0	0	1.0000	6.00
20	.0116	.9979	5.99
40	.0231	.9599	5.76
60	.0347	.8844	5.31
80	.0463	.7992	4.80
100	.0579	.7164	4.30
120	.0694	.6409	3.85
140	.0816	.5687	3.41
160	.0926	.5103	3.06
180	.1042	.4551	2.73
270	.1562	.2724	1.63
365	.2112	.1583	0.95

^{*}Read from Table 10.

Computation by the method of Brooks.

Time Days	^h c	$(h_c - \frac{H_o}{2})$	h ₁	$(h_1 + \frac{H_0}{2})$
0	6.00	3.00	3.00	6.00
20	5.99	2.99	2.99	5.99
40	5.76	2.76	2.81	5.81
60	5.31	2.31	2.46	5.46
80	4.80	1.80	2.04	5.04
100	4.30	1.30	1.62	4.62
120	3.85	0.85	1.22	4.22
140	3.41	0.41	0.83	3.83
160	3.06	0.06	0.50	3.50
180	2.73	-0.27	0.18	3.18
270	1.63	-1.37	-0.96	2.04
365	0.95	-2.05	-1.75	1.25

Notes: The numbers in the column headed h_c are those of the first approximation. The numbers in the column headed $(h_c - \frac{H_o}{2})$ are those of the first approximation referred to an origin $(H_o/2)$ above the level of the drains. The column headed h_1 is Brooks second approximation, computed by use of formula 8-10. The figures in the last column are those of the previous column referred back to drain level. They compare with the first approximation figures in the column headed h_c .

Reference: Brooks, R.H., 1963, ASCE Paper 3420.

Laplace type solution.

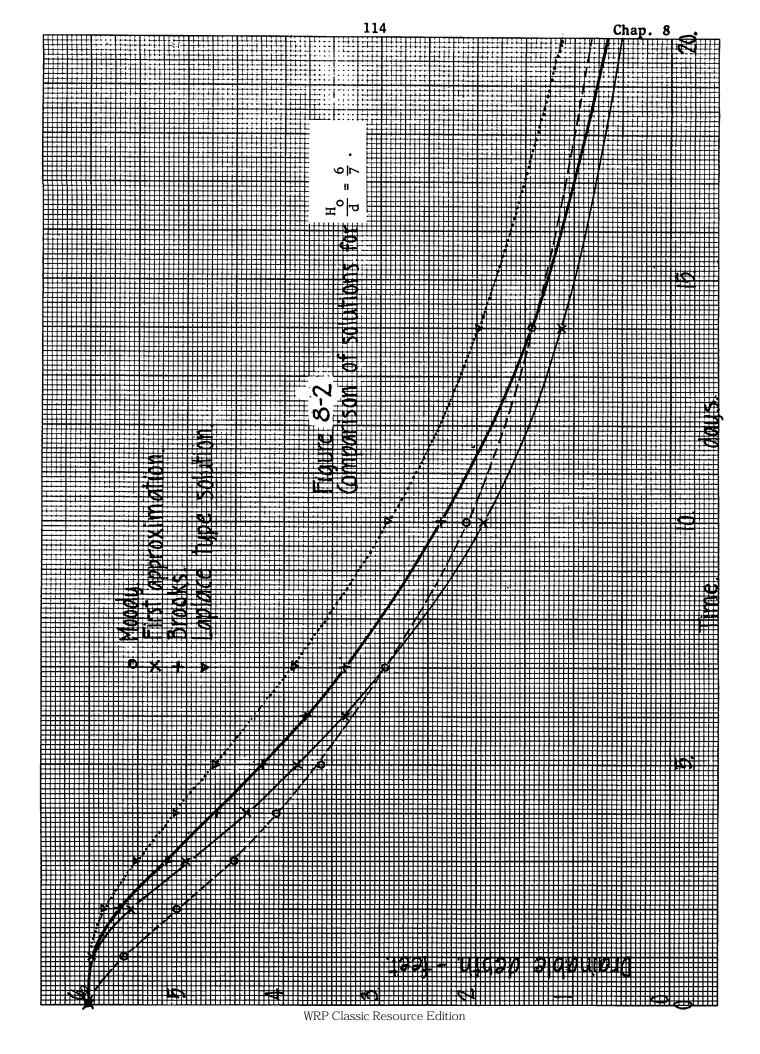
d = 7.0 ft
$$\alpha_{L} = \frac{Kd}{V} = \frac{(1.4)(7.0)}{0.093} = 105.4 (ft^{2}/day)$$

$$L = 510 ft H_{0} = 6.00 ft$$

Time Days	$(\frac{\alpha_L^t}{L^2})$	$(\frac{h_c}{H_o})$	h _c
0	0	1.000	6.00
20	.0081	1.000	6.00
40	.0162	.989	5.93
60	.1243	.953	5.72
80	.0324	.901	5.40
100	.0405	.842	5.05
120	.0486	.782	4.69
140	.0567	.725	4.35
160	.0648	.670	4.02
180	.0729	.619	3.72
270	.1094	.432	2.60
365	.1479	.296	1.77

Note: The values in the column headed (h_c/H_o) were obtained from Table 10.

The results of these computations are shown on figure 8-2. The solid heavy line represents Brooks second approximation which will here be used as a basis for comparison. The light solid line shows the results obtained by use of the first approximation solution. It holds up surprisingly well even though here the drainable depth is almost half of the original saturated depth and nearly equal to the saturated depth below the drains. The dashed curve shows the results obtained from using Moody's computer solution. This solution and the first and second approximation solutions are not strictly comparable because they have different initial conditions. The initial



condition of Moody's solution, however, represents closely a configuration which would appear at an early epoch in the drainage of a uniform drainable depth. If Moody's initial configuration is superimposed on such a chart as that of figure 7 of USBR Monograph 31, it will be found to correspond nearly to the profile for $\frac{\alpha t}{r^2}$ = 0.014. A second approximation curve obtained by use of Brooks formula indicates that the parameter should be about $\frac{\alpha t}{L^2}$ = 0.010 to produce a close fit. Since the value of $\frac{\alpha t_1}{L^2}$ = 0.015 for time 20 days is substantially this amount it can be concluded that if Moody's curve is shifted to the right about 20 days on figure 8-2 the effect of the differing initial conditions will be accounted for. If this is done Moody's result and Brook's second approximation will be in close agreement over the first ten days. The solution obtained from the Laplace formulation is similar in shape to the second approximation curve of Brooks but lies above The reason for this seems to be that this solution accounts for the head loss needed to produce vertical as well as horizontal flow whereas the other solutions account for the horizontal component of flow only. A particle of water initially at the water table ten feet back from the drain, for example, has to travel six feet vertically and 10 feet horizontally to reach the The solution of the Laplace equation accounts for this but the solutions derived on the Dupuit-Forchheimer basis only account for the horizontal ten feet of distance. The drainage is therefore slowed near the drain and the drainage of water remote from the drain is also slowed because it cannot reach the drain until the water close to the drain is disposed of. This comparison brings out the important effect of flow convergence near the drain. More will be said on this point later.

Selection of drain spacings

The formulas described can be used as a means for computing drain spacings on a cut and try basis. The procedure will be illustrated by use of the first approximation formula. The $h_{\rm c}/{\rm H}$ values will be obtained from Table 10. The computation will be based upon figures adapted from Lee D. Dumm's 1964 paper. The allowable rise of the water table at mid-span at the end of the irrigation season is 4 ft. It will be assumed that this height is attained at the end of the previous irrigation season.

Data are:

K = 10 ft/day KD = 220 ft²/day
D = 22 ft
$$\alpha = \frac{KD}{V} = 1222.23 \text{ ft}^2/\text{day}$$
V = 0.18
$$\frac{\alpha}{L^2} = \frac{1222.23}{1500^2} = 0.00054321$$

Try a spacing of 1500 feet.

Application	Time Days	Drainable Depth ft	$(\frac{\alpha t}{L^2})$	$(\frac{h_c}{H})$	h _c
Apr 22*	132	0.46	0.0717	0.6267	0.288
June 6	87	0.46	0.0473	0.7920	0.364
July 1	62	0.46	0.0337	0.8887	0.409
July 21	42	0.46	0.0228	0.9616	0.442
Aug 4	28	0.46	0.0152	0.9917	0.456
Aug 18	14	0.46	0.0076	0.9998	0.460
Sept 1	0	0.46	0	1.0000	0.460
-	365	4.00	0.1983	0.1799	0.720
			То	tal	3.599

^{*}Snowmelt

This spacing can be widened. Try a spacing of 1700 ft.

Application	Time Days	Drainable Depth ft	$(\frac{\alpha t}{L^2})$	$(\frac{h_c}{H})$	h _c
Apr 22*	132	0.46	0.0558	0.731	0.336
June 6	87	0.46	0.0368	0.869	0.400
July l	62	0.46	0.0262	0.942	0.433
July 21	42	0.46	0.0178	0.984	0.453
Aug 4	28	0.46	0.0118	0.998	0.459
Aug 18	14	0.46	0.0059	1.000	0.460
Sept 1	0	0.46	0	1.000	0.460
	365	4.00	0.1544	0.277	1.110
			To	tal	4.111

*Snowmelt

This spacing is too wide. By interpolation, a spacing of 1657 feet would just meet the requirements of a four-foot rise at the end of the irrigation season.

The method of M. Maasland

The previous treatments of drainage by parallel drains has been based upon the concept of a drainable depth which comes into existence at time zero. This is an idealization of furrow irrigation practices where the irrigation water is applied during a brief interval of time and produces, by deep percolation, a drainable depth H.

The Maasland approach is somewhat different and possesses certain advantages which will be described later. He assimilates the deep percolation from a succession of irrigations to an average infiltration rate i. The consequences of such an idealization may be approached through the use of the formula for drainage of a single uniform drainable depth H (f. 8-1). This is:

$$h = \frac{4H}{\pi} \sum_{n=1,3,5,\dots}^{n=\infty} \frac{e^{-n^2\pi^2(\frac{\alpha t}{L^2})}}{n} \sin(\frac{n\pi x}{L})$$

Suppose the drainable depth dH appears at the time ξ where ξ represents a time variable running between 0 and t. The variable ξ indicates the time of occurrence of an event whose effect is to be computed at the time t.

The drainable depth at the point $\,x\,$ at the time $\,t\,$ will then be given by

$$h = \frac{1}{V} \int_{0}^{t} \frac{4}{\pi} \sum_{n=1,3,5...}^{n=\infty} \frac{e^{-n^2 \pi^2 (\frac{\alpha(t-\xi)}{L^2})}}{n} \sin \left(\frac{n\pi x}{L}\right) d\xi$$

Where $\frac{i}{V}$ is the rate of rise of the water table due to the constant infiltration rate i. The infiltration is considered to be entire water.

By integration

$$h = \frac{14}{V\pi} \frac{L^2}{\alpha \pi^2} \sum_{n=1,3,5...}^{n=\infty} \frac{e^{-n^2 \pi^2 (\frac{\alpha(t-\xi)}{L^2})}}{e^{n^3}} \sin(\frac{n\pi x}{L})$$

or

$$h = \frac{i4L^{2}}{Kd\pi^{3}} \sum_{n=,1,3,5...}^{n=\infty} \frac{1}{n^{3}} \sin\left(\frac{n\pi x}{L}\right)$$

$$-\frac{i4L^{2}}{Kd\pi^{3}} \sum_{n=1,3,5...}^{n=\infty} \frac{e}{n^{3}} \sin\left(\frac{n\pi x}{L}\right)$$
(8-18)

At x = L/2, the point midway between the drains,

$$\frac{n\pi x}{L} = \frac{n\pi}{2}$$

A1so

$$\frac{1}{1^3} - \frac{1}{3^3} + \frac{1}{5^3} - \frac{1}{7^3} + \dots = \frac{\pi^3}{32}$$

This is one of the Euler numbers. A plot of formula 8-18 is shown on figure 8-3.

The summation of descending exponentials disappears in time. Then there is an ultimate steady state given by the summation which is free of exponentials. This is:

$$h_{c} = \frac{i4L^{2}}{Kd\pi^{3}} \sum_{n=1,3,5...}^{n=\infty} \frac{1}{n^{3}} \sin \left(\frac{n\pi}{2}\right) = \frac{i4L^{2}}{Kd\pi^{3}} \frac{\pi^{3}}{32} = \frac{iL^{2}}{8Kd}$$
(8-19)

An independent development for the steady state will be of interest. The statement that the flow is equal to the supply is

$$Kd \frac{dh}{dx} = i \left(\frac{L}{2} - x\right).$$

By integration, if h = 0 when x = 0

$$h = \frac{i}{2Kd} x(L - x).$$

When $x = \frac{L}{2}$

$$h_{c} = \frac{iL^2}{8Kd} \tag{8-20}$$

as before. It will be found that the term

$$\frac{i4L^2}{V\alpha\pi^3} \sum_{n=1,3,5,...}^{n=\infty} \frac{1}{n^3} \sin \left(\frac{n\pi x}{L}\right)$$
 is the Fourier series

which represents

$$h = \frac{i}{2Kd} \times (L - x) .$$

Formulas of the type of equation 8-20 have been used to estimate drain spacings. It will be clear that this procedure implies that the irrigation season is long enough to establish the ultimate steady state. Under ordinary conditions the irrigation season is too short to establish an ultimate steady state and the result is that drain spacings obtained by use of ultimate steady state relations are closer than necessary to provide drainage. To put this in other words, the use of ultimate steady state formulas, based upon the concept of a continuous infiltration rate i, neglects the favorable effects of the winter drain-out period.

This difficulty can be substantially overcome if the effects of a succession of seasonal applications are considered. This is a case of intermittent operation as treated in Chapter 11. With an irrigation pattern as illustrated on figure 8-4 the effects of previous irrigations and cessation of irrigations can be treated in the following way. The height of the water table midway between drains at the end of the last irrigation season is of interest. Here T represents the yearly period and T/3 the irrigation period.

Suppose

$$(\alpha T/L^2) = 0.2$$

then the computation is made in the following way.

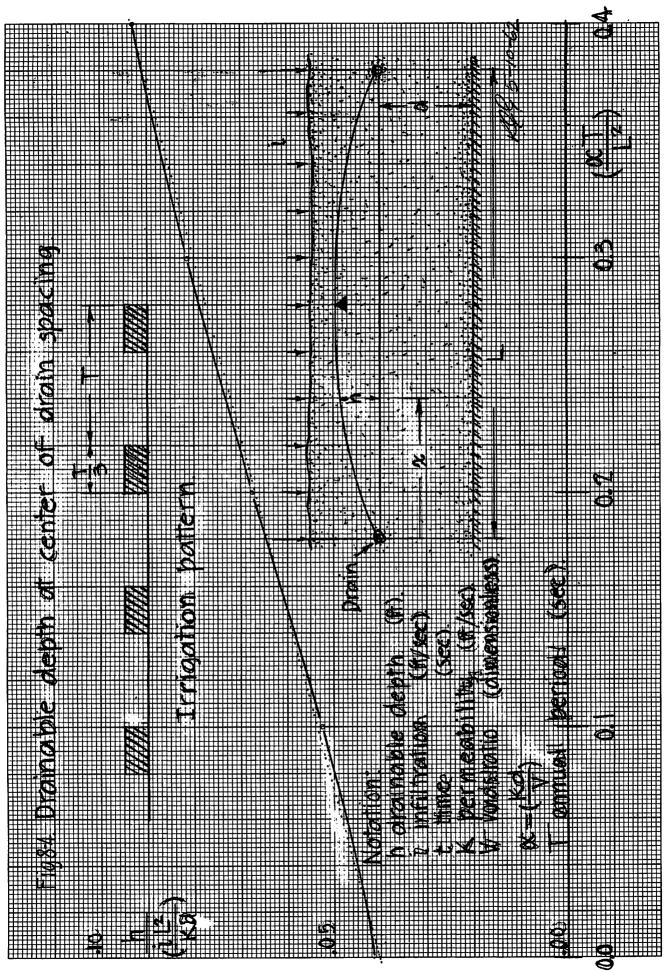


Table 8-2 Computation of $h/(\frac{iL^2}{Kd})$ for intermittent irrigation. $(\alpha T/L^2) = 0.2$.

Irrigation period T/3.

$(\alpha t/L^2)$		$h/(\frac{tL^2}{Kd})$	
0.0667		+0.0585	
0.2000		-0.1071	
0.2667		+0.1137	
0.4000		-0.1225	
0.4667		+0.1250	
	Total	+0.0676	
		(Compare with figur	e 8-4.)

The first figure represents the effect at the end of the last irrigation period which is 1/3 of a year in length. Then

$$(\alpha t/L^2) = (0.2/3) = 0.0667$$

The corresponding value for $h/(\frac{iL^2}{Kd})$ is obtained from figure 8-3. The next figure represents a cessation of irrigation at the end of the previous irrigation period. Here $(\alpha t/L^2) = 0.2000$ because the time is one year. The third figure with $(\alpha t/L^2) = (4) (0.2)/3 = 0.2667$ accounts for the beginning of the previous irrigation period. The fourth and fifth figures, together, account for the irrigations made two years previous. As the $(\alpha t/L^2)$ values grow larger the two values of the pair approach equality. This explains how convergence can be obtained even though the values are growing larger with time. A series of such computations will permit the construction of a chart such as shown on figure 8-4. With specified values of i , L , K , d , α , T an $(\alpha T/L^2)$ value can be computed and an $h/(\frac{iL^2}{Kd})$ value can be read directly from the chart which includes the effects of irrigations in previous years. An h value can then immediately be computed. A cut and try procedure for

estimating drain spacings can then be used which will take account of the effects of irrigations in previous years.

A direct approach to this problem can be made if it is noted that the graph of figure 8-4 is nearly a straight line. The straight line approximation shown has the formula

$$\frac{h_c}{(\frac{iL^2}{Kd})} = 0.040 + 0.1325 (\frac{\alpha T}{L^2}) \qquad \text{For } 0 < (\frac{\alpha T}{L^2}) < 0.4$$

From which, by rearrangement,

$$\left(\frac{L^2}{\alpha T}\right) = 25 \left(\frac{c}{iT}\right) - 3.3125$$
 (8-21)

The problem of determining a drain spacing for the conditions of the problem used to illustrate application of the first approximation solution of Chapter 8 may now be reconsidered. With

$$K = 10 \text{ ft/day}$$
 $V = 0.18$ $D_a = 22.225 \text{ ft}$ $K D_a = 222.25 \text{ ft/day}$ $\alpha = 1234.7 \text{ ft}^2/\text{day}$

Irrigation applications contributing 0.46 ft of drainable depth were made on June 6, July 1, July 21, August 4, August 18 and September 1. A similar contribution from snowmelt was indicated for April 22. In all (7) (0.46) = 3.22 feet of drainable depth were contributed in 132 days. To accommodate this to our chart conditions we can assume that these applications were made in 1/3 year or 122 days. Then

$$i = \frac{(3.22) (0.18)}{122} = 0.00475 \frac{ft}{day}$$

The allowable drainable depth at the center of the span is 4.0 ft and T=365 days. Then

$$\frac{\text{hV}}{\text{iT}} = \frac{(4.0) (0.18)}{(0.00475) (365)} = \frac{0.72}{1.734} = 0.4152$$

$$\frac{\text{L}^2}{\alpha \text{T}} = (25) (0.4152) - 3.3125 = 7.0675$$

$$\text{L}^2 = (7.0675) (1234.7) (365) = 3185100$$

$$\text{L} = 1785 \text{ feet}$$

This compares with Dumm's estimate of 1450 feet. The difference is largely due to a difference in assumed intervals between applications. In our case it was about 17 days whereas his last three irrigations were made at 14 day intervals. A corresponding infiltration rate would be

$$i = \frac{(3) (0.46) (0.18)}{(3) (14)} = 0.00591 \frac{ft}{day}$$

and

$$\frac{\text{hV}}{\text{iT}} = \frac{(4.0) (0.18)}{(0.00591) (365)} = \frac{0.720}{2.157} = 0.3338$$

$$\frac{\text{L}^2}{\alpha \text{T}} = (25) (0.3338) - 3.3125 = 5.0325$$

$$\text{L}^2 = (5.0325) (1234.7) (365) = 2265000 \text{ ft}^2$$

$$\text{L} = 1506 \text{ feet}$$

The ultimate steady state formula would give

$$L = \sqrt{\frac{8 \text{ h}_{c} \text{ Kd}}{i}} = \sqrt{\frac{(8) (4.0) (222.25)}{0.00591}} = 1097 \text{ feet}$$

This is admittedly too short for the reasons mentioned previously. The trial procedure described in the paragraph on "Selection of drain spacings" is much shortened if the trial value is close. Chart 8-4 and formula 8-21 can provide good trial values.

Flow of water to drains

The flow of drainage water from the width L between drains to the two drains bordering the width is:

$$q_2 = 2Kd \left(\frac{\partial h}{\partial x}\right)_0 = \frac{8iL}{\pi^2} \sum_{n=1,3,5...}^{n=\infty} \frac{\frac{-n^2\pi^2(\frac{\alpha t}{L^2})}{n^2}}{\frac{e}{n^2} \sum_{n=1,3,5...}^{n=\infty} \frac{e}{n^2}$$

Since the cosine terms which arise as a result of the indicated differentiations are 1 when x = 0. But

$$\frac{1}{1^2} + \frac{1}{3^2} + \frac{1}{5^2} + \dots = \frac{\pi^2}{8}$$

This is one of the Bernoulli numbers. Then

$$\frac{q_2}{iL} = 1 - \frac{8}{\pi^2} \sum_{n=1,3,5...}^{n=\infty} \frac{e}{n^2}$$
(8-22)

This can be put in the form:

$$\frac{q_2}{iL} = 1 - p \tag{8-23}$$

where values of p can be obtained from Table 11.

As an example of the use of this result we may compute the flow of drainage water from the width between drains using the data of an example from Chapter 8. It is worthwhile to note that the flow so obtained will be appropriate for the flow of drainage water to one drain from both sides. With

$$\alpha = 1222.23 \text{ ft}^2/\text{day}$$
 $(\frac{\alpha t}{L^2}) = 0.06627$
 $L = 1500 \text{ ft}$ $i = 0.00591 \text{ ft/day}$
 $t = 122 \text{ days}$

From tables

$$p = 0.4217$$

$$1 - p = 0.5783$$

Then

$$q_2 = iL(1-p) = (0.00591) (1500) (0.5783) = 5.1266 ft^2/day$$

This means that each drain must be able to pick up and carry away a little over five cubic feet of drainage water per foot of drain per day. This estimate can be expected to be below that obtained by the methods which account for the initial rush of water to the drains immediately following the application of irrigation water. Drains designed in this way could be expected to run at maximum capacity for a few days following irrigation.

Some comparisons will be found in the paragraph on "Local convergence losses." Local convergence losses

Where tile drains are used, the flow, which has been occupying the entire saturated depth, must converge toward the drain. This means that the flow

must pass through restricted areas and it must be expected that increased

head losses will be required to move the flow to the drain.

The following development has for its purpose the evaluation of these convergence losses.

Consider the expression (Byerly)

$$P_1 = \frac{P_0}{\pi} \log_e \left[\cosh^2 \left(\frac{\pi x}{d} \right) - \cos^2 \left(\frac{\pi y}{d} \right) \right]$$

where p represents the pressure needed to drive the flow. It is measured in feet of water. It is a solution of the Laplace equation

$$\frac{\partial^2 p_1}{\partial x^2} + \frac{\partial^2 p_1}{\partial y^2} = 0$$

By differentiation

$$\frac{\partial p_1}{\partial x} = \frac{p_0}{\pi} \left[\frac{2 \cosh \left(\frac{\pi x}{d}\right) \sinh \left(\frac{\pi x}{d}\right)}{\cosh^2 \left(\frac{\pi x}{d}\right) - \cos^2 \left(\frac{\pi y}{d}\right)} \right] \frac{\pi}{d}$$

Then

$$\frac{\partial p_1}{\partial x} = 0 \quad \text{if} \quad x = 0 \quad \text{when} \quad y > 0$$

By differentiation with respect to y

$$\frac{\partial p_1}{\partial y} = \frac{p_0}{\pi} \left[\frac{2 \cos \left(\frac{\pi y}{d}\right) \sin \left(\frac{\pi y}{d}\right)}{\cosh^2 \left(\frac{\pi x}{d}\right) - \cos^2 \left(\frac{\pi y}{d}\right)} \right] \frac{\pi}{d}$$

Then

$$\frac{\partial p_1}{\partial y} = 0 \quad \text{when} \quad y = d$$

$$= 0 \quad \text{when} \quad y = 0 \quad \text{if} \quad x > 0$$

The idealization is as shown in figure 8-5.

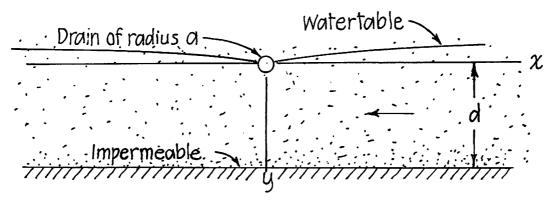


Fig. 8-5 Drain.

The origin is at the center of the drain and a sink is located there. The flow approaches from the right and goes to the sink without crossing the y axis. The idealization neglects the presence of saturated depth above the elevation of the drains. The tacit assumption is also made that the flow enters the drain through a quadrant. The quantity p_1 can be considered as the pressure which drives the flow. The flow approaches the drain along a strip of uniform width d. The pressure difference between the point y=0 and x and the point y=0 and y=0 and y=0 and y=0 and y=0

$$p_1 - p_3 = \frac{p_0}{\pi} \log_e \frac{\left[\cosh^2(\frac{\pi x}{d}) - 1\right]}{\left[\cosh^2(\frac{\pi x}{d}) - 1\right]}$$

The pressure gain out to x due to the uniform flow in the strip is:

$$p_2 = \frac{2p_0 x}{d}$$

The gradient, when x is large compared to d , can be inferred from the expression for $(\partial p_1/\partial x)$. When $(\pi x/d) >> 1$ then $\cosh (\frac{\pi x}{d})$ and $\sinh (\frac{\pi x}{d})$ become large compared to unity and nearly equal while $\cos^2 (\frac{\pi y}{d})$ can never exceed unity. Then when $(\pi x/d) >> 1$, $\frac{\partial p}{\partial x} \to 2$ p_0/d . The above expression for p_2 can be derived from this result. It represents the head loss which would be needed to drive the flow

$$Kd \frac{\partial p_1}{\partial x} = 2Kp_0$$

from the point x to the origin if there were no convergence.

The pressure loss due to convergence is:

$$[p_1 - p_2 - p_3] = \frac{p_0}{\pi} \log_e \left[\cosh^2 \left(\frac{\pi x}{d} \right) - 1 \right] - \frac{p_0}{\pi} \log_e \left[\cosh^2 \left(\frac{\pi a}{d} \right) - 1 \right] - \frac{2x}{d}$$

when $(\frac{\pi x}{d})$ is large compared to unity then $\cosh^2(\frac{\pi x}{d})$ will be large compared to unity and

$$\cosh^2\left(\frac{\pi x}{d}\right) - 1 \stackrel{\text{\tiny $=$}}{=} \cosh^2\left(\frac{\pi x}{d}\right) \quad (\text{If } \frac{x}{d} >> 1)$$

under these conditions also

$$\cosh^2\left(\frac{\pi x}{d}\right) = \frac{e^{\left(\frac{2\pi x}{d}\right)}}{4} \qquad (\text{If } \frac{x}{d} >> 1)$$

then approximately

$$\frac{p_0}{\pi} \log_e \left[\cosh^2 \left(\frac{\pi x}{d} \right) - 1 \right] = \frac{p_0}{\pi} \left(\frac{2\pi x}{d} \right) \log_e e - \frac{p_0}{\pi} \log_e 4 = \left[\frac{2x}{d} - 0.44127 \right] p_0$$

If (x/d) is large compared to unity the quantity 0.44127 can be dropped. If $(\pi a/d)$ is small compared to unity

$$\cosh^2 \left(\frac{\pi a}{d}\right) \stackrel{\sim}{=} 1 + \left(\frac{\pi a}{d}\right)^2 + \frac{1}{3} \left(\frac{\pi a}{d}\right)^4 + \dots$$

and approximately

$$[\cosh^2 \left(\frac{\pi a}{d}\right) - 1] \stackrel{\text{def}}{=} \left(\frac{\pi a}{d}\right)^2$$

so that

$$\frac{p_0}{\pi} \log_e \left[\cosh^2 \left(\frac{\pi a}{d} \right) - 1 \right] \stackrel{\text{def}}{=} \frac{2p_0}{\pi} \log_e \left(\frac{\pi a}{d} \right) .$$

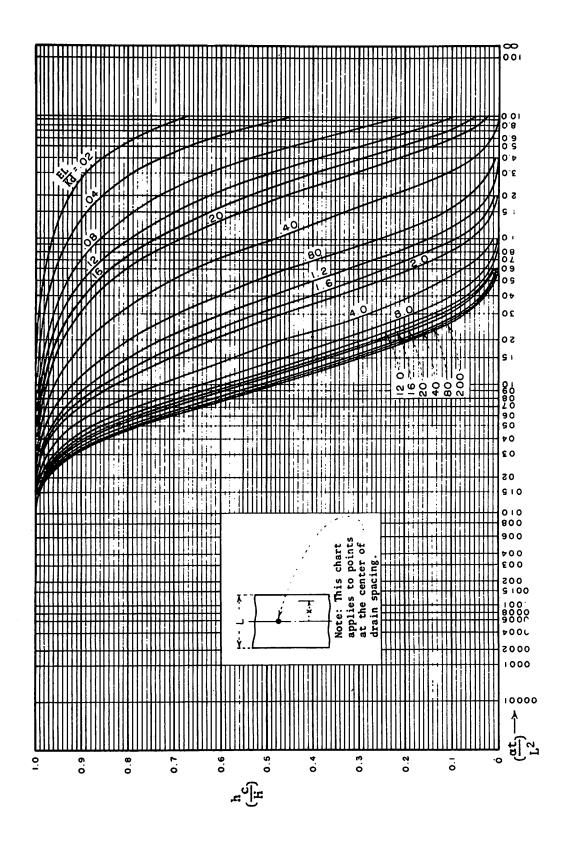
Finally

$$[p_1 - p_2 - p_3] = \frac{2p_0x}{d} - \frac{2p_0}{\pi} \log_e(\frac{\pi a}{d}) - \frac{2p_0x}{d}$$

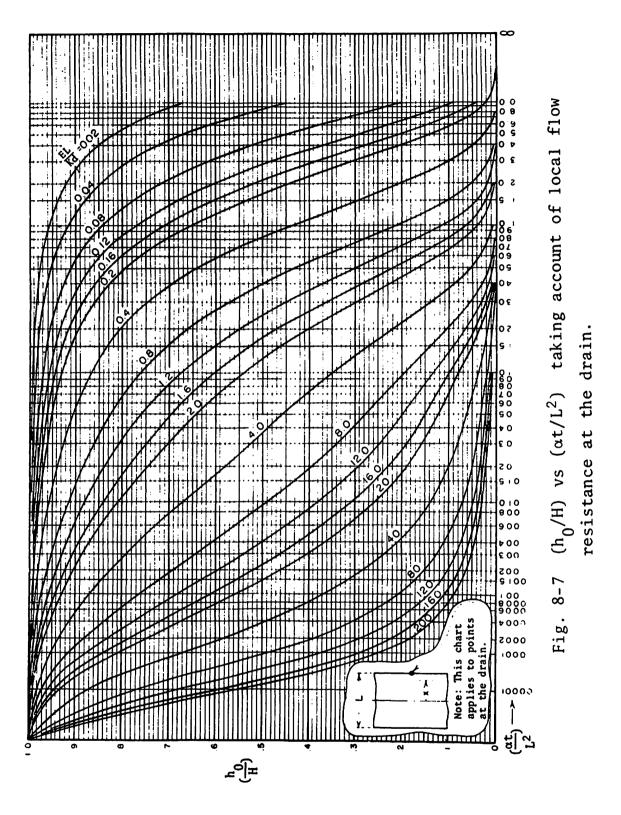
Figures 8-6 and 8-7 are reproduced here through the courtesy of the American Concrete Institute.

These figures illustrate the close relationship of the mathematical treatments of the flow of heat in solids and the flow of groundwater. These charts first appeared in a paper on "Insulation for Protection of New Concrete in Winter" by L. H. Tuthill, R. E. Glover, C. H. Spencer and W. B. Bierce in the Journal of the Concrete Institute for November 1951. The charts appear on pages 262 and 264.

The local resistance incident to converging flow to a drain here replaces the resistance to heat flow produced by form insulation.



 (h_c/H) vs $(\alpha t/L^2)$ taking account of local flow resistance at the drain. 9-8



or

$$[p_1 - p_2 - p_3] = \frac{2p_0}{\pi} \log_e (\frac{d}{\pi a})$$

We need a ratio expressing the flow rate and the head causing the flow. Set

$$E h_0 = q ag{8-24}$$

Where h_0 represents the head required to overcome the convergence losses and q represents the flow to unit length of drain from one side. Then:

$$E = \frac{q}{h_0} = \frac{2Kp_0}{\frac{2p_0}{\pi} \log_e(\frac{d}{\pi a})} = \frac{\pi K}{\log_e(\frac{d}{\pi a})}$$
 (8-25)

As an example of the use of this result the problem whose solution is given in the paragraph on "Selection of drain spacings" will be resolved taking the convergence losses into account. In so doing, the charts prepared for an analogous problem in the flow of heat will be used. With

K = 10 ft/day
$$\alpha = 1222.23 \text{ ft}^2/\text{day}$$

d = 22 ft Kd = 220 ft²/day
a = 0.5 ft L = 1500 ft
 $(\frac{d}{\pi a}) = \frac{22}{1.5708} = 14.006$ $(\frac{d}{\pi a}) >> 1$
 $\log_e 14.006 = 2.63949$
E = $\frac{\pi K}{\log_e (\frac{d}{\pi a})} = \frac{(3.1416)(10)}{2.63949} = 11.902 \text{ (ft/sec)}$
 $\frac{EL}{KD} = \frac{(13.843)(1500)}{220} = 81.152 \text{ (Dimensionless)}$

At the end of the irrigation season the depth of water at the drains is estimated in the following manner:

Application	Time Days	Drainable Depth (feet)	$(\frac{\alpha t}{L^2})$	$(\frac{h_0}{H})^*$	h ₀ (feet)
Apr 22*	132	0.46	0.0717	0.025	0.0115
June 6	87	0.46	0.0445	0.030	0.0138
July 1	62	0.46	0.0337	0.035	0.0161
July 21	42	0.46	0.0228	0.045	0.0184
Aug 4	28	0.46	0.0152	0.055	0.0253
Aug 18	14	0.46	0.0076	0.082	0.0377
Sept 1	0	0.46	0	1.000	0.4600
	365	4.00	0.1938	0.010	0.0400
					0.6228

^{*}Snowmelt

Then if water flows to the drain from both sides

$$2q = 2Eh = 2 (11.902) (0.6228) = 14.825 ft^2/day$$
.

This value is about three times as high as was obtained for this case by using the Maasland idealization. The reason for the difference is that the value computed above is a peak value whereas the Maasland value is in the nature of an average. It would be good engineering to design the drains to carry the peak flows since, otherwise, the computed drainage performance could not be obtained. It may be noted also that the additional cost of a slightly larger tile would be a small part of the cost of installing the drains.

It remains to assess the effect of the convergence losses upon the drainage performance. The following computation will provide drainable depth values at midspan which can be compared with similar values where the convergence loss was neglected.

^{**}From chart of figure 8-7.

Application	Time Days	Drainable Depth (feet)	$(\frac{\alpha t}{L^2})$	(<u>c</u>)	h (feet)
Apr 22*	132	0.46	0.0717	0.625	0.288
June 6	87	0.46	0.0445	0.825	0.380
July 1	62	0.46	0.0337	0.900	0.414
July 21	42	0.46	0.0228	0.960	0.442
Aug 4	28	0.46	0.0152	0.990	0.455
Aug 18	14	0.46	0.0076	1.000	0.460
Sept 1	0	0.46	0	1.000	0.460
	365	4.00	0.1938	0.210	0.840
					3.739

^{*}Snowmelt

A comparison of the results of this computation with a similar one in the paragraph on selection of drain spacings will show that the effect of local convergence on drain performance is not great. As would be expected, the computations show a slower drainage when local convergence losses are accounted for.

^{**}From chart of figure 8-6 with (EL/KD) = 81.2.

Return flows from irrigations

The idealizations described in this chapter can be adapted to the task of estimating the pattern of return flows supplied by deep percolations originating in irrigations. These return flows are often an important part of the water supply in irrigated areas. Figures 8-8 and 8-9 show how the conditions in a river valley can be correlated with the parallel drain idealization.

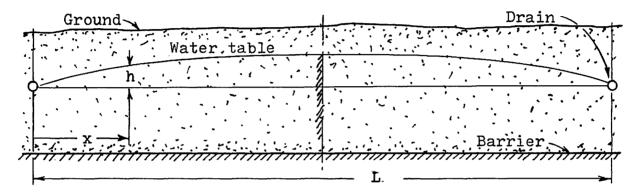


Fig. 8-8 Parallel drain idealization.

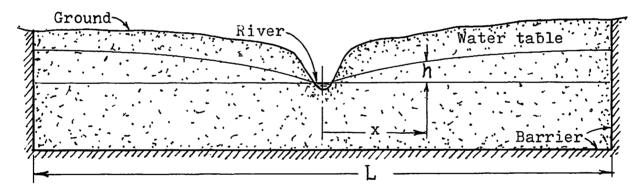


Fig. 8-9 Idealization of a river valley.

Because there is no flow across the line midway between drains, as shown in figure 8-8, the figure may be cut in two there and rearranged to bring the drains into coincidence, as shown in figure 8-9. Here the river replaces the drains. Use will be made of idealization 8-9 later. Mathematically, the idealizations of figures 8-8 and 8-9 are identical so long as L represents both the valley width and the drain spacing.

Chapter 8

Problems

(8-1) Drainage is needed in an irrigated area. Furrow irrigation is practiced and the deep percolation from each irrigation produces a drainable depth of 0.5 ft. Drainable increments originate on the following dates, the first of these being an accumulation of moisture received during the spring.

Date	Interval (Days)	Drainable Depth (feet)	Days to Sept 2
May 1	0	0.5	124
May 27	26	0.5	98
June 17	21	0.5	77
July 8	21	0.5	56
July 22	14	0.5	42
Aug 5	14	0.5	28
Aug 19	14	0.5	14
Sept 2	14	0.5	0
	124	4.0	

The drains are installed at a depth of 8 ft below ground surface and the water table is to be kept 4 ft below ground surface. The barrier is 28 ft below ground level. With a permeability of 15 ft per day estimate the drain spacing by use of formula 8-20 if V = 0.12. K = 15 ft/day KD = 300 ft²/day $\alpha = 2500$ ft²/day.

Answer: L = 1497 ft

(8-2) Estimate the required drain spacing by use of formula 8-21.

Answer: L = 1995 ft

(8-3) Check the suitability of a 2000 ft spacing using the method described in the paragraph on selection of drain spacings. As a basis for the computation use $\alpha = (Kd/V)$ as required by the Laplace type solution.

Answer: The water table height at the end of the irrigating season is 3.98 ft. This is satisfactory.

(8-4) If the drain of problem (8-3) is an 8-inch diameter tile bedded in a 4-inch thickness of gravel estimate, by use of formula 8-25, the approximate convergence loss factor E.

Answer: E = 20.9 ft/day.

(8-5) If the convergence loss h_1 of equation 8-24 were applied to drive the flow q through a length ℓ of the aquifer of depth d the equivalence could be explained by the relation

$$E h_1 = q = \frac{Kdh_1}{\ell}$$

Where & represents an equivalent length of aquifer then

$$\ell = \frac{Kd}{E}$$

By using this relation estimate how much the drain spacing of problem (8-3) would need to be shortened to compensate for the convergence loss incurred by the use of tile drains instead of open drainage canals.

Answer: $\ell = 14.4$ ft L = 2000 - (2) (14.4) = 1971 ft.

(8-6) Check this estimate by use of figure 8-6 and the procedure of problem (8-3).

Answer: $\frac{EL}{Kd}$ = 137. The water table height at the end of the irrigating season is 3.95 ft. This is satisfactory.

(8-7) By use of figure 8-7 compute the drainable depth at the drain and estimate the flow to the drains at the end of the irrigation season on September 2.

Answer: Drainable depth 0.606 ft. $q_1 = E h_1 = 12.7 \text{ ft}^2/\text{day}$ or 12.7 cubic ft per day per ft of drain from one side.

(8-8) Check this figure on the basis that the ultimate steady state has been reached and the drains must carry away the infiltration as fast as it is received.

Answer: $q_1 = \frac{iL}{2} = 4.22$ ft²/day or 4.22 cubic ft per day, per ft of drain, from one side. Transient conditions produce greater demands on drain capacity.

Chapter 9

Stream depletion due to a well

Formula 3-1 of Chapter 3 is appropriate where a well draws water, at the constant rate Q, from an aquifer of uniform properties and of infinite extent. The presence of a flowing stream may impose a condition of no drawdown along its course. If the course of the stream can be idealized as a straight line the condition of no drawdown can be imposed by use of an image well as explained in Chapter 10. In this case the image well is a recharge well of strength -Q and is located at the same distance from the stream as the pumped well and directly across the stream from it. The gradients imposed transverse to the stream by this combination can be computed from equation 3-1 and the flows produced by them can be summed along the whole stream length. In the mathematical sense this will be from $-\infty$ to $+\infty$. The depletion flow will be zero at time zero and will gradually rise toward Q as time increases.

The depletion of the stream by the well, computed in this way is given by the expression

$$\frac{q_1}{Q} = 1 - \frac{2}{\sqrt{\pi}} \int_0^{(\frac{x_1}{\sqrt{4\alpha t}})} e^{-u^2} du$$
 (9-1)

For a given case this can be evaluated by use of Table 8. Details of this development are given in the paper by Glover and Balmer 1954.

Example

A well is located one mile from a stream. The aquifer properties are $KD=0.270~\rm ft^2/sec~V=0.17~\alpha=1.59~\rm (ft^2/sec)$. It is desired to estimate what part of the flow of this well will be depleting the stream after the pumping has continued for three months.

With $x_1 = 5280$ ft = t (3)(2628000) = 7884000 seconds

$$\sqrt{4\alpha t} = \sqrt{(4)(1.59)(7884000)} = 7077 \left(\frac{x_1}{\sqrt{4\alpha t}}\right) = 0.746$$

From Table 8:
$$\frac{2}{\sqrt{\pi}} \int_{0}^{0.746} e^{-u^2} du = 0.70858$$

then

$$\frac{q_1}{Q} = 1 - 0.70858 = 0.29142$$

and the stream depletion at this time is about 29 percent of the well flow. If the well had maintained a flow of Q = 1.50 ft³/sec the stream depletion, at this time, would be

$$q_1 = (1.50)(0.29142) = 0.437 (ft^3/sec).$$

The pattern of stream depletion due to a well can be of interest. If equation 3-1 is written in the form:

$$s = \frac{Q}{2\pi KD} \int_{0}^{\infty} \frac{e^{-u^2} du}{u}$$

$$\frac{\sqrt{x^2 + z^2}}{\sqrt{4\alpha t}}$$

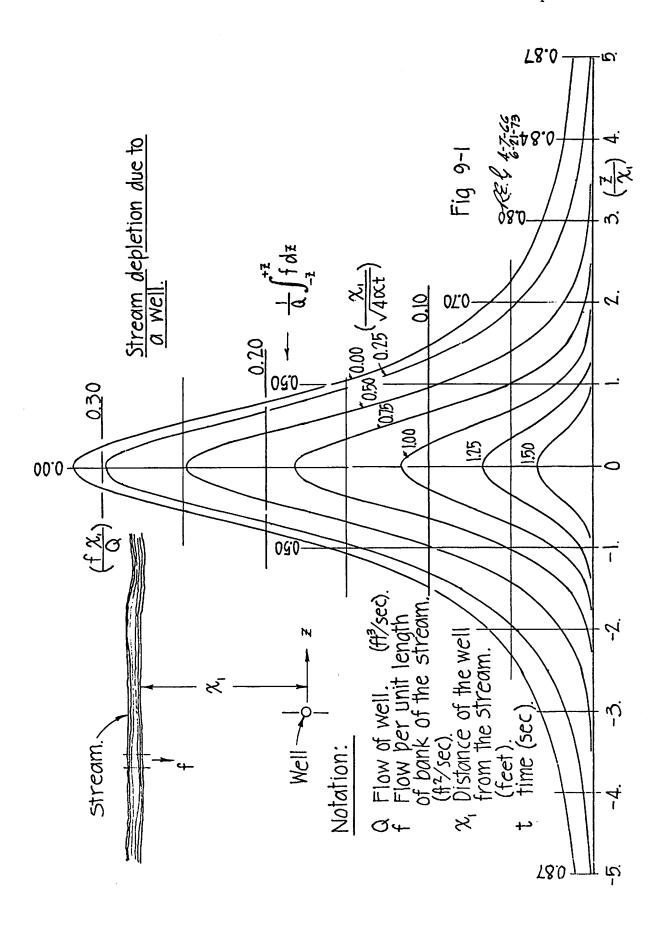
Then the cross stream gradient is

$$\frac{\partial s}{\partial x} = \frac{-Q}{2\pi KD} \frac{xe}{x^2 + z^2}$$

The image well will produce a similar gradient. Then the flow from the stream, per unit length of the stream, will be:

$$f = 2KD \frac{\partial s}{\partial x} = \frac{Q}{\pi x_1} \frac{x_1^2 e^{-(\frac{x_1^2 + z^2}{4\alpha t})}}{(x_1^2 + z^2)}$$
(9-2)

The chart of figure 9-1 has been prepared by use of this expression.



An ultimate steady state is reached when $(x_1/\sqrt{4\alpha t})$ becomes zero. Under these conditions the flow coming from the reach -z to +z is given by

$$\int_{-z}^{+z} f dz = \frac{Q}{\pi x_1} \int_{-z}^{+z} \frac{x_1^2 dz}{(x_1^2 + z^2)} = \frac{2Q}{\pi} \arctan \frac{z}{x_1}$$
 (9-3)

The figures on the chart which read vertically show values of this integral. When the ultimate steady state is reached, for example, one-half of the flow of the well will come from a reach of the river $2x_1$ long centered on the well. A similar reach $10x_1$ long will supply 87 percent of the well flow.

Chapter 9

Problems

(9-1) For the problem (3-1) of Chapter 3 estimate the stream depletion caused by this well if it is one-half mile from the river. Make the computation for the end of each six month interval out to the end of five years. Answer:

Time (months)	Stream depletion (ft ³ /sec)
0	0
6	0.705
12	0.841
18	0.905
24	0.943
30	0.970
36	0.990
42	1.005
48	1.017
54	1.028
60	1.036

(9-2) By using figure 9-1 or formula 9-2 make a plot of f vs z for time six months, based upon the conditions of problem 9-1.

Answer:

	${\bf f}$
(z/x_1)	(ft ² /sec)
0	0.0001249
1	0.0000539
2	0.0000138
3	0.0000033
4	0.0000007
5	0.0000001

Chapter 10

The use of images and treatment of intermittent operation

These two topics can well be treated together because both are concerned with procedures for extending the usefulness of basic types of solutions. The principle underlying these methods comes from the work of mathematicians. One of the fundamental questions to be settled relates to the conditions to be imposed if the solution of a differential equation is to be unique. Names associated with these investigations are those of Cauchy, Kowalewsky and Darboux (Cohen 1933). For our purposes the results of their investigations can be reduced to the following formulation. The solution of a differential equation is unique if:

- (a) The solution satisfies the differential equation.
- (b) The solution meets the appropriate boundary conditions.
- (c) The solution meets the appropriate initial conditions.

The procedures for using images and for accounting for intermittent operation are best illustrated by use of examples.

Example 1

It is desired to account for the influence of a nearby stream on the drawdown produced by pumping a well. It is supposed that the well draws water from an aquifer which underlies the stream and that the stream is able to maintain the water table level along its course. The basic solution 3-1 is appropriate for an aquifer of infinite extent from which the flow of the well Q is supplied from water initially stored in the aquifer. It may be noted, in passing, that the basic solution does not converge to an ultimate steady state but, instead, the water table level sinks continuously to supply the flow of the well.

The boundary condition imposed by the presence of the stream can be met if the stream location is represented by a straight line and a recharge well

is introduced on the far side of the stream and at a distance of x_1 from it. The hypothetical recharge well is to be located directly opposite the pumped well. Its recharge is to match the rate of discharge of the pumped well and the initiation of pumping and recharge are to be simultaneous. With this arrangement the drawdown produced by the pumped well along the course of the stream is everywhere annulled by the rise of the water table produced by the recharge well. It is to be understood that the recharge well has no real existence. It has the status of a mathematical ruse whose sole purpose is to accomplish a meeting of the boundary condition imposed by the stream. The solution, consisting of the algebraic sum of the drawdown of the pumped and recharge wells, is to be considered valid only on the side of the stream occupied by the pumped well.

For the conditions $x_1 = 1320 \text{ ft } (\frac{1}{4} \text{ mile})$ $\alpha = 1.50 \text{ ft}^2/\text{sec}$ $Q = 1.5 \text{ ft}^3/\text{sec}$ KD = 0.255 V = 0.17x = 100. ft y = 0 t = 15,768,000 seconds (6 months).

Then, for the pumped well

$$\left(\frac{Q}{2\pi KD}\right) = \frac{1.5}{(6.2832)(0.255)} = 0.9362$$

$$\left(\frac{r_1}{\sqrt{4\alpha t}}\right) = \frac{100}{\sqrt{(4)(1.5)(15768000)}} = 0.01028$$

From tables:

$$\int_{\frac{r_1}{\sqrt{4\alpha t}}}^{\infty} \frac{e^{-u^2} du}{u} = 4.28993$$

$$s_1 = (0.9362)(4.28993) = 4.016 \text{ ft}$$

For the image well

$$r_2 = 1320 + 1320 - 100 = 2540 \text{ ft}$$

$$(\frac{r_2}{\sqrt{3r_2}}) = \frac{2540}{9727} = 0.26113$$

From Table 1.

$$\int_{0}^{\infty} \frac{e^{-u^{2}} du}{u} = 1.08765$$

$$s_{2} = (0.9362)(1.08765) = 1.108 \text{ ft}$$

$$(\frac{r_{2}}{\sqrt{4\alpha t}})$$

Then the drawdown at this point, with the river present, will be 4.016 - 1.018 = 2.998 feet. If the river were absent the drawdown would be 4.016 feet.

The present situation does lead to an ultimate steady state since the stream provides a supply. The integral which appears in the drawdown formula can be developed into a series of the form:

$$\int_{0}^{\infty} \frac{e^{-u^{2}} du}{u} = -0.288608 - \log_{e} z + \frac{z^{2}}{1!2} - \frac{z^{4}}{2!4} + \frac{z^{6}}{3!6} - \dots$$

For the two wells the drawdown will be

$$s_{3} = s_{1} - s_{2} = \left(\frac{Q}{2\pi KD}\right) \left[-0.288608 - \log_{e} \frac{\binom{r_{1}}{\sqrt{4\alpha t}}}{\sqrt{4\alpha t}} + \left(\frac{r_{1}^{2}}{8\alpha t}\right) - \dots \right]$$

$$+ 0.288608 + \log_{e} \left(\frac{r_{2}}{\sqrt{4\alpha t}}\right) - \left(\frac{r_{2}^{2}}{8\alpha t}\right) + \dots \right]$$

As time increases the quantities $(\frac{r_1}{\sqrt{4\alpha t}})$ and $(\frac{r_2}{\sqrt{4\alpha t}})$ will become small compared to unity and all of the power terms will become negligibly small.

When this happens an ultimate steady state is reached in which

$$s_4 = (\frac{Q}{2\pi KD}) \log_e(\frac{r_2}{r_1})$$
 (10-1)

It will be of interest to compute the ultimate steady state drawdown at the chosen point.

With

$$(\frac{r_2}{r_1}) = \frac{2540}{100} = 25.40$$
 $\log_e 25.40 = 3.23475$
 $s_4 = (0.9362)(3.23475) = 3.028 \text{ feet}$

This is only slightly greater than the transient state drawdown computed for six months of pumping.

If the origin is taken at the point where the line between the wells intersects the idealized stream location and x is counted positive toward the recharge well while y is counted positive downstream then s_4 will be found to be a solution of the Laplace steady state equation:

$$\frac{\partial^2 s_4}{\partial x^2} + \frac{\partial^2 s_4}{\partial y^2} = 0$$

As another example we may consider the case of a well one mile away from the river of the previous example and inquire as to how much the presence of a shale barrier two miles back from the river, on the well side, will alter the stream depletion pattern produced by the well. If the aquifer extended back from the stream to a great distance the stream depletion due to the well would be given by the expression

$$\frac{q_1}{Q} = 1 - \frac{2}{\sqrt{\pi}} \int_{0}^{\frac{x_1}{\sqrt{4\alpha t}}} e^{-u^2} du$$

A computation of the stream depletion to be expected under these conditions is shown below.

Time (months)	$(\frac{x_1}{\sqrt{4\alpha t}})$	$(\frac{q_1}{Q})$
0	œ	0.0000
1	1.3297	0.0600
2	0.9402	0.1836
3	0.7677	0.2776
6	0.5428	0.4427
9	0.4432	0.5308
12	0.3838	0.5873
18	0.3134	0.6576
24	0.2714	0.7011
36	0.2216	0.7540
48	0.1919	0.7861
60	0.1717	0.8081

For one month, considered to be (1/12) year:

$$\left(\frac{x_1}{\sqrt{4\alpha t_1}}\right) = \frac{5280}{\sqrt{(4)(1.5)(2628000)}} = \frac{5280.}{3971.} = 1.3297$$

When the effect of the impermeable shale boundary is to be accounted for two boundary conditions must be met. In the above computation the condition that there would be no change in the water table level at the stream was met by use of a single recharge well image. The formula used recognizes this. The condition that there be no flow across the shale boundary must now be imposed. If we image the pumped well in the shale boundary there will be no flow if the image well is a pumped well. To meet the boundary condition of no change in water table level at the river we image both of the above wells in the line representing the course of the stream. These will be recharge wells. Introduction of the recharge wells will cause a flow at the line representing the shale boundary and to rectify

this we image the recharge wells in the shale boundary thereby doing a modicum of damage to the boundary at the stream. To counteract this we image the new wells in the stream boundary. Each change rectifies conditions at one boundary but disturbs them slightly at the other. Each change, however, improves the result. The series obtained is unending but convergent. The figures in the last column show what the depletion would be if the well were pumped for six months and then shut down. The values are obtained by superposition of $(\frac{q_1}{Q})$ values from the previous column. As an example, for time 12 months, $(q_1/Q) = 0.6838 - 0.4641 = 0.2197$. A comparison of the total (q_1/Q) values from the above table with the (q_1/Q) values of the previous table will show that the effect of the barrier begins to be discernible at the end of three months and at the end of 60 months (5 years) there is a very significant difference as will be seen by reference to table 10-1.

The case of a well in the corner between a stream and a tributary can be treated by use of images in the pattern shown below.

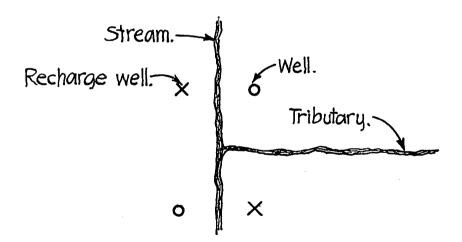


Fig. 10-1 Well in a corner.

The well is first imaged in the stream and then the well and its image are imaged in the line of the tributary. The solution is valid in the corner where the well is located. The circles represent pumped wells and the crosses represent recharge wells.

The problem of meeting the appropriate initial conditions arises when the effects of intermittent operation are to be evaluated. As in the previous treatment, examples provide the best approach to an understanding of the principles involved. Consider, for example, the case of a well which is pumped for six months and is then shut down for six months. It will be convenient to use the well of the preceding example. We must arrange the computation so that the solution satisfies the differential equation and meets the initial and boundary conditions. Suppose we try to meet these requirements by superimposing a recharge, at the pump location, which will nullify the flow of the well and compute the stream depletion thereafter as the algebraic sum of the pumped well and recharge well effects. If we check this proposed procedure against the requirements we find that the first requirement is met since the sum of two solutions of a linear differential equation is also a solution. The sum of the two solutions satisfies precisely the initial conditions at the time of change because the initial conditions are then those conditions which have been established by the pumped well. The boundary condition that the sum of the two solutions should conform to a condition of no well discharge is also met. Then we find that this sum of two solutions does meet the requirements for a unique solution as worked out by the mathematicians. Its application will disclose no shortcomings. A method for handling intermittent operations can then be outlined as follows. Where operations are intermittent each factor, once placed in operation, must be assumed to go on forever. New conditions are to be accounted for by superimposing additional solutions in such a way as to meet the appropriate initial and boundary conditions.

An example of these procedures can be supplied by estimating the stream depletion produced by the well of the previous problem if it is operated

continuously for six months and is then shut down. The result is shown in the last column of Table 10-1. These values are obtained by subtracting from each quantity in the "Total" column the value from the same column for a time six months less. For the first six months the values in the two columns are identical because there is no value to subtract. For the end of the seventh month the computation is made by subtracting from the seven month value the one month value, thus:

$$\left(\frac{q_1}{Q}\right) = 0.5098 - 0.0600 = 0.4498$$

The other values are found similarly. It may be noted that the stream depletion continues after the pump is shut down. One year after pumping started and six months after shutdown the stream depletion is still given by the ratio $(q_1/Q) = 0.2197$. The actual depletion at this time is:

$$q_1 = Q (0.2197)$$

This is still a little more than one-fifth of the pumping rate.

An important application of this principle is met when making computations of the return flows from irrigations. Operations studies are generally made in terms of acre foot volumes and with time intervals of months because records are commonly kept in these units.

To develop a computation procedure for this use we may start with the Maasland development of Chapter 8. The return flow from both sides of the valley is given by the expression:

$$\frac{q_2}{tL} = 1 - \frac{8}{\pi^2} \sum_{n=1,3,5,\dots}^{n=\infty} \frac{e^{-n^2 \pi^2 (\frac{\alpha t}{L^2})}}{n^2}$$

Table 10-1 Sample computation of the stream depletion due to operation of a well between a river and a barrier

Tab	le 10-1	Samp well	le c	ompu ween	tati a r	on c	of t		st:	rea	n d ier	ep]	let	ion	du	ie	to op	erati	on c	fa		
Six months	(-F) (-Q)	0.0000	0.2787	0.4641	0.4498	0.3104	0.2432	0.2197	0.1298	0.0991	0.0588	0.0446	0.0345	0.0160	0.0097	0.0061	0.0041	because we neglected lining wells at this long time	ì		wells	
Total	$\sqrt[6]{2}$	0.0000	0.2787	0.4119	0.5098	0.5891	0.6551	0.6838	0.8136	0.8557	0.9145	0.9342	0.9490	0.9771*	*8986.0	0.9929*	(1.0332)	because w		0	.Recharge wells	
well	to [∓] [o	000	000	00	00	000	00	00	0	0	0.0001	0.0002	0.0003	0.0014	0.0028	0.0049	0.0590		-River	X	pottern.	
Pumped well	x √ √ √ 4at	14.6267									2.8149	2,6705	2.5462	2.2570	2.1112	1.9905	1.8883		7	200	že mi	
well	 ∫o	000	000	000	00	00	0	00	00	0.0002	0.0006	0.0020	0.0032	0.0090	0.0146	0.0213	0.0289		Barrier-	×	Pumped Well.— Image	
Pumped well	$\sqrt[x]{\frac{x}{4\alpha t}}$	 11.9673							2.8207	2.6115	2.3031	2.1850	2.0833	1.8467	1.7274	1.6286	1.0925			0	Pump	
recharge	le <u>r</u> jo	000	000	00	00	000	00	0.0001	0.0019	0.0041	0.0072	0.0163	0.0220	0.0422	0.0574	0.0732	0.2294					
Second recharge	$\sqrt[4]{1}$	9,3079				7 6424	2.8064	2.6870	2.1939	2.0312	1.7913	1.6994	1.6203	1.4363	1.3435	1.2667	0.8497		9.3079		9673	14.6267
charge	$\begin{pmatrix} x_1 \\ \sqrt{4\alpha t} \end{pmatrix} \begin{pmatrix} q_1 \\ \sqrt{Q} \end{pmatrix}$	000	000	0	0.0004	0.0017	0.0046	0.0067	0.0257	0.0402	0.0550	0,0860	0.1020	0.1468		0.2007	0.3907	tly.	6		= 11.9673	0
First re	√4at	6.6485		2.7142	2.5129	2.2126	2.0046	1.9193	1.5671	1.4508	1.2795	1,2139	1.1573	1.0259	9656.0	0.9048	0.6069	Additional images would decrease these values slightly	(7) (5280)	Mat ₁	(9) (5280) /4at ₁	(11) (5280) /4¤t ₁
Image in	$\frac{x_1}{4\alpha t}$ $\frac{q_1^{4}}{q_0^{1}}$	000	0.0011	0.0116	0.0330	0.0600	0.0889	0.1034	0.1836	0.2183	0.2776	0.3030	0.3261	0.3840	0.4155	0.4426	0.6093	these val				
Imag Shale	4at	3.9891	2.3031	1.7840	1.5078	1.3297	1.2028	1.1516	0.9402	0.8705	0.7677	0.7283	0.6944	0.6156	0.5758	0.5429	0.3614	decrease	1.3297		3,9891	6.6485
Pumped well	(o	0.0600	0.2776	0.4003	0.4772	0.5308	0.5708	0.5872	0.6576	0.6815	0.7174	0.7313	0.7434	0.7717	0.7861	0.7980	0.8637	es would	u			n.
Pumpec	$\sqrt[4]{\int_{4\alpha t}^{x}}$	1.3297	0.7677	0.5947	0.5026	0.4432	0.4009	0.3839	0.3134	0.2902	0.2559	0.2428	0.2315	0.2052	0.1919	0.1810	0.1214	onal imag	(1) (2280)	/4at	(3) (5280) /4at ₁	(5) (5280) /4at ₁
Time	S S	0+0	1 W 4	o 0	r 00	9 5	11	12	18	21	27	30	33	42	48	24	120	Additi				

The total return is given by the relation:

$$R = \frac{0^{\int_{1}^{t} q_{2} dt}}{iLt} = 1 + \frac{8}{\pi^{4}} \left(\frac{L^{2}}{\alpha t}\right) \sum_{n=1,3,5...}^{n=\infty} \frac{e^{-n^{2}\pi^{2}} \left(\frac{\alpha t}{L^{2}}\right)}{n^{4}} - \frac{1}{12} \left(\frac{L^{2}}{\alpha t}\right)$$
(10-2)

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In obtaining this result use has been made of the identity

$$\frac{1}{1^4} + \frac{1}{3^4} + \frac{1}{5^4} + \dots = \frac{\pi^4}{96}$$

Values of the R function may be found in Table 12.

The following developments will be difficult to follow unless some explanation is made concerning purposes, methods and details of procedures. The purpose will be to develop a set of factors which can be used to estimate return flows from waters reaching the water table in a valley traversed It is supposed that the valley floor is composed of permeable by a stream. sediments occupying a trench eroded in less permeable materials, as shown by figure 8-9, that the valley is of a uniform average width L and that the river follows a course down the middle of the valley. It is further supposed that the permeable sediments of the valley floor are of a uniform character whose transient ground water flow characteristics can be expressed by a single value of the aquifer constant α . It is intended that the factors obtained will have, for a given valley, a permanent usefulness so that they can be put into the memory of a digital computer, for example, for the purpose of estimating return flows needed for operations studies and that these factors will be valid for any amounts reaching the water table in any months preceding the month for which the return flow is to be estimated. Following generally accepted procedures the quantities derived will be expressed in volumes accruing during a month. The factors will be derived for a unit volume (iLt₁) = 1 reaching the water table during a one month period t_1 .

The factors will be useful for estimating stream depletions due to distributed pumping by considering the amounts removed by the pumps as a negative infiltration. It is supposed that the infiltrations reaching the water table or the depletions due to pumping can be idealized as being uniformly distributed over the width of the valley and, in order to facilitate the computation work, a month will be assumed to be a uniform (1/12) year. It will be recognized that some fraction of the water applied will reach the water table but for convenience of computation the whole amount of water applied may be assumed to reach the water table and the appropriate fraction applied to the final result. This procedure was followed in the preparation of Tables 10-3 and 10-4.

Factors could be obtained for any number of prior months but after some time has passed the total return flow from earlier months will be small. To expedite computations it will, therefore, be found desirable to limit the computation of individual return flows to a selected number of months and to approximate the return flow from all previous months by applying a residue factor to an average monthly increment reaching the water table, or, in the case of pumping, removed from it. In the computation shown in Table 10-3 return flows are computed for 48 months.

The residue is the difference between the sum of the factors and unity. In the example given the sum of factors is 0.98714 and the residue is 1 - 0.98714 = 0.01286. This is the residue factor to be applied to an average monthly increment. In the example, the average monthly increment is arbitrarily obtained from the year 1954 and is 27,350 acre feet per month. The contribution for all months before October 1954 to the return flow for September 1958 is therefore estimated to be (27350)(0.01286) = 352. acre feet. This is added in as shown in Table 10-4.

Where pumping is practiced in areas served by surface diversions a composite return flow fraction can be specified. Where there is considerable variation of valley width the valley length can be divided into reaches, a set of factors derived for each reach, and these combined later to yield a single set of factors as described above. It will be found that where application times run forward, as shown in Table 10-3 the factors obtained from Table 10-2 must be reversed in order to match the appropriate factor to the increment reaching the water table during a previous month. In the computation of Table 10-3, for example, the return flow for September is (60300)(0.14140) = 8526 acre feet. The factor 0.14140 is the factor for month 1 of Table 10-2. The estimate of return flow for August is (82100)(0.11720) = 9623 acre feet. The factor 0.11720 is the factor for month 2 of Table 10-2. Similarly, the return for September coming from the July application should be obtained by applying to it the factor 0.07628 of Table 10-2 for the third month. Having outlined the purposes to be served and the methods to be used the detail of the development of the set of factors can now be approached.

What is needed is a sequence of factors representing the return flow during some selected month due to application of irrigation water at a unit rate in the first month. To get this sequence we proceed in the manner shown in Table 10-2 below. For the sake of brevity only a computation for the first 12 months is shown. A month is considered to be (1/12) year or 2,628,000 seconds. This interval is here represented by the symbol t_1 .

The column "R times months" represents the total return from the accumulation of infiltration at the rate $iLt_1 = 1$ at the stated time. The application is here supposed to be continuous out to the stated time. The "First difference" represents the total return, accumulating out to the stated time, from the application $iLt_1 = 1$ made during the first month

only. The "Second difference" represents the return flow accumulating during the stated month from the unit application iLt₁ = 1 made during the first month only. The return flow accumulating during some specified month due to an accumulation of infiltration during the first month is found by multiplying the accumulated quantity by the appropriate factor from the "Second difference" column. Since the factors in the "Second difference" column are dimensionless the product will take the units used to express the accumulation. Then if N acre feet of water reached the water table in month 1 the return flow from it accumulating during the month selected from the "Time-months" column of Table 10-2 will be N times the corresponding factor from the "Second difference" column. It is implied that the unit quantity of water reaching the water table during month 1 accumulates at a uniform rate during month 1.

Table 10-2

Partial return in some specified month due to application of water in a previous month

Time months	$(\frac{\alpha t}{L^2})$	R	R times months	First difference	Second difference
0	0	0	0		
Ĺ	0.0088375	0.1414034	0.1414034	0.1414034	0.14140
2	0.0176750	0.2000021	0.4000042	0.2586008	0.11720
3	0.0265125	0.2449622	0.7348867	0.3348824	0.07628
4	0.0353500	0.2828602	1.1314409	0.3965542	0.06167
5	0.0441874	0.3162288	1.5811440	0.4497031	0.05315
6	0.0530249	0.3463434	2.0780605	0.4969166	0.04721
7	0.0618624	0.3739350	2.6175448	0.5394843	0.04257
8	0.0706999	0.3994691	3.1957527	0.5782079	0.03872
9	0.0795374	0.4232571	3.8093137	0.6135610	0.03529
10	0.0883749	0.4455212	4.4552115	0.6458978	0.03234
11	0.0972124	0.4664286	5.1307150	0.6755035	0.02960
12	0.1060498	0.4861115	5.8333385	0.7026234	0.02712
Note:	For one month	with $\alpha = 1.9$	$50 \text{ ft}^2/\text{sec}$ L =	21120 ft	

Note: For one month with $\alpha = 1.50 \text{ ft}^2/\text{sec}$ L = 21120 ft.

$$\left(\frac{\alpha t_1}{L^2}\right) = \frac{(1.50)(2628000)}{(21120)^2} = 0.008837487$$

The R values are obtained from Table 12 by linear interpolation. The "R times months" values are obtained by multiplying the R values by the values in the "Time months" column at the left. The difference between successive values in the "R times months" column is shown as the first difference. The "Second difference" values are obtained in the same way from the "First difference" values. The "Second difference" values are given to only five places since linear interpolation and loss of figures by differencing will not justify more places.

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An example of a return flow computation is shown in Table 10-3 below. The irrigation water applied is shown for each month for the years 1954 to 1958 inclusive. The "Factors" come from Table 10-2. They will be seen to be in reverse order for the reason previously explained. The factor for month 1 is being applied to the irrigation volume used during September 1958. The "Sum of products" is the sum of the products of the "Factor" and the "Irrigation water applied" in the adjacent column. Since the volumes of water applied are expressed in acre feet and the "Factor" is dimensionless the "Sum of products" represents a volume, of return flow, in acre feet. When the estimated 352 acre feet correction is added to the "Sum of products" it represents the return flow which would have accumulated during the month of September 1958 if all of the irrigation water applied during all previous years had reached the water table.

Table 10-3 Computation of return flow for September 1958

 $\alpha = 1.50 \text{ ft}^2/\text{sec}$

L = 21120 ft

Year		Irrigation water applied A.F.	Factor	Year	Irrigation water applied A.F.	Factor
	Jan.	0		Jan.	300	.01234
		2300		3 43.1.1	2100	1350
		2100			10300	1468
		33100			7200	1610
		52600			19900	1750
1954		49000		1957	92700	1915
		43700			114900	2084
		45500			106300	2279
		37500			83200	2480
		33700	.00119		21700	2712
		27300	127		2100	2960
	Dec.	1400	141	Dec.	1100	3234
	Jan.	500	148	Jan.	0	35 29
		0	170	oui.	Ö	3872
		4900	180		Ö	4257
		36200	200		1800	4721
		48500	214		49700	5315
1955		32800	239	1958	86200	6167
		41300	254		79500	.07628
		46800	282		82100	.11720
		35800	304	Sept.	60300	.14140
		40400	337	•	20200	
		5500	362		2300	
	Dec.	0	401	Dec.	0	
				Sum of		
	Jan.	0	432	products	45288	.98714
		0	475			
		4500	514			
		29800	566			
		44000	614			
1956		53700	674			
• •		42300	730			
		51100	801			
		36100	869			
		33600	.00955			
		1900	.01035			
	Dec.	700	1137			

Notes:

1 - 0.98714 = .01286 (Residue) Average month (1954) is 27350 A.F.

Estimate of contribution from years 1954 and before: (27350)(.01286) = 352. A.F.

Similar computations for each month provide the data for the following table. The sum of products shown at the bottom of Table 10-3 would be the return flow due to application of irrigation water from October 1954 to September 1958 if all of the applied water reached the water table. Allowances must be made for application prior to October 1954 and for the irrigation water consumed. The following table shows the return flows estimated on the basis that one-third of the applied water is consumed and two-thirds returns to the water table.

Table 10-4 Summary of estimated return flows - 1958

Month	Sum of products A.F.	Correction A.F.	Total A.F.	Return flow A.F.			
Jan.	27440	352	27792	18528			
Feb.	24770	352	25122	16748			
Mar.	22541	352	22893	15262			
Apr.	20804	352	21156	14104			
May	25985	352	26337	17558			
June	35257	352	35609	23739			
Ju1y	40874	352	41226	27484			
Aug.	44930	352	45282	30188			
Sept.	45288	352	45640	30427			
Oct.	40003	352	40355	26903			
Nov.	33770	352	34122	22748			
Dec.	29279	352	29631	19754			
		Total for the year					
	21954						

Average monthly return flow

A comparison of quantities in Tables 10-3 and 10-4 will show that while the applications ordinarily go to zero in the winter the return flows continue throughout the year at a nearly steady rate. The return flows peak in September at about 30,000 acre feet per month and reach a minimum of about 14,000 A.F./mo in April. Deviations from the average of about 22,000 A.F./mo reach only about 8000 A.F./mo. This is due to the equalizing effect of the ground water reservoir.

Chapter 10

Problems

(10-1) From the data of problem (9-1) estimate the stream depletion at the end of each six months for a seasonal operation in which the pump is on for each six months of the growing season and off for each six months of the nongrowing season.

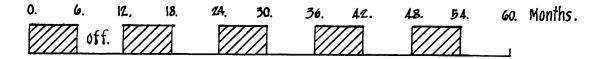


Fig. 10-2 Pumping pattern.

Answer:

Time (months)	Depletion rate (ft ³ /sec)	Time (months)	Depletion rate (ft ³ /sec)
0	0	36	0.207
6	0.752	42	0.865
12	0.145	48	0.220
18	0.820	54	0.876
24	0.186	60	0.230
30	0.848		

(10-2)

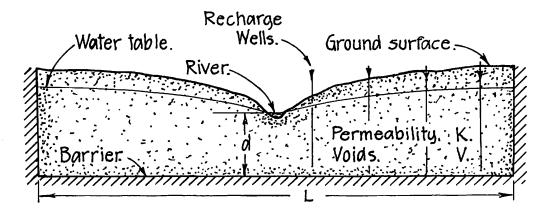


Fig. 10-3 Cross section of a river valley.

The cross section of a drained strip shown on fig. 8-3 is symmetrical about the center, and has a zero gradient there, so the center line is a line

of no flow. The flow pattern would be unchanged, therefore, if the section were cut in two and the two parts rearranged to bring the drains into coincidence, as shown above, to give an idealization of conditions where a river has back-filled a pre-existing trench, in an impermeable formation, with permeable alluvial sediments. This situation occurs frequently. Here the river represents the drains which collect return flow supplied by deep percolation from irrigations when the valley floor is farmed.

It should be possible to make an independent approximate check of the validity of formula 8-23 by replacing the uniformly distributed recharge by four recharge wells, as shown on the above figure. Since the two halves are symmetrical the computation can be made for each half separately. Check the point for $(\alpha t/L^2) = 0.1$ by using the method of images.

Answer: The images in the barrier, of the first eight images in the river, must be included.

$$\left(\frac{q_2}{iL}\right) = 0.696$$
 From Maasland's method 0.698

(10-3) Suppose the reservoir of problem (6-1) had been in service for five years and then was emptied to supply water during a drought. If the drawdown can be idealized as a drop of 10 feet at the beginning of each of the first four months of a six month irrigation period estimate the rate of ground water return flow at the end of the irrigation period. Assume that the reservoir was kept filled to an average 40 foot depth before the beginning of the irrigation period.

Answer: $24.0 \text{ ft}^3/\text{sec}$.

(10-4) If the above reservoir was an off stream reservoir whose bottom was well above the water table would there be any ground water return on drawdown?

Answer: No.

(10-5) For the case of problem (10-3) estimate the total ground water return during the irrigating season.

Answer: 13,860 acre feet.

Chapter 11

Effects of ground water drift

Comments

The reason for considering the effects of ground water movement under the action of a regional gradient is not that the effects are generally important but because administrators have been and can be confronted with objections which, in the absence of definitive answers, can be very perplexing. How, for example, does one answer the objection of a man who has been denied a permit to drill a well because the water supply is overincumbered when he points out that the water table in his area has a gradient of perhaps ten feet to the mile and that the cones of depression which would result from the operation of his well would be continually swept away by the ground water drift caused by the aquifer gradient and that his proposed well could, therefore, not run out of water?

Effect of ground water drift on drawdown

The ground water drift under consideration is that caused by a regional gradient which maintains a steady movement of the ground water below any fixed point on the ground.

The drawdown due to removal of the quantity of water W at r=0 when t=0 is given by the expression

$$s = \frac{-\left(\frac{r^2}{4\alpha t}\right)}{4\pi KDt}$$
 (11-1)

This is a solution of equation 2-4 which satisfies the condition s=0 when t=0 for r>0 and the above requirement. Such a depression, once created, would move with the water in the aquifer at the drift rate

$$f = \frac{K\gamma}{V} \tag{11-2}$$

Where γ represents the regional gradient. The factor V accounts, approximately, for the increased velocity caused by movement through the interstices instead of the gross area. Consider a well located as shown in figure 11-1 below

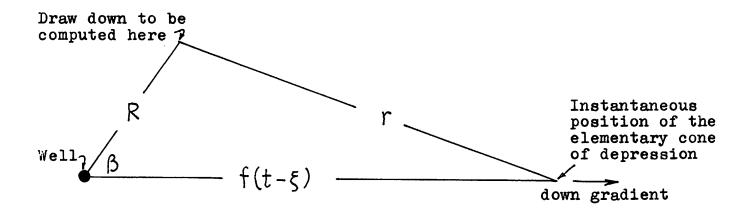


Fig. 11-1 Positions

Let ξ represent a time variable running between 0 and t. Consider ξ to represent the time at which an event occurs and t to be the time at which the effect of the event is to be computed. If, during the time interval d ξ at time ξ , the well removes the quantity of water dW = Qd ξ then the elementary cone of depression caused by this removal will be:

$$ds = \frac{Qd\xi e^{-\frac{r^2}{4\alpha(t-\xi)}}}{4\pi KD(t-\xi)}$$

If the point at which the drawdown is to be computed is defined by the radius R and the angle β measured from the direction of drift then, by the cosine law:

$$r^2 = R^2 + f^2(t-\xi)^2 - 2Rf(t-\xi) \cos \beta$$

and

$$\frac{\mathbf{r}}{4\alpha(\mathbf{t}-\xi)} = \frac{\mathbf{R}^2 + \mathbf{f}^2(\mathbf{t}-\xi)^2 - 2\mathbf{R}\mathbf{f}(\mathbf{t}-\xi)\cos\beta}{4\alpha(\mathbf{t}-\xi)}$$

or

$$\frac{\mathbf{r}}{4\alpha(\mathbf{t}-\xi)} = \frac{R^2}{4\alpha(\mathbf{t}-\xi)} + \frac{\mathbf{f}^2(\mathbf{t}-\xi)}{4\alpha} - \frac{R\mathbf{f} \cos\beta}{2\alpha}$$

The drawdown at the radius R from the well can then be expressed as

$$s = \int_{0}^{t} \frac{R^2}{4\alpha(t-\xi)} - \frac{f^2(t-\xi)}{4\alpha} + \frac{Rf \cos \beta}{2\alpha}$$

or

$$s = \frac{Q e^{\frac{Rf \cos \beta}{2\alpha}}}{4\pi KD} \int_{0}^{t} \frac{e^{-\frac{R^2}{4\alpha(t-\xi)} - \frac{f^2(t-\xi)}{4\alpha}}}{(t-\xi)}$$

Let

$$u^{2} = \frac{f^{2}(t-\xi)}{4\alpha} \cdot \frac{1}{(t-\xi)} = \frac{f^{2}}{4\alpha u^{2}} \cdot 2udu = -\frac{f^{2}d\xi}{4\alpha} \cdot d\xi = \frac{-8\alpha udu}{f^{2}} \cdot \frac{1}{4\alpha u^{2}} \cdot \frac{1}{(t-\xi)} = \frac{f^{2}d\xi}{4\alpha u^{2}} \cdot \frac{1}{(t-\xi)} = \frac{f^{2}d\xi}{4\alpha$$

By substitution

$$s = \frac{\frac{Rf \cos \beta}{2\alpha}}{4\pi KD} \int_{\frac{f^2t}{4\alpha}}^{0} e^{-u^2 - \frac{1}{4}(\frac{Rf^2}{2\alpha})\frac{1}{u^2}} du$$

or

$$s = \frac{\frac{Rf \cos \beta}{2\alpha}}{2\pi KD} \int_{0}^{\frac{f^{2}t}{4\alpha}} \frac{e^{-u^{2} - \frac{1}{4}(\frac{Rf}{2\alpha})^{2} \frac{1}{u^{2}}}}{u}$$
(11-3)

The integral which appears here can be evaluated by use of Table 14.

It is now possible to evaluate the effect of a regional gradient. The following example will illustrate: Suppose a well is operated for 25 years at the average rate of 1.0 ft 3 /sec. The regional gradient in the area is 22 feet per mile or $\gamma = 0.004167$.

Aquifer properties are:

D = 120 ft K = 0.001 ft/sec KD = 0.120 ft²/sec
V = 0.15
$$\alpha$$
 = 0.80 ft²/sec

The drift rate is:

$$f = \frac{K\gamma}{V} = \frac{(0.001)(0.004167)}{0.15} = 27.78(10)^{-6} \text{ft/sec or } 876.0 \text{ ft/yr.}$$

$$\frac{Q}{2\pi KD} = \frac{1.00}{(6.2832)(0.120)} = 1.326 \text{ ft}$$

Twenty five years is 788,400,000 seconds. The drawdown at 1320 ft or 1/4 mile from the well is to be found.

If the water table is level

$$\left(\frac{\mathbf{r}}{\sqrt{4\alpha t}}\right) = \frac{1320}{\sqrt{(4)(0.8)(788400000)}} = 0.026$$

$$s = \frac{Q}{2\pi KD} \int_{-\infty}^{\infty} \frac{e^{-u^2} du}{u} = (1.326)(3.350) = 4.44 \text{ ft}$$

$$\left(\frac{\mathbf{r}}{\sqrt{4\alpha t}}\right)$$

For the aquifer with drift

$$\frac{(\frac{Rf}{2\alpha})}{(\frac{Rf}{2\alpha})} = \frac{(1320)(27.78)(10)^{-6}}{(2)(0.8)} = 0.02292 \quad m = (\frac{Rf}{4\alpha}) = 0.01146$$

$$e^{(\frac{Rf}{2\alpha})} = 1.02318 \quad e^{-(\frac{Rf}{2\alpha})} = 0.97734 \quad m^2 = (\frac{Rf}{4\alpha})^2 = \frac{1}{4}(\frac{Rf}{2\alpha})^2$$

$$\sqrt{\frac{f^2t}{4\alpha}} = \sqrt{\frac{(27.78)^2(10)^{-12}(0.788)(10)^9}{3.2}} = 0.4360$$

From Table 14
$$(\frac{Rf}{4\alpha}) \qquad \int_{0}^{\infty} \frac{e^{-u^2 - (\frac{Rf}{4\alpha})^2 \frac{1}{u^2}} du}{u} \int_{0.430}^{\infty} \frac{e^{-u^2 - (\frac{Rf}{4\alpha})^2 \frac{1}{u^2}} du}{u} \int_{0.430}^{0.430} \frac{e^{-u^2 - (\frac{Rf}{4\alpha})^2 \frac{1}{u^2}} du}{u} \int_{0.430}^{0.430} \frac{e^{-u^2 - (\frac{Rf}{4\alpha})^2 \frac{1}{u^2}} du}{u} \int_{0.440}^{0.440} \frac{e^{-u^2 - (\frac{Rf}{4\alpha})^2 \frac{1}{u^2}} du}{u} \int_{0.436}^{0.436} \frac{e^{-u^2 - (\frac{Rf}{4\alpha})^2 \frac{1}{u^2}} du}{u} \int_{0.436}^{0.430} \frac{e^{-u^2 - (\frac{Rf}{4\alpha})^2 \frac{1}{u^2}} du}{u} \int_{0.436}^{0.430} \frac{e^{-u^2 - (\frac{Rf}{4\alpha})^2 \frac{1}{u^2}} du}{u} \int_{0.430}^{0.440} \frac{e^{-u^2 - (\frac{Rf}{4\alpha})^2$$

s = (1.326)(3.29538) = 4.370 ft. Computation for the upstream and downstream points is made as follows:

$$β$$
 $cosβ$ $e^{(\frac{R£}{2α})cosβ}$ s

0 1.000 1.02318 4.471

 $π/2$ 0 1.00000 4.370

 $π$ -1.000 0.97734 4.271

It may be noted that the drawdown values computed for the aquifer with and without drift are almost the same. During the entire 25 year period the drift has been only (876.0)(25) = 21900 feet or about four miles. The elementary cones of depression which left the well at the beginning of the

pumping period are now about this far from the well but they have long since been filled in. The only active cones are the young ones near the well and it is to be expected, therefore, that the drawdown patterns with and without drift would be closely similar.

Effect of ground water drift on stream depletion caused by pumping

In an irrigated river valley surface diverters convey water in their canals to the outer edge of the valley to irrigate the lands between the canal and the river. Deep percolation losses from the applied water reach the water table and cause a flow of ground water back to the stream. This is the "return flow" which is a common feature of irrigation projects. If irrigation by pumping is also practiced the wells will intercept some of the return flow and reduce the flow of the stream below what it would be if the pumping were not done. The question arises as to whether the ground water drift influences the depletion pattern sufficiently so that it should be taken into account in making estimates of stream depletion.

Evaluation of the effect of the drift can be made as before by summing the effects of the elementary cones of depression which are created by the well and are thereafter carried along with the ground water drift. The drawdowns produced by the removal of the quantity of water W at the radius r at the time t is, as given previously

$$s = \frac{\frac{-(\frac{r^2}{4\alpha t})}{e}}{\frac{4\pi KDt}{}}$$

Let x and y represent rectangular coordinates measured, respectively, toward and along the stream. Then

$$r^2 = x^2 + v^2$$

By differentiation

$$\frac{\partial \mathbf{r}}{\partial \mathbf{r}} = \frac{\mathbf{r}}{\mathbf{x}}$$

The cross stream gradient is:

$$\frac{\partial s}{\partial x} = \frac{\partial s}{\partial r} \frac{\partial r}{\partial x} = \frac{-2Wxe}{16\alpha\pi KDt^2}.$$

The flow into the bank, per unit length of stream is, when $x = x_1$

$$- KD \frac{\partial s}{\partial x} = \frac{2Wx_1e}{16\alpha\pi t^2}$$

where x_1 represents the distance from the well to the stream. The total flow is:

$$\int_{-\infty}^{+\infty} KD \frac{\partial s}{\partial x} dy = \frac{2Wx_1 e^{-(\frac{x_1^2}{4\alpha t})}}{16\alpha\pi t^2} \int_{-\infty}^{+\infty} e^{-(\frac{y^2}{4\alpha t})} dy$$

Let

$$u^2 = (\frac{y^2}{4\alpha t})$$
 dy = $\sqrt{4\alpha t}$ du

Then

$$\int_{-\infty}^{+\infty} e^{\frac{y^2}{4\alpha t}} dy = \sqrt{4\alpha t} \int_{-\infty}^{+\infty} e^{-u^2} du = \sqrt{4\alpha t} \sqrt{\pi}$$

and the total flow is:

$$\int_{-\infty}^{+\infty} KD \frac{\partial s}{\partial x} dy = \frac{2Wx_1 e^{-(\frac{x_1^2}{4\alpha t})} \sqrt{4\alpha t}}{16\alpha t^2 \sqrt{\pi}}$$

If an image is placed on the other side of the stream to satisfy the boundary condition that the drawdown at the stream is zero then the flow is doubled. Then if the stream maintains the water table level along its course the flow into the bank will be,

$$2 \int_{-\infty}^{+\infty} - KD \frac{\partial s}{\partial x} dy = \frac{4\alpha W x_1 e^{-\left(\frac{x_1^2}{4\alpha t}\right)}}{\sqrt{\pi} (4\alpha t)^{3/2}}.$$

Let ξ represent a time variable running between 0 and t . As before, ξ will represent the time at which an event occurs while t will represent the time at which the effect of the event is to be computed. The lifetime will be $(t - \xi)$. When the rate of drift is represented by f as before, the stream depletion at the time t due to removal of the volume of water $Qd\xi$ at the time ξ is:

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$$dq = \frac{4\alpha Q d\xi (x_1 - f(t - \xi)) e^{-\frac{(x_1 - f(t - \xi))^2}{4\alpha (t - \xi)}}}{\sqrt{\pi} (4\alpha (t - \xi))^{3/2}}$$

But since

$$\frac{(x_1^{-f(t-\xi)})^2}{4\alpha(t-\xi)} = \frac{x_1^2}{4\alpha(t-\xi)} - \frac{2x_1^f}{4\alpha} + \frac{f^2(t-\xi)}{4\alpha}$$

The total flow is:

$$q = \frac{4\alpha Q x_1 e^{(\frac{x_1 f}{2\alpha})}}{\sqrt{\pi} (4\alpha)^{3/2}} \int_0^t \frac{e^{-\frac{x_1^2}{4\alpha (t-\xi)} - \frac{f^2(t-\xi)}{4\alpha}}}{(t-\xi)^{3/2}}$$

$$-\frac{4\alpha Q f e^{(\frac{x_1 f}{2\alpha})}}{\sqrt{\pi} (4\alpha)^{3/2}} \int_{0}^{t} \frac{e^{-\frac{x_1^2}{4\alpha (t-\xi)} - \frac{f^2 (t-\xi)}{4\alpha}}}{(t-\xi)^{1/2}}$$

Let

$$u^2 = \frac{f^2(t-\xi)}{4\alpha} . \qquad d\xi = -\frac{4\alpha}{f^2} 2u \ du$$

$$\frac{x_1^2}{4\alpha(t-\xi)} = (\frac{x_1^f}{4\alpha})^2 \frac{1}{u^2} = \frac{1}{4}(\frac{x_1^f}{2\alpha})^2 \frac{1}{u^2}$$

Then, after substitution

$$q = \frac{Q e^{\frac{x_1 f}{2\alpha}}}{\sqrt{\pi}} (\frac{x_1 f}{2\alpha}) \int_0^{\frac{f^2 t}{4\alpha}} \frac{e^{-u^2 - \frac{1}{4}(\frac{x_1 f}{2\alpha})^2 \frac{1}{u^2}}}{u^2} du$$

$$- \frac{2Q e^{\frac{x_1 f}{2\alpha}}}{\sqrt{\pi}} \int_0^{\frac{f^2 t}{4\alpha}} e^{-u^2 - \frac{1}{4}(\frac{x_1 f}{2\alpha})^2 \frac{1}{u^2}} du . \qquad (11-4)$$

The integrals which appear here can be evaluated with the aid of Tables 15 and 13.

Comparison computation

A pump one mile from a river delivers 1.5 $\mathrm{ft}^3/\mathrm{sec}$. The aquifer properties are

$$K = 0.00384 \text{ ft/sec}$$
 D = 66.6 ft $V = 0.171$

 α = 1.50 ft²/sec KD = 0.256 ft²/sec. The water table gradient toward the river is ten feet per mile or γ = 0.001894. The stream depletion due to six months of pumping is to be evaluated.

Solution for no water table gradient

$$\frac{q_1}{Q} = 1 - \frac{2}{\sqrt{\pi}} \int_{0}^{\left(\frac{x_1}{\sqrt{4\alpha t}}\right)} e^{-u^2} du$$

Six months is 15,768,000 seconds or (1/2) year.

$$\sqrt{4\alpha t} = \sqrt{(4)(1.5)(15768000)} = \sqrt{9468000} = 9726.66 \text{ ft}$$

$$\left(\frac{x_1}{\sqrt{4\alpha t}}\right) = \frac{5280}{9726.66} = 0.54284$$
.

From Table 8:

$$\frac{2}{\sqrt{\pi}} \int_{0}^{0.54284} e^{-u^{2}} du = 0.55733$$

9.09316

Then

$$\frac{q_1}{Q} = 1 - 0.55733 = 0.44267 \quad q_1 = (1.5)(0.44267) = 0.664 \text{ ft}^3/\text{sec.}$$

With a water table gradient of $\gamma = 0.001894$

20.45226

$$f = \frac{K\gamma}{V} = \frac{(0.00384)(0.001894)}{0.171} = 0.0000425319$$

$$\sqrt{\frac{f^2t}{4\alpha}} = 0.0689490 \qquad (\frac{x_1f}{2\alpha}) = 0.0748561$$

$$m = \frac{1}{2}(\frac{x_1f}{2\alpha}) = 0.0374280 \quad e^{2m} = 1.07773$$
From Table 15
$$\int_{0}^{0.0689} \frac{e^{-u^2 - \frac{m^2}{u^2}}}{u^2} du}{u^2}$$

$$m \qquad 0 \text{ to } \infty \qquad .0689 \text{ to } \infty \qquad 0 \text{ to } .068$$

$$.030 \qquad 27.82507 \qquad 11.96689 \qquad 15.85818$$

$$.037428 \qquad 10.83312$$

11.35910

Then

.040

$$\frac{q_1}{Q} = (0.60804)(.0748561)(10.83188) - (0.60804)(2)(0.02060)$$

$$= 0.46797$$

$$q = (1.5)(0.46797) = 0.702 \text{ ft}^3/\text{sec}.$$

The stream depletion at this time is increased about 5.7 percent. Again, the effect is a minor one. In this example it was assumed that none of the flow of the well returned to the water table in the area. When well water is used for irrigation a considerable part of the pumped water can return to the water table and, in such cases, only the part consumed depletes the stream.

Chapter 11

Problems

The time spent in interpolation is often a large part of the work of making computations based upon tabular values. When the table is a double entry table the time which must be allotted to interpolation must be increased. It is sometimes possible to modify the problem slightly so that tabular values may be used directly and still obtain results which are suitable for the intended purposes. The following problems are selected to illustrate this point.

(11-1) Remake the computation illustrating the use of formula 11-3 by changing the time and the radius R slightly so that the values needed can be read directly from Table 14.

Answer:
$$m = 0.010$$
 $R = 1152$ feet $\sqrt{\frac{f^2t}{4\alpha}} = 0.44$ $t = 25.455$ years

For no water table gradient s = 4.637 feet.

With a water table gradient present

$$\beta$$
 s (feet) 0 4.604 $(\pi/2)$ 4.513 π 4.424

The conclusion that the drift will not significantly alter the observed drawdowns is confirmed.

(11-2) Make a recomputation of the example of the use of formula 11-4 by slightly changing the values of f and t to cause the values of m and $\sqrt{\frac{f^2t}{4\alpha}}$ to be such as can be read directly from the tables.

Answer:

m = 0.040 f = 45.455 x
$$10^{-6}$$

 $\frac{f^2t}{4\alpha}$ = 0.070 t = 14,229,000 seconds

For no gradient $q_1 = 0.629 \text{ ft}^3/\text{sec}$

With the gradient present $q_1 = 0.641 \text{ ft}^3/\text{sec}$

The difference is 1.9 percent. Either this value or the 5.7 percent obtained from the interpolated values indicate that the effect is a minor one.

Chapter 12

Analogs and Digital Computers

The analytical methods which have been described in the previous chapters are pretty well restricted to cases which can be idealized as having regular boundaries and uniform aquifer conditions. When attempts are made to extend these methods to treat cases having irregular boundaries and nonuniform aquifer conditions serious mathematical difficulties are encountered. To avoid these, analog or digital methods may be used.

The electric analog is based upon a similarity of the laws of flow of the electric current and the flow of ground water. A flow of electricity can, then, represent a flow of ground water, a voltage represent a water table elevation and, when transient changes are modeled, a capacitance can represent the ability of the aquifer to store water on a rise of the water table. In a common form of analog the electrical components are mounted on pegboard sheets. These sheets are commonly four feet wide and eight feet long. They are about 3/16 inch thick and have perforations at one-inch centers both ways. The holes scale about 9/32 inch in diameter. Nylon insulating bushings incorporating a "banana plug" type of connector are commercially available. These can be mounted in the holes in the pegboard and secured by means of a locknut. A metal prong is provided to which connections may be soldered. The bushings come in an assortment of colors so that important points on the network can be identified by a colored bushing. Stream courses can be marked by blue bushings and will show up clearly if white bushings are used for the other node points. This type of analog is fundamentally a direct current device.

Before the bushings are installed, a map to the proper scale should be mounted on the pegboard sheet. It helps to have a reversed map mounted on

the back of the pegboard. This map should show what would be seen if the map on the front of the panel were placed face down on a light table and viewed from the back. If these two maps are carefully coordinated the map on the back side of the pegboard will be found to be of the greatest help when the electrical components are being mounted. Having these elements on the back will free the front of the pegboard for reference and operation. The panels can be mounted on a frame to hold them at a convenient height. Casters on the frames will make it easy to move them around. When several panels are needed to cover the area of interest they may be hinged together so that the assembly can be folded up and rolled out of the way when not in use. A panel of this type with the maps and electrical components in place ready to go, will cost about a dollar per node point. When capacitors are provided transient ground water movements can be modeled. By making use of this analogy the field problem can be taken into the laboratory and tests can be run which would be prohibitively costly and time consuming if tried in the field. Such an analog is shown on figures 12-1 and 12-2. The network arrangement is shown in figure 12-3.

A method of selecting analog components to meet the needs of a specific problem will be described later but it may be stated here that the analog builder has two basic choices. He can choose to use small resistances and capacitances to create a very high speed analog or he can use large resistances and capacitances to create an analog which will be slowed sufficiently to permit the use of a direct writing oscillograph as a readout device. If the high speed choice is made an oscilloscope is the only device with a high enough writing speed to be useful for readout purposes. To get permanent records will require a camera to photograph the face of the oscilloscope tube. Even so, it will generally be necessary to use an electronic device to impose the

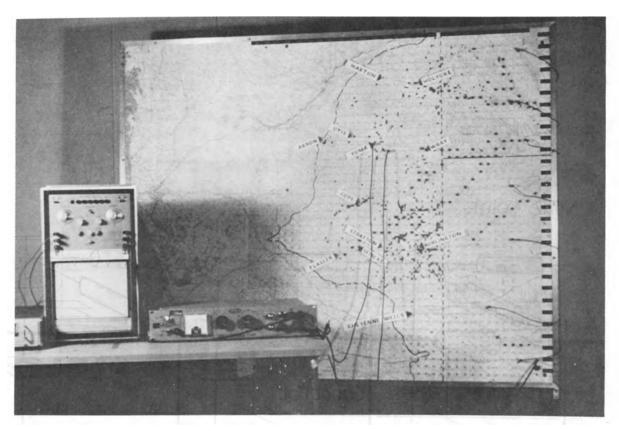


Fig. 12-1. Transient type analog.

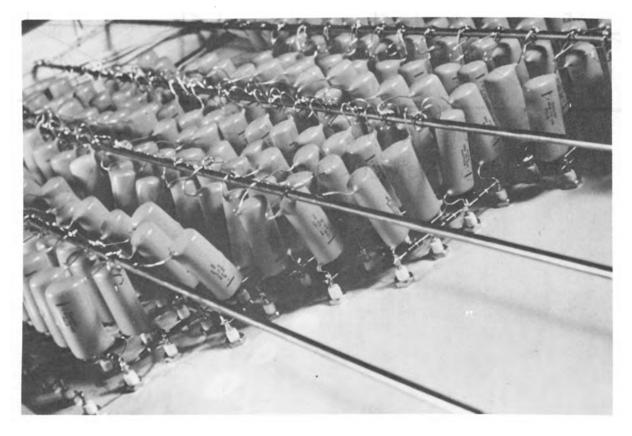


Fig. 12-2. View of back of panel showing capacitors and ground wires.

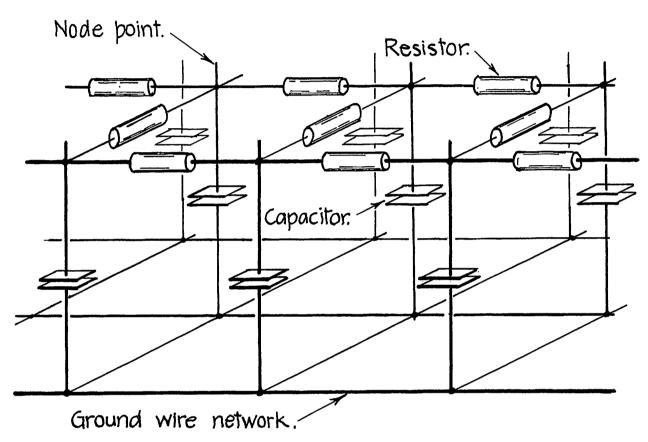


Fig 12-3 Network arrangement.

problem conditions on the analog network repeatedly. If the repetitions are made often enough persistence of vision will cause the successive traces on the oscilloscope tube to appear as a steady pattern. This may be read, or photographed when a permanent record is needed. Cameras of the Polaroid type which give a finished picture in a few seconds are very convenient for this use.

When the slower type of analog is used a finished record is obtained directly from the direct writing oscillograph. A complete record, including the making of notes and identifications can be made in about 20 seconds but this rate of operation with direct writing equipment is made at some cost. With resistances of the order of 1 million ohms and capacitances of the order of 1 microfarad the network resistance, from a selected node point to ground, may be of the order of 10 million ohms. A readout instrument with an input impedence of around 1 million ohms connected between a node point and ground would essentially short circuit the network and cause it to give spurious readings. It will be essential, then, to interpose an electronic servo device which will sense the network voltage and impose it on the oscillograph input without drawing current from the network. Such devices are commercially available or can be constructed. Such a device is generally unnecessary with the high speed type of analog. Inputs into the high speed analog can be controlled by use of resistances which are high compared to the network resistance but with the low speed analog the network resistance is already so high that some other method of control must be found. This takes the form of another electronic servo device which will feed a constant current into a network node point without being influenced by network voltage changes. With the low speed analog an excessive amount of time may be required for network potentials to drain to ground when a sequence of runs is to be made. To

overcome this difficulty a multi-contact switch can be installed and connected between selected node points and the ground. With 30 contacts 30 such connections can be made. This switch must be opened, of course, before the next run is made. A time controlled relay with a pilot light which comes on when the switch is activated and goes off when the relay opens the circuit is a useful arrangement. Tests will show how long these drainage contacts need to remain closed. Something like 20 seconds may be needed.

A simpler type of analog which is, however, restricted to steady state conditions can be made with the use of a conducting sheet. Such a sheet may be obtained by rubbing a soft pencil lead on paper but this is difficult to make uniform. A much better material is sold commercially under the name "Teledeltos" paper. It is made for use on recording instruments. The back of the paper has a conducting coating and if this is in contact with a metal surface and a metal stylus is in contact with the face side it will leave a mark on the paper if a suitable voltage is applied between the metal surface and the stylus. The conducting surface on the back side is of interest for our purposes. The resistance measured across a square inch is about 3000 ohms. A conducting ink is commercially available. It is used primarily for making complex electrical circuits. It flows well from a drawing pen. Being based on the use of silver it is more expensive than ordinary inks. Manufacturers of recording instruments and electrical shops are possible sources of these materials. An analog of this type representing the "Northern High Plains" area in Colorado is shown in figure 12-4. Even though this type of analog is appropriate for steady state conditions it can often be used effectively for transient conditions by combining its use with analytical methods. When so used it provides an effective means for establishing boundary conditions. An example of such a use will be given later under the heading: Combination methods.

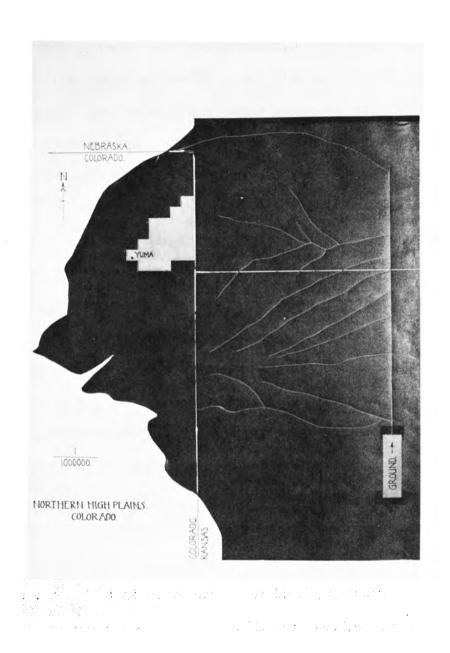


Fig. 12-4. Steady state analog.

When a digital computer is available an arithmetical approach can be used. The computation procedure is basically a simple one but the amount of computation required would be prohibitive if desk computers were to be used. The basic differential equation of ground water flow comprises two fundamental requirements. These are that Darcy's law is obeyed and that water volumes must be accounted for. The second of these is the so-called continuity requirement. One way to go about the computation would be as follows: Suppose the area is represented by node points, as in the first of the electrical analogs described above, and an initial water table configuration is given. Gradients can be computed from the differences of water table elevation between adjacent node points. If the grid is rectangular, there will be four of these for each interior node point. The algebraic sum of these multiplied by the product of the transmissibility, the node spacing and selected time interval will give the flow of water into the square area tributary to the node point. This product divided by the horizontal area represented by the node point and divided by the voids ratio V will yield the rise of the water table during the selected time interval. When the water table elevations have all been corrected the process can be repeated.

With the analytical procedures there is no approximation in either space or time. The node-wise electric analog is step-wise in space but continuous in time. The digital computer procedure is step-wise in both space and time. Both of these latter two methods are, therefore, to some extent, approximations. Experience with analogs indicates, however, that they do give a satisfactory representation of the behavior of the aquifer prototype. Their local lack of resolution must be kept in mind, however, as will be explained later. Design of analogs

For purposes of analog design, continuity of space and time will be assumed. The modifications needed to adapt the results to a node-wise analog

will be made as the last step in the design procedure. A basic fact relating to the conductance of a uniform conducting sheet may as well be described now. The resistance to the flow of current between opposite sides of a square area is proportional to the length and inversely proportional to the width. This means that the resistance measured between opposite sides of a square area cut from the material is independent of the size of the square. The resistivity of the material can, therefore, be specified in terms of the resistance between opposite sides of a square area cut from the sheet. The resistivity of the sheet can, then, be specified in ohms. The conductance is the reciprocal of the resistance and can be expressed, if needed, in mhos.

The design can proceed by the following steps:

- (1) Express the flow and continuity relationships for the aquifer.
- (2) Express the flow and continuity relationships for the electrical conducting sheet.
- (3) Write a series of correlation equations relating the coefficients of the aquifer and analog equations.
- (4) Seek, by trial, a set of correlation equation coefficients that will adapt the analog to represent the prototype in a way which will be compatible with commercially available electrical components and the capabilities of the recording equipment.

If we are to deal with the design of a node-wise transient type analog and wish to accommodate its characteristics to work with a direct writing oscillograph the largest capacitor size conveniently available should be chosen. Capacitors of the "Milar" type with 1 microfarad capacities can be obtained. Larger values could be obtained with electrolytic types but these are vulnerable to being punctured by accidental reversed voltage and their use for analog elements is considered unwise. The 1 microfarad size has served well.

The pertinent equations are the following:

Aquifer Analog $q = -KD \frac{\partial s}{\partial x} \qquad \qquad i = -\frac{1}{\rho} \frac{\partial E}{\partial \xi}$ $\frac{\partial q}{\partial x} = V \frac{\partial s}{\partial t} \qquad \qquad \frac{\partial i}{\partial \xi} = C \frac{\partial E}{\partial \eta}$ (12-1)

The correlation equations are:

$$n_1 = \frac{q}{i}$$
 $n_2 = \frac{s}{E}$ $n_3 = \frac{x}{\xi}$ $n_4 = \frac{t}{\eta}$ (12-2)

By substitution into the first of the aquifer equations

$$n_1 i = - KD \frac{n_2 \partial E}{n_3 \partial \xi}$$

and, by comparison with the first of the analog equations

$$\frac{1}{\rho} = \frac{\text{KDn}_2}{n_1 n_3}$$
 or $\rho = \frac{n_1 n_3}{\text{KDn}_2}$ (12-3)

A substitution into the second of the aquifer relations yields

$$\frac{n_1}{n_3} \frac{\partial i}{\partial \xi} = V \frac{n_2 \partial E}{n_4 \partial \eta}$$

A comparison with the corresponding analog relationship will yield

$$C = \frac{n_2 n_3 V}{n_1 n_4} \tag{12-4}$$

An important relationship which will be required for analog operation is the current, I, which represents a flow Q in the prototype. If, for example,

a well lifts water at the rate Q in the prototype, what current I will represent this flow in the analog? To develop this relationship suppose a closed loop to be drawn around the well in the prototype. Then if ℓ_1 represents a length measured along the loop and m_1 , a length measured normal to the loop

$$Q = \int_{0}^{\infty} q d\ell_{1} = KD \int_{0}^{\infty} \frac{\partial s}{\partial m_{1}} d\ell_{1}$$

where the lower limit of the integral signifies that the integration is to be carried around the loop. In the analog the total current is, similarly,

$$I = \frac{1}{\rho} \int_{0}^{\infty} \frac{\partial E}{\partial m_2} dk_2$$

By substitution from the correlation equations

$$Q = KD \int_{0}^{\infty} \frac{n_{2}^{\partial E} n_{3}^{\partial L}}{n_{3}^{\partial m_{2}}} = n_{2}^{0} KD\rho I$$
 (12-5)

But from formula 12-3 KD $\rho = \frac{n_1 n_3}{n_2}$ and, by substitution

$$Q = n_1 n_3 I \tag{12-6}$$

Where transient conditions are being studied the loop must be shrunk until releases of storage between the loop and the well become negligible.

An example will illustrate the use of these relationships. The construction of an analog to represent the "Northern High Plains" area in eastern Colorado will be our goal. The aquifer is in the Ogallala and similar formations. These are water laid deposits which increase in thickness toward the east. Infiltration from precipitation sustains saturated thicknesses of

up to 300 feet or more. The water table slopes toward the east and there is a ground water flow in this direction also. A part of this appears on the surface to sustain the base flow of streams but the bulk of the flow crosses the state line as underground flow. Pumping for irrigation and other uses has been developed. The flow pattern which is sustained by the precipitation will be considered as a basic steady state solution of the equations of ground water flow and the transient changes produced by pumping will be superimposed upon it. The configuration of the water table before much pumping was done is known from an early investigation (McGovern and Coffin, 1963). The analog will be used to evaluate the transient changes produced by pumping. The area to be represented is about 9000 square miles.

With a permeability of 0.001 (ft/sec) the transmissivities will be:

Saturated Thickness (feet)	Transmissivity (ft ² /sec)
0	0
50	0.050
100	0.100
200	0.200
300	0.300
400	0.400

A suitable available map is to the scale (1/250,000). This fixes the value of $n_3 = (x/\xi)$ making it $n_3 = 250,000$. The task of finding the remaining n values can now proceed. It is a trial process. The first set of trial values chosen will often be found to be hopeless but they will generally indicate what steps must be taken to make an improvement. After several trials the following values are taken:

$$n_1 = 4$$
 $n_3 = 250,000$ $n_2 = 5$ $n_4 = 315,360,000$

The n_4 value so chosen will mean that one second of analog time will represent 10 years of prototype time. Then the resistivities can be evaluated from the relation

$$\rho = \frac{n_1^{n_3}}{KDn_2} = \frac{(4)(250,000)}{KD(5)} = \frac{200,000}{KD}$$

KD	ρ	Some Values Commercially Available in 1/2 Watt Resistors
(ft²/sec)	(ohms)	(ohms)
0	∞	
0.050	4,000,000	3,900,000
0.100	2,000,000	2,200,000
0.200	1,000,000	1,000,000
0.300	667,000	680,000
0.400	500,000	470,000
	(ft ² /sec) 0 0.050 0.100 0.200 0.300	(ft ² /sec) (ohms) 0

On the basis that the effective porosity is V = 0.15 the value of distributed capacitance needed is

$$C = \frac{n_2 n_3 V}{n_1 n_4} = \frac{(5)(250,000)(0.15)}{(4)(315,360,000)} = 0.0001486 \left(\frac{\text{farads}}{\text{ft}^2}\right)$$

Up to this point the analog quantities have been treated as being distributed but we must now convert them to lumped quantities. The arrangement of the analog elements is shown in figures 12-2 and 12-3. We choose to install node points at one inch centers. Since a rectangular grid is being used each resistor must represent the resistance across a one-inch square. Then, by the rule previously given, the resistors needed are of the values shown in the computation for ρ . If the capacitance needed is 0.0001486 farads per square foot of the analog area then the capacitance needed at each node point, which represents one square inch of the analog area, is;

 $\frac{0.0001486}{144}$ = 0.00000103 farads or, closely enough, one microfarad.

This will be the capacitance needed at each node point. Capacitors and resistors with a 10 percent tolerance have been found to be satisfactory. It may be noted that the computed resistance often cannot be exactly matched with commercially available components anyway. Capacitors with a voltage rating of 200 volts have performed well.

Readout equipment

A direct writing oscillograph offers advantages for analog use because a permanent record is immediately available without the need for development, as would generally be the case with other types of oscillographs. The direct writing oscillograph used with the analog of figure 12-1 had a choice of paper speeds of 5, 10, 20, 50, 100 and 200 mm/sec. This would permit recording of a 100-year period on about eight inches of records at the 20 mm/sec speed. This proved suitable for the intended purposes. When many records are to be run it may be profitable to have rubber stamps made to show the horizontal and vertical scales. Other needed identification can be written directly on the record.

Personnel

A crew of two is generally needed to construct and operate an analog. These are a hydrographer and an electronics man. The electronics man should be able to construct the analog, to test the electrical equipment, to maintain the input and readout equipment and, when needed, to design and construct special instrumentation. If he is able to design and construct the servo devices mentioned previously, time and money can often be saved. The hydrographer will have the responsibility for setting up programs which will yield the information for which the analog was designed.

Test panel

Before the analog is constructed it is desirable to build and test a panel representing a circular area having elements appropriate for some selected part of the analog area. For the analog of the example a saturated depth of 200 feet could be chosen. A test panel with ten node points on a radius would serve very well. The ground wire is connected to all of the elements of the outer perimeter. There should be a node point at the center. Data from runs on this test panel should be compared with analytical solutions obtained by use of Table 6. Confirmation of the correctness of the design can be obtained from this test panel as well as some ideas about the resolution of the analog in the neighborhood of a node point. The center node point will represent a well of constant flow if the results are to conform to solutions obtained from Table 6. Near the center node point the analog and analytical solutions will not agree because the mesh of the analog is locally too coarse to have the needed resolution. The first node point out should conform fairly well and the second node point out from the center node point should give readings in good agreement with the analytical solution. If readings from the second node point out from the center are accepted and drawdowns close to the center node point are carried in from the second node point by use of the analytical development for steady state drawdowns around a well of constant flow, a good correlation with the solution obtained from Table 6 should be realized. If such a correlation is not realized an error in design of the analog is indicated.

Example of use of a steady-state analog

In the Northern High Plains area previously described it would be pertinent to inquire as to how much of the annual infiltration supply could be recovered. Under primitive conditions all of this supply moved eastward

under the action of a regional gradient to appear as the base flow of streams. To recover the water for irrigation or other uses pumps are used. These must modify the gradients sufficiently to divert the flow to the well. About 50 feet of saturated depth is needed to sustain the flow of a well yielding about one cubic foot per second (USGS WSP 1819-1) which is considered to be about the smallest yield which would make the well economically viable (Colo GW Circular No. 8). Then recovery means that the well must compete with the pre-existing gradients for the water it gets. It must be expected that only a part of the total supply can be recovered.

The analog, constructed of Teledeltos paper, is shown on figure 12-4. The scale is 1/1,000,000. Pumping was assumed to be concentrated within an area of 25 townships covering, roughly, the area in Colorado within the 200 foot saturated depth contour. This area is outlined in conducting ink. Stream and river systems east of the state line are traced in conducting ink and connected to a ground line which crosses them from north to south near the east boundary of the analog sheet. A measurement of resistance between the boundary of the pumped area and the ground line gives 600 ohms. Test measurements made on a one inch square and between concentric circles give $\rho = 3000$ ohms as the resistance of the coating.

It will be convenient to choose $n_2 = 100$ (ft/volt) and to use formula 12-5.

$$Q = n_2 K D \rho I$$

For this area with K = 0.00128 (ft/sec) and D = 200 (feet) KD = 0.256 (ft²/sec). If 50 feet of saturated depth must remain to supply the well then the permissible drawdown is 200 - 50 = 150 feet. With $n_2 = 100$ feet per volt the 150 foot drawdown would be represented by 1.5 volts applied between the pumped

area boundary and the ground line. From Ohm's law and the measured resistance

$$I = \frac{1.5}{600} = 0.0025$$

then

$$Q = (100) (0.256) (3000) (0.0025) = 192 (ft3/sec)$$

This would be equivalent to 139,000 acre feet per year. A similar analog constructed for a square area of 25 townships in the Burlington area south of the Arickaree River and handled in the same way yields 44,000 acre feet per year as the recoverable yield of this area. Then the total estimated yield is 139,000 + 44,000 = 183,000 acre feet per year. If this figure is compared with the estimated 430,000 acre feet per year of total infiltration (Colo GW Circular No. 8) then the recoverable yield is about 43 percent of the supply. Estimates based on other considerations indicate that something around one-half of the supply can be recovered with present methods on a permanent basis. The method of operating the analogs accounts for the effects of pumping on the Colorado side of the state line only.

Check of the analog

On a piece of Teledeltos paper two concentric circles are drawn with radii 5 mm and 49 mm respectively. The circles are drawn with conducting ink and the resistance between them is found to be 1100 ohms. Then with KD = 0.256 (ft²/sec) and one volt impressed between the circles the flow would be

$$Q = \frac{(100)(0.256)(3000)(1)}{1100} = 69.8 \text{ (ft}^3/\text{sec)}$$

For the prototype the radius of the outer boundary is

$$b_1 = \frac{(49)(1000000)}{304.8} = 160760 \text{ (feet)}$$

A similar computation yields $a_1 = 16404$ feet. The drawdown would be $s_a = n_2(1.0) = 100$ (feet). The flow would be, from the steady state flow formula,

$$Q = \frac{2\pi KDs_a}{\log_e(\frac{b_1}{a_1})} = \frac{(6.2832)(0.256)(100)}{2.28238} = 70.5 \text{ (ft}^3/\text{sec)}$$

This is close to the 69.8 (ft³/sec) value found from the test analog and it can be concluded that the steady state high plains analog is properly designed.

If it should be desired to make an approximate estimate of the time required for water table readjustments to take place the pumped area could be idealized as being circular with a sink at the center. The flow of the wells in the pumped area must be obtained by depleting the flow of the streams which run eastwardly over the Ogallala formation. A source placed somewhere to the east of the state line could be substituted for the stream depletion if we knew where to put it. A circular area 16.93 miles in radius would have the same area as the 25 townships. The center of gravity of the pumped area lies 14.28 miles west of the state line. Then the idealization would be shown below

Point at which drawdown

Boundary equivalent to the eastwardly running streams.

Sink

Source-Q

Fig 12-5 Source-sink combination.

The drawdown produced by a source sink combination, as shown, is

$$s = \frac{Q}{2\pi KD} \log_e(\frac{r_2}{r_1})$$
 r_1 , $r_2 < 0$ (12-7)

Along a line drawn midway between them, so that $r_1 = r_2$, the drawdown is zero. Suppose it is desired to estimate the drawdown at a point on the rim of the area north of the sink after pumping has continued for 100 years. It is first necessary to find the distance b. With Q = 192 (ft³/sec) KD = 0.256 (ft²/sec)

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$$\frac{Q}{2\pi KD} = \frac{192}{(6.2832)(0.256)} = 119.4 \text{ (feet)} \frac{s}{(\frac{Q}{2\pi KD})} = \frac{150}{119.4} = 1.257$$

From tables with, approximately

$$\log_e \left(\frac{2b}{a}\right) = 1.257$$
 $\left(\frac{2b}{a}\right) = 3.515$

$$b = \frac{(3.512)(16.93)}{2} = 29.73 \text{ miles}$$
 2b = 59.50 miles

The distance from the source to the point north of the sink is

$$r_2 = \sqrt{16.93^2 + 59.50^2} = 61.86 \text{ miles}$$

or 326,630. feet. 16.93 miles is 89,290. feet. (Note that here it is necessary to revert to consistent notation whereas before the ratio (r_2/r_1) would be the same whether the radii were expressed in miles or feet.) 100 years is 3,153,600,000. seconds. With

$$\sqrt{4\alpha t} = \sqrt{(4)(1.707)(3153.6)(10)^6} = 146700$$

$$\left(\frac{\mathbf{r}_1}{\sqrt{4\alpha t}}\right) = \frac{89390}{146700} = 0.609 \quad \left(\frac{\mathbf{r}_2}{\sqrt{4\alpha t}}\right) = \frac{326431}{146700} = 2.226$$

From Table 1 for

$$\left(\frac{\mathbf{r}_{1}}{\sqrt{4\alpha t}}\right) = 0.609 \qquad \int_{0.609}^{\infty} \frac{\bar{e}^{u} \frac{du}{du}}{u} = 0.37690$$

$$\left(\frac{\mathbf{r}_{2}}{\sqrt{4\alpha t}}\right) = 2.226 \qquad \int_{2.226}^{\infty} \frac{\bar{e}^{-u} \frac{du}{du}}{u} = 0.00061$$

$$\left(\frac{Q}{2\pi KD}\right) = \frac{192}{(6.2832)(0.256)} = 119.366$$

Then the drawdown at this time is

$$s = 119.366 (0.37690 - 0.00061) = 44.92 feet$$

Runs made on the transient state analog previously described indicated that something like 8000 years would be required to establish a new regimen in this area. Computations like those above will show that a drawdown of about 130 feet would be attained in 1000 years and that the full 150 feet would be substantially obtained in 8000 years. These results are only true if the extraction rate is one that can be permanently sustained. If the resource is over-encumbered, depletion can come quickly.

Combination methods

Each of the available methods for quantitatively evaluating the flow of ground water has its advantages and its limitations. These methods should be thought of as tools and when some specific task has to be done the best tool for the job should be selected to do it. Sometimes a combination of methods can be used which utilizes their possibilities to the best advantage and

permits doing a specific task in a more effective way than would be possible if either were used alone. With a transient type analog, responses to unit inputs can be quickly obtained and if some sort of an operations study is to be made, these data can be fed into a digital computer whose prodigious ability to handle arithmetic can make short work of a job which, without it, would be laborious and time consuming. If a programmable input device were available the operations study could be done on the analog but so far as this writer is aware, no such device is now in existence. With digital computers at hand there seems to be no very good reason to try to develop one. A combination of analytical and digital methods is often very effective. A case has already been described where a simple steady state analog was used to give a boundary location which would permit a transient type analytical computation to be made.

Chapter 12

Problems

(12-1) A paper having a conducting coating has concentric circles of 100 mm and 10 mm diameter drawn on it with conducting ink. A measurement of the electrical resistance between the two circles yields 1040 ohms. Compute the resistivity ρ of the conducting coating.

Answer: 2838 ohms.

(12-2) A transient type analog is to be constructed to represent a 40 mile reach of river valley. The resistors are to be of around 100,000 ohms rating and the capacitors are to be of 1 microfarad size. The analog network is to be mounted on a 4 x 8 foot pegboard panel having holes at one inch centers. There is to be a node point at each half mile spacing, to scale. There are to be 1200 node points. The pegboard is to be mounted on a frame to support it at a convenient height. Estimate the cost of the completed network panel.

Answer: The cost should be in the neighborhood of one dollar per node point (at 1972 prices).

- (12-3) What should be the scale of the maps to be mounted on the above panel? Answer: 1/31680.
- (12-4) Suppose that maps were available to a scale of 1/24000. What length of river valley could be accommodated on an 8-foot long pegboard sheet?

 Answer: 36 miles.
- (12-5) If available maps are reduced to the 1/31680 scale by photographic processes will it then be possible to locate node points at section corners and quarter corners?

Answer: It is not to be expected. The difficulties of mapping a spherical earth onto flat sheets does not permit perfect regularity of section lines representing mile squares.

Notation

The notation used herein has the following significance. Physical dimensions are specified in foot-second units but since consistent units are used everywhere in this text the formulas can be used without change in any consistent unit system. Basic quantities must, of course, be expressed in the chosen units.

```
h
                a drainable depth
                                                                     (ft)
     h_1
                a pressure departure from a hydrostatic state,
                as used in Chapter 2
                                                                     (ft)
                a first approximation drainable depth as used in
     h<sub>1</sub>
                Chapter 6
                                                                     (ft)
     h<sub>2</sub>
                a saturated depth, as used in Chapter 2
                                                                     (ft)
     h<sub>2</sub>
                a drainable depth as used with Werner's treatment
                a drainable depth as used by Hauchild and Kruse
     hz
     ^{\rm h}{_{\rm c}}
                a drainable depth midway between drains
                                                                     (ft)
     ^{\rm h}{}_{\rm o}
                a depth over a tile drain acting to cause flow
                to the drain
                                                                     (ft)
     H
                an initial saturated depth
     i
                a gradient
                                                                     (dimensionless)
     i
                an infiltration rate, reckoned as entire water
                                                                     (ft/sec)
     i
                current in an analog
                                                                     (amperes/ft)
     I
                a total current representing an input to, or output
                from, an analog
                                                                     (amperes)
J_0 and J_1
                Bessel's functions of the zeroth and first orders
     k
                (a/b)
K and K
                modified Bessel's functions of the zeroth and first orders
     K
                the permeability of an aquifer
                                                                     (ft/sec)
     1
                an equivalent length
                                                                     (ft)
     L
                a distance between drains
                                                                     (ft)
     L
                a valley width
                                                                     (ft)
     m
                a parameter which appears in Tables 13,14 and 15
                (for definitions see Chapters 5 and 11)
m = 0.74930
                a number which appears in the development of formula (4-13)
                a term number in a Fourier series
     n
n_1, n_2, n_3 and n_4 numbers which appear in the correlation equations (12-2)
```

```
the part of an original drainable volume which remains at
        p
                   the time t (see Table 11)
                                                                          (dimensionless)
                   a permeability for vertical flow through a slowly permeable
        p
                   bed (see Figure 5-1)
                                                                          (ft/sec)
                   a pressure measured in feet of water representing a
        p
                   departure from a hydrostatic state (refer to developments
                   based on formula 8-14)
                   a first derivative. Defined where used
        р
        P
                                                                         (ft^3)
                   a total production volume
                                                                         (ft<sup>3</sup>/sec)
                   a stream depletion caused by a well
        q<sub>1</sub>
                                                                         (ft<sup>2</sup>/sec)
                   a canal seepage rate as used in Chapter 7
        q
                   a return flow rate used in the development of
        q_2
                                                                         (ft<sup>2</sup>/sec)
                   formula 10-2
                                                                         (ft<sup>3</sup>/sec)
                   a rate of flow. A well flow
        Q
                   a radius
                                                                         (ft)
        r
R(\frac{\alpha t}{L^2})
                   a factor used to estimate a volume of return flow.
                   (see Table 12)
                                                                         (dimensionless)
        R
                   a radius drawn from a well to a point where the drawdown
                   is to be computed. It is used where a groundwater drift
                   is present
                                                                         (ft)
                   a drawdown
                                                                         (ft)
        S
        t.
                   time
                                                                         (sec)
        T
                   a time required to complete a cycle
                                                                         (sec)
                   a generalized variable
                                                                         (dimensionless)
U_{0}(\beta_{n}r)
                   J_{O}(\beta_{n}a) \quad Y_{O}(\beta_{n}r) - Y_{O}(\beta_{n}a) \quad J_{O}(\beta_{n}r)
                   a generalized variable
                                                                         (dimensionless)
        ν
        ٧
                   the ratio of the drainable or fillable voids to the gross
                   volume. It is used where a free water table is present
                                                                         (dimensionless)
        ٧,
                   the volume yield of an artesian aquifer per unit of
                   horizontal area per unit of pressure reduction (dimensionless)
                   a variable of integration
                                                                         (dimensionless)
        W
```

		W		a quantity of water assumed to be instantaneous removed	ly (ft ³)
		x		a coordinate distance	(ft)
		x ₁		the distance of a well from a river	(ft)
		у		a coordinate distance, defined where used	(ft)
		у		a pressure reduction near an artesian well	(ft)
		z		a coordinate distance	(ft)
		z		a parameter, defined where used	
α =	= <u>I</u>	<u>V</u>		a constant for a given aquifer which defines th with which a transient change will take place	e rapidity (ft ² /sec)
		β_n		a root of a Bessel's function	(dimensionless)
		Υ		a naturally occurring water table gradient	(dimensionless)
		η		time in an analog	(seconds)
	λ thickness of a semi-permeable confining bed		(ft)		
		μ		a viscosity	
		ξ		length in an analog	(ft)
		π	=	3.1415926535+	(dimensionless)
		ρ		resistance in an analog	(ohms)
		σ	=	$\frac{Q}{2\pi KD^2}$ (see formulas 3-2 and 3-3)	(dimensionless)

Notation used in the design of analogs

$C = \frac{n_2 n_3 V}{n_1 n_4}$	Capacitance per unit of area	$(\frac{\text{farads}}{\text{ft}^2})$ or $(\frac{\text{amp sec}}{\text{ft}^2\text{volt}})$
D	Saturated thickness in the prototype	(ft)
E	Electrical pressure in the analog	(volts)
i:	Current flow in the analog	(amperes/ft)
K	Permeability in the prototype	(ft/sec)
KD	Transmissivity in the prototype	(ft ² /sec)
$n_1 = \frac{q}{i}$		(ft ³ /sec amp)
$n_2 = \frac{s}{E}$		(ft/volt)
$n_3 = \frac{x}{\xi}$		(dimensionless)
$n_4 = \frac{t}{\eta}$		(dimensionless)
q	Flow in the prototype	(ft ² /sec)
s	Drawdown in the prototype	(ft)
t	Time in the prototype	(seconds)
V	Voids ratio in the prototype	(dimensionless)
x	Distance in the prototype	(ft)
η	Time in the analog	(seconds)
ξ	Length in the analog	(feet)
$\rho = \frac{n_1 n_3}{KDn_2}$	Resistance measured across a square are	a (ohms) or (volts/amp)

Definition of terms

Aquifer

A water bearing formation.

Aquifer Constant

A number characteristic of an aquifer which denotes the speed with which transient changes will take place in it. In this text it is represented by the symbol $\,\alpha$.

Artesian well

A well which taps a confined aquifer and which has a pressure sufficient to support a flowing well.

Bank storage

The water contained in an aquifer hydraulically connected with a stream or lake and capable of supplying water to the stream or lake following a lowering of the free water surface or of storing water flowing from the stream or lake on a rise of the free water surface.

Barrier

An impermeable formation in contact with an aquifer which confines the flow of groundwater to the aquifer.

Boundary conditions

Conditions imposed by boundaries.

Condition of continuity

The requirement that water volumes must be strictly accounted for. Confined aquifer

An aquifer sandwiched between impermeable formations.

Consistent units

A consistent unit system is defined as being one that permits only one unit of a kind.

When data come to the computer expressed in units other than those of a chosen system the first task is to convert them to the chosen system. Factors for making this conversion for some commonly met units are given under the heading: Conversion factors, equivalents and useful values.

Conversion factors, equivalents and useful values

To convert	to	multiply by
Gallons per minute	cubic feet per second	0.002228
Meinzers unit (permeability)	feet per second	1.5472(10) ⁻⁶
Meinzers unit (transmissivity)	feet squared per second	1.5472(10) ⁻⁶
Acre feet	cubic feet	43560
Cubic feet per second	gallons per minute	448.8
One year (365 days)	seconds	31,536,000
One month (1/12 year)	seconds	2,628,000
One day	seconds	86,400

A township has an area of 23,040 acres or 1003.62(10)⁶ square feet.

A section has an area of 640 acres or 27.8784(10)⁶ square feet.

One cubic foot per second running for one day will deliver

1.983471 acre feet. One cubic foot per second running for 365 days will deliver 723.9669 acre feet.

Note that while a year of 365 days is assumed for computation purposes herein a year is 365.2422 days (Smithsonian Physical Tables). This is 31,556,930 seconds. A cubic foot per second running for one year will deliver 724.447 acre feet.

Darcy's law

A law discovered by Henry Philibert Gaspard Darcy (1803-1858). His experiments showed that the velocity of flow through porous media is proportional to the first power of the gradient.

Drainable depth

A depth of groundwater above the level of a system of drains or above the level of a river, in an aquifer hydraulically connected thereto.

Drawdown

The amount a water table has sunk from an initial stable configuration.

A term used to describe a flow of groundwater under the action of a naturally existing regional gradient.

Dupuit-Forchheimer idealization

An idealization whose use was pioneered by Arsene Jules Emile Juvenal Dupuit (1804-1866) and Philipp Forchheimer (1852-1933). Under this idealization the gradient of the water table is assumed to be effective throughout the saturated thickness of the aquifer. When the water table gradient is small compared to unity the postulated conditions are substantially realized.

Entire water

A term used to describe water which occupies volume to the exclusion of everything else. Water flowing in a canal or a river is Entire water. Interstitial water occupies only the interstices between grains in an aquifer.

Exponential Integral

A tabulated function of the variable x of the form

$$\int_{x}^{\infty} \frac{e^{u} du}{u} \qquad \text{or} \qquad \int_{x}^{\infty} \frac{e^{-u} du}{u}$$

Gradient

A slope of the water table tending to cause the flow of groundwater.

Image

A hypothetical well, source or sink used as a mathematical device to satisfy a boundary condition.

Impermeable

Not permeable.

Infiltration

Water moving into the ground from a surface supply such as precipitation or irrigation. Infiltration rates are reckoned on the basis that the water is entire water.

Initial conditions

The conditions that prevail at the time of initiation of a transient.

Interstitial water

Water which occu ies the interstices between grains in a permeable bed. (see Entire water)

Linearization

Many of the differential equations representing physical relationships are inherently nonlinear in form. Such relationships are generally difficult to handle and it is often desirable to replace them with approximations which have a more tractable linear form. This may be done in specific cases, by neglect of small quantities, by replacing a curve with its tangent in a range of interest or by other means. The process is called linearization.

Line source

A source uniformly distributed along a line.

Local convergence

A convergence of flow of groundwater as to a drain tile.

Meinzer unit

A unit of permeability used in the older publications of the U.S. Geological Survey. It is defined as the flow of water in gallons per day through a cross sectional area of one square foot under a gradient of one foot head of water per foot of length, measured in the direction of flow, at a temperature of 60°F.

Parallel drains

Drains of the type installed for drainage of agricultural land. They can be of the form of open ditches or buried tile lines.

Permeability

A term used to describe the ability of water or oil to move through a porous formation under the action of a gradient. The facility with which a fluid will move through a formation is greater for some than for others. For a given bed the permeability is expressed by a constant K representing the flow through unit area in unit time under the influence of a unit gradient. The flow is expressed in terms of Entire water.

Probability integral

A tabulated function of the variable x of the form

$$\frac{2}{\sqrt{\pi}}\int_{0}^{x}e^{-u^{2}}du$$

Production

The total volume of well flow counted from the time of initiation of flow.

Radial flow

Flow converging toward a center.

Root

A value of an argument which will cause some given function of the argument to pass through zero.

Steady state cases

These are groundwater conditions which do not change with time.

Stream depletion

A depletion of stream flow caused by the operation of wells installed in an aquifer hydraulically connected to the stream.

Transient cases

These are groundwater conditions which are changing with time.

Voids ratio

The ratio of the volume of drainable or fillable voids to the gross volume.

Water table

The upper limit of the completely saturated material in an aquifer.

Linear interpolation

The tables presented herein are not extensive enough to insure that linear interpolation will give intermediate values with an accuracy comparable to the tabular values. Take, for example, these figures from Table 14 for m = 0.005.

Х	Tabular value
0.40	0.70454
0.41	0.68358
0.42	0.66330

Interpolation between the extreme values yields 0.68392 for the intermediate value. This differs from the tabular value by 0.68392 - 0.68358 = 0.0034. If the true curve is approximated, in the interval of interest, with a parabola, then it can be inferred that the error in the middle of a tabular increment would be one-fourth of the error at the middle of two tabular increments, as above. On this basis there could be an error of eight units in the last place, due to linear interpolation, in certain parts of this table. Most of the uses to which these tables may be put will not require precisions which will exhaust the capabilities of the tables. However, if some usage would require such precision then the reader should be aware that linear interpolation may be inadequate.

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Table 1

Values of:

$$\int_{x}^{\infty} \frac{e^{-u^2} du}{u}$$

This table was computed by Simpson's rule integration. This integral can be converted to the exponential integral by the substitution $u^2=v$. The relation is:

$$\int_{x}^{\infty} \frac{e^{-u^{2}} du}{u} = \frac{1}{2} \int_{x^{2}}^{\infty} \frac{e^{-v} dv}{v} = \frac{1}{2} E_{1}(x^{2}).$$

The following comparison can be made:

x	$\frac{1}{2} \int_{x^2}^{\infty} \frac{e^{-v}dv}{v}$	Tabular Value
0.00001	11.224317	11.22432
0.0001	8.921732	8.92173
0.001	6.619147	6.61915
0.01	4.316612	4.31661
0.1	2.018964	2.01896
1.0	0.109691	0.10969
3.0	0.00006	0.00001

																								٠	'nχ		·																								
6	.0270	.2798	.857	-560	.3325	.146	- 990	6.85487	.735	.629	532	.445	.364	.289	.220	.155	•094	.036	.982	.931	.881	.835	.790	.74785	70686	.66749						5,45913										7	7	~	∹	9	9	0	9	5.01172	σ.
00	.14	.33	7.89211	• 58	5	• 16	ĕ	8676	• 1469	•6393	.5421	.4536	.372	.2970	2271	.1617	.1003	.0425	•		~	٣,				۳	٦	-;	-;	۳,	٧.		4	٧.	7	"	.316	-289	.263	.237	.213	188	.165	.142	119	.097	075	•054	034	5.01373	.993
~	.2784	.3911	σ	•6134	.3741	1815	•0196	.8805	.7584	9649	.5514	.4621	.3801	.3043	2338	.1680	.1063	.0481	.9932	.9411	.8916	-8444	.7993	.7562	.7149	.6752	.6370	.6003	.5648	.5305																		5.05681		5.01574	.9958
•	.432	.451	96	- 640	395	. 198	034	893	• 769	•659	.560	.470	(1)	177	1,4	7	٦.	٩,	v.	٠.	۳.	۳	w,	7	٦.	*	٧.	¥	4	e,	٣,		4	4	m.	L.	m	7	~	7	.217	.193	.169	.146	.124	101	.080	.058	•038		.997
ĸ			8.00544	.6899	4176	-2169	0499	8	.7816	.6704	.5703	479	.396	.319	.247	.180	.118	059	.003	.951	.901	.853	808		7	.68	ø	.607	.571	.537	.5040	4	.4405	4	e.	e.	m,	~	.2710	•24	.2204	.196	.1722	.1489	.126	.1040	.082	٩.	040	0	66.
4	.838	.585	0462	•6979	440	.2353	.0654	.9202	• 793	•68	.579	.488	404	.3264	2	.1873	.1244	90.	•000	• 956	906	858	-812		.727	.686	.6483	.611	.5753	.5407	. 507	4	.443	.413	.383	.355	.327	3000	.273	.2419	5.22291	.1984	.1745	.1512	.128	.1062	.0844	•063	.0422	0	.0017
en	1257	.6593	9	. 7278	4631	.2540	.0811	.9338	.8054	.6917	.5895	6965	12	.3339	•26	.1938	.1305	.07	.0148	.9616	.9111	.8630	.8171	5.17328	.7312	69.	.6521	.6148	.5788	•	.51	.4781	•	.4161	.3865		.3299		.2762	.2505	.22	.2008	.1769	.15	.1307	91.	•0866	.0652	.0443	0238	• 003
2	.5311	94	.13	.7585	4866	•2730	.0971	.9476	.8176	. 7025	.5993	.5058	203	.3415	684	.2004	.1367	0768	.0203	.9668	.9160	.8677	.8216	.7775	.7353	.6948	.6559	.6185	.5824	.5475	.5138	.4813	.4497	.4191	.3895	•3606	•3326	.3054	.2789	-2530	.2278	.2033	.1793	.1559	.1330	.1106	.0887	613	.0463	5.02585	.0057
-	.2243	8264	•	.7903	5107	• 2824	.1134	• 961	829	2	• 609	.514	.428	.349	-275	.207	.142	082	.025	.972	.921	.872	.826	.781	.739	698	.659	.622	.585	.550	.517	-484	.452	.422	.392	.363	•335	•308	.281	.255	.230	.205	.181	.158	.135	.112	• 090	69	.048	0	• 007
0	,	.9217	8.22859	.8231	.5354	.3122	.1299	.9758	.8422	.7245	.6191	.5238	368	.3567	.2826	.2136	.1491	.0885	.0313	.9772	.9260	.8772	.8306	5.78624	.7436	.7028	.6636	.6259	.5895	.5544	.5205	.487	.4560		.3953	.3663	.3382	.3108	.2841	.2581	.2328		.1840	.1605	.13	.1150	.0931	91	.0505	029	1600
×	0	8	-0002	•0003	•0004	• 0002	• 0000	-000	*000	6000•	0	•0011	O	•0013	.0014	.0015	•0016	8	0	O	•0020	0	•0022	8	8	90	•0026	8	•0028	0	•0030	0		•0033	0	0	0	0	9	03	.0040	.0041	9	9	9	90	9	9	9	0049	05

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6	.6748	.2522	956	C071•	.3874	.2528	.1344	.0289	.9337	8	•7676	•6945	•6262	.5628	.5034	1.44/65		1000	7537	2113	1,17084	.1322	.0953	.0599	0261	m	624	324	035	757	6	6	1982	275093	7284	.70673	7	•66535	456	265	080	901	27	S	395	237	98
&	.7289	.2873	982	1641.	401	.2654	.145	•0389	.9428	8554	.7752	013	.6328	5689	• 509	ລີ	•		25.0	215	-174	136	960	0634	0294	.99681	654	23	.90641	82	16	762	7008	.75322	7306	.70887	17	•66736	75	84	960	19	.57447	75	411	2	960
7	.7860	3236	3.00891	5777	2.41670				•	1.86379	•	1.70851	•	1.57515		* * U	1.40537		26.2	219	_	7	1.10256	1.06692	0	1.00002	.96857	.93834	092	.88126	.85428	200	8031	ם נס	7328	\sim	868	.66937	•64954	•63033	.61172	936	761	S	27	268	11
9	.8466	٠	0362 7915	5052	14	.2913	-1684	.0594	.9613	-87	1907	157	• 646	.5813	.5208	1000	1.41055	2122	25669	2238	•	•	•	•	1.03612	•	.97165	.94131	.91211	.88401	.85693	78068*	. 80563	.75782	.73512	.71317	•69164	713	514	•63222	.61355	54	4	609	444	284	128
ĸ	.9112	4005	3.06441	61133	446	.3044	.1801	.0697	•9706	1.88081	798	22	-652	587	n,	\$.	417	1 7	27.2	228	_	1474	1098	0739	0394	94	.97476	.94429	•91498	.88677	. 85959	8353	.80811	.76014	7373	.71533		734	•65345	341	153	972	•57965	625	•	299	.51441
4	980	.441	90		.462	.317	.191	080	980	883	-806	1.73025	•659	.593	1.53264	•	1.442100	•	•	1.23228	•	1.15131	•	1.07745	1.04286	1.00970	.97787	.94728	.91786	m (.86227	ດເ	09018.	.76246	396	.71750	.69613	•67544	•65542	•63603	.61724	.59903	.58138	.56427	.54767	.53157	•51595
e	.0542	•4839	7 8	4500	.4780	.3313	.2037	.0908	.9897	.89	.8145	376	999•	6003	5385	n a	4407	270	, 0	10	_	_		0	O	129	.98100	502	207	923	49	267	01618.	47	7418	~	.69823	74	•65739	•63794	• 61909	800	.58312	629	493	Ē	-
~	.134	.528	3.15392	400	493	.345	•215	.101	• 999	906	.822	•745	673	909	544	200	104.		786	240	198	.159	1.12097	.084	•049	•016	.98413	953	.92366	895	.86764	684115	817	.76712	744	.72186	.70035	σ	6	39	20	20	.58487	29	20	34	19
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0	.3166	.6236	218	7083	5266	.3731	.2403	.1233	.0189	.9247	.8388	.7600	.6872	.6197	ů.		7744.	7076	2938	2494	2071	.1669	.1284	.0917	0565	.0228	904	593	294	200	73	400	9078	.77182	7486		045	836	633	437	246	062	• 58838	710	545	379	221
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4	~	86	717	4 <	4212	-41862	.40627	.39427	.38261	.37127	*36026	.34956	2	.31921	*30966	.30037	.29134	-28256	.27403	25767	26026	.24222	.23481	.22761	-22062	.21381	20120	19454	18	œ	.17684	•	16062	מונ	15	14	•14106	n o	.13211	~	.12367	961	157	.10823	
т	.50228	-48752	.47319	12664.	01644	.41988	.40749	.39545	•38376	.37239	436135	.35061	34018	32018	.31060	.30129	.29223	• 28343	27,487	0.0000	25041	.24297	.23554	.22832	.22131	-21449	20786	.19515	18907	.18316	_	17	16114	1560	1510	1462	15	13697	325	1282	1240	200	1161	.10859)
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7	760	1001	7 6	9	935	9	873	4	15	.07878	9	46	6	.06849	.06611	.06381	.06158	.05942	.05733	.05530	.05334	.05144	.04960	.04782	.04610	.04443	.04282	•04126	•03976	•03830	•03689	.03552	.03421	.03293	.03170	.03051	95650	07070	02416	21020	02618	02228	67670	02770	02045	20010	60100	*010°	75810.	66710*	06010•
9	10205	07001	.1004	47/60°	.09389	6	• 08768	w	.08184	•07905	.07635	.07373	•07119	.06873	v	•06404	.06180	.05963	•05753	.05550	.05353	.05163	.04978	.04800	.04627	.04460	.04298	.04142	.03990	.03844	•03703	•03266	.03434	•03306	.03182	• 03063	04670	02220	02625	02526	02428	02226	77220	02157	02073	01000	26100	*1610*	60100	99/10	9
Ð	10420	10083		14160	12460	50160*	_	.08501	.08212	.07933	• 07662	•07399	_	0	•	.06426	•06202	.05984	•05774	.05570	LO.	.05181	• 04996	.04817	• 04644	• 04476	.04314	•04157	.04005	• 03858	-03717	• 03579	.03447	.03318	.03195	.03075	66670.	02720	02635	02625	02438		4020 ·		02082) C) C	77610	יו ס	*0110*	_
4	_		• (007.00		5	∞ .	ഹ	.08241	Dr.	.07688	4	.07169	Or.	vo	•	•06224	90090*	•05795	.05590	•05392	•05200	.05015	.04835	.04661	.04493	•04330	.04173	.04020	.03873	•03730	•03293	.03460	.03331	.03207	.03087	11670	02220	02.50	200	.02447	. "	ינ	02175	05020	0000	38	חנ	9 6	•01710	7 7
8	1050		3 6	106	98460	19160	• 08829	.08559	• 08269	.07988	•07715	.07451	.07194	4	2	47	o	.06027	.05815	561	541	521	.05033	.04853	.04678	•04509	•04346	.04188	9	.03887	•03745	•03606	.03473	.03344	•03219	86050	70770	02761	02656	02554	02457	02362	02271	02183	86020	91020	Э О	10100	01	-01/88	-
7	5	5	90	o i	260	576	9880	822	823		774	0747	722	69	672	649	626	709	583	563	54	523	505	0487	469	452	436	450	100	390	37	362	0348	333	323	1200	,,,,,	7770	266	0256	0246	720	,,,	0210	210		֓֞֜֝֞֜֜֞֜֜֞֜֜֓֓֓֓֓֜֜֜֜֓֓֓֓֓֓֡֓֜֜֡֓֓֓֓֡֓֜֡֓֡֓֡֡֡֡֓֜֡֓֡֡֡֡֡֡	701		.01795	7
_	687	10001	7700	000	ט ניני	223	892	861	832	9	776	50	•07245	96	15	21	.06291	.06071	.05857	.05651	.05451	.05257	•05070	•04888	.04713	.04543	•04378	•04219	-04065	*03917	•03773	•03634	•03499	•03369	•03244	27150	00000	02782	77970		02476	5	a c	50		16	3 6	-		20100	-
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80	.01616	.01551	.01489	•	.01371	01316	N	N	11	_	0	.01023	.00980	• 00940	00600*	•00862	• 00826	.00791	.00757	• 00725	•00694	• 00664	• 00636	*0000	.00582	.00556	•00532	• 00509	• 00486	00400	44400	2000	00387	.00370	• 00353	.00337	•00322	.00307	•00293	.00280	.00267	.00254	.00243	~	.00220	.00210	2	6	•00182
7	.01622	.01558	.01495	æ	.01377	ന	\sim	N	.01166	•01118	-01072	.01027	.00985	• 00944	060	• 00866	8	•00194	.00761	•00728	. 00697	.00667	• 00638	.00611	.00584	• 00559	•00534	.00511	• 00488	19400	00440	2000	00389	.00371	• 00355	•00339	.00323	• 00309	•00294	9	02	02	.00244	0	02	02	9	-00192	.00183
૭	.01629	0156	.01501	_	.01383	_	5	~	_	.01123	_	~	0	• 00948	80600	.00870	•00833	0	0	.00731	00200	.00670	.00641	Ð	LD.	SO.	n	ຄຸ	•	69400			.00391	ິຕ	35	34	32	3	53	28	56	25	24	23	22	21	႙	6	.00184
ы	.01636	.01570	-	.01447	.01388	.01332	.01278	.01226	.01176	.01127	.01081	•01036	• 00993	.00952	.00912	•00874	.00837	.00801	.00767	.00735	.00703	• 00673	*00644	• 00616	• 00589	6 0 1	• 00239	.00516	.00493	1/400		11400	00393	.00375	.00358	.00342	• 00326	.00311	• 00297	028	027	025	05	023	022	05	050	5	.00184
4	.01642	015	151	145	33	13	12	~	~	.01132	.01085	.01040	16600	• 00956	• 00916	.00877	84	•00805	.00771	•00738	•00700	• 00676	.00647	•00619	•00592	• 00566	• 00542	.00518	.00495	.00473	-00432	00432	46600	.00377	ന	.00343	.00328	8	02	0	•00272	02	02	023	022	021	050	.00195	8
٤)	.01649	15	25	45	6	.01343	28	.01236	.01186	.01137	06010*	.01045	.01002	09600*	.00920	.00881		• 00808	7	0	0	• 00679	10	• 00622	• 00595	.00569	.00544	-00520	.00497	.004/2	200	1400	96600	0037	.00361	.00345	032	.00314	030	m	.00273	026	02	.00237	• 00226	• 00215	.00205	0	.00186
7	LC1	•01590	N	சு	6 3	•	┏•	•	~	•	•		\mathbf{c}	S	•00924	മ	.00848	_	~	•00744	_	ന	.00653	N	₽	~		~ ,	_	. 004.7	200	600400	4 ^	•00380	•		~	•00316	\sim	~	~	•	•00250	~				•00196	
_	166	5	153	147	141	35	129	124	119	114	109	105	101	960	6	088	085	081	078	074	Ξ	8	5	22	8	57	4	25	2 3	00480	֓֞֞֜֝֓֓֓֓֓֓֓֓֓֟֓֓֓֓֓֓֓֓֓֓֓֡֓֓֡֓֡֓֓֡֓֓֡֓֡֓֡֓֡] [38	36	34	33	031	30	028	027	026	025	023	022	21	050	61	018
0	69910.	160	153	141	141	36	130	125	120	'n	110	105	101	160	660	680	085	081	078	075	07.1	068	065	063	090	057	055	052	020	28400	9 4 9	7 7 0	040	038	036	035	033	031	30	029	027	026	25	024	022	21	020	<u>, , , , , , , , , , , , , , , , , , , </u>	018
×	1.51	S	ŝ	S	ŝ	Š	ů.	ŝ	S	•	9	Φ	9	•	9	9	1.67	1.68	1.69	1.70	1.71	1.72	1.73	1.74	~	1.76	-	- 1	~ (08.7	o a	1 0 0 1	σ	8	œ	æ	1.88	æ	1.90	ç.	1.92	٠,	1.94	٥.	ę.	1.97	٠,	5	•

6	.00172	*00100	00120	-00142	-00135	.00128	•00122	•00116	•00110	.00105	.00100	• 00005	06000	98000*	.00081		0000	99000	• 00063	09000	.00057	-00054	•00051	•00048	• 00046	.00043	.00041	60003	5000	00033	.00031	.00030	•00028	-00027	• 00025	-00054	.00023	.00021	• 00050	61000	•00018	2000	91000*	\$1000.	*1000°	.00013
۳O	.00173	60100	90100	.00142	-00135	.00129	.00123	.00117	.00111	•00105	•00100	• 0000 •	•00001	98000*	78000	9,000	0,000	99000	• 0000	09000	.00057	.00054	.00051	• 00049	• 00046	-00044	14000	• 00039 75000	5000	00033	•00032	• 00030	• 00028	-00027	-00025	.00024	• 00023	•00021	•00050	•00019	• 00018	1000	• 00016	•00015	400014	
7	.00174		00120	.00143	-00136	• 00129	.00123	•00117	.00111	• 00106	.00101	96000	16000	98000	2000	97000	02000	00067	.00063	09000*	.00057	• 00054	.00051	• 00049	-00046	• 00044	24000.	600039	- 000 s	00034	-00032	• 00030	• 00028	-00027	-00025	•00054	• 00023	• 00052	•00050	61000	•00018	1000.	•00010	-00015	41000	.00013
ખ	.00175	1000	00151	.00144	-00137	.00130	.00124	•00118	.00112	•00106	•00101	96000	.00091	-00087	• 00083	2000	12000	19000	• 00064	• 00061	.00057	• 00054	• 00052	• 00049	• 00046	• 00044	74000	000040	96000	.00034	•00032	• 00030	.00029	00027	• 00026	• 00024	.00023	*00055	• 00021	•1000	• 00018	1000.	•00016	91000	00014	.00013
iD.	.00176	20100	00152	00144	-00137	.00131	•00124	.00118	.00113	.00107	.00102	26000	*0000	18000	• 00083	4,000	12000	89000	• 00064	.00061	.00058	• 00055	• 00052	• 00049	- 00047	-00044	24000	040040	96000	00034	•00032	•00030	• 00059	.00027	• 00026	•00054	• 00023	22000	.00021	00000	•00018	1000.	.00016	• 00016	4000	.00013
4	.00177	90000	00160	00145	-00138	.00131	.00125	•00119	.00113	•00108	•00102	16000	20000	88000	68000	87000	12000	0000	•00064	•00061	•00058	•00055	*00052	•00020	-00047	.00045	75000	040040	96000	00034	•00032	.00031	•00059	• 00027	•00056	• 00025	-00023	•00052	.00021	000050	•00019	*00018	1000	•00016	41000	.00013
ĸ	.00177	.00100	00161	00146	00139	.00132	.00126	•00120	.00114	•00108	.00103	* 0000	• 00003	88000	18000	• 00000	2000	99000	• 00065	•00061	.00058	• 00055	• 00053	.00050	-00047	.00045	24000*	. 00040	96000	00034	.00032	.00031	•00059	.00028	• 00026	.00025	• 00023	*00052	.00021	000050	• 00019	.00018	,1000.	• 00016	41000	
7	.00178	3 6	•00105			.00133		•00120				86000		•00089	\$00084 00004	90000	22000	69000	• 00065	•00062	•00029	•00056	•00053	•0000	.00047	٠.	.00043	000040	א מ	000034	.00033	•00031	•00059	•00028	•00056	.00025	• 00023	•00022	.00021	•00050	┙.	•00018	← .	•00016	-	.00013
_	.00179	.001/1	-00102	.00147	00140	013	012	•00121	.00115	•00100	.00104	66000*	* 0000 *	• 00089	58000	00000	6,000	69000	.00065	•00062	•00029	•00026	.00053	•00020	• 00048	.00045	.00043	.00041	75000	.00035	.00033	•00031	•00059	•00058	•00056	•00025	*0005	*00052	•00021	•00050	_	•00018	.00017	•00016	41000	.00013
0	.00180	1,100.	9100	00148	0014	0013	0012	•00121	.00115	.00110	.00104	600	*0000	600	-00085	9000		64000	99000	-00062	•00029	•00056	•00053	.00051	•00048	• 00046	.00043	.00041		00035	0003	.00031	•00030	•00028	• 00056	• 00025	.00024	•00052	.00021	•00050	•00019	•00018	.00017	•00016	41000	.00014
×	2.01	2.02	Z-03	2,05	2,06	2.07	2.08	0	2.10	2.11	2.12	٦.	2.14	2.15	2.16	71.7	2 10	2.20	2.21	2.22	2.23	2.24	2.25	2.26	2.27	2.28	2.29	2.30	2.51	2,33	2.34	2.35	2-36	2.37	2.38	2.39	2.40	2.41	2.42	4	2.44	2.45	4	2.47	. 4	2.50

ဇ	.00012	.00010	60000	.00008	20000	90000	90000	-00005	.00005	40000	•00004	400004	*0000*	.0000	•00003	• 00003	80000	-00002	• 00002	-00005	-00002	00005	• 00005	20000	.00001	00000	10000	.00001	.00001	10000	10000	.00001	.00001	.00001	.00001	·
sO.	.00012 .00012 .00011	.00010	60000	.00008	-00007	•0000	90000	.00005	. 00005	40000	*0000*	• 00004	40000	£0000°	.00003	.00003	£00003	-00002	.00002	-00005	200005	00005	.00002	• 00002	10000	.00001	.00001	.00001	.00001	10000	00001	.00001	.00001	.00001	00001	•
7	.00012	.00010	60000	.00008	70000°	90000	90000	• 00005	-00005	00000	•0000	. 0000 .	*0000*	00003	• 00003	•00003	£0000°	00003	-00002	• 00002	-00005	00005	-00002	-00005	.00001	.00001	.00001	.00001	.00001	10000	10000	.00001	.00001	.00001	.00001	•
9	.00012	.00010	60000	* 00008	. 00007	90000	90000	.00005	.00005	- 00005	• 0000	.00004	400004	.00003	.00003	.00003	.00003	20000	-00002	-00002	.00002	-00002	-00002	-00002	.00001	.00001	.00001	.00001	.00001	.00001	10000	.00001	.00001	.00001	. 00001	•
ß	.00012	.00010	60000	. 00008	70000°	10000	*0000°	• 00005	.00005	00000	• 00004	• 00004	40000	•00003	.00003	.00003	60000	20000	-00002	-00002	20000	00005	-00002	-00002	.00001	10000	.00001	.00001	.00001	10000	10000	.00001	.00001	.00001	10000	•
4	.00012	.00011	60000	.00008	40000 ·	.00007	•00000	.00005	\$0000	.00005	+0000+	*00004	*0000 *	£0000°	•00003	.00003	• 00003	20000	.00002	*00005	• 00002	-00002	• 00002	-00005	200005	.00001	.00001	•00001	.00001	10000	10000	.00001	.00001	.00001	.00001	•
Ю	.00013	.00011	60000	00000	.00007	.00007	• 00006	90000	\$0000	. 00005	*0000	• 00004	*0000	.00003	• 00003	•00003	£00003	£0000°	-00002	-00002	.0000	00005	-00002	-00002	200003	10000	.00001	.00001	.00001	.0000	10000	.00001	.00001	.00001	. 00001	•
7	.00013	.00011	60000	*00008	10000	88	*0000	88	.00005	• 00005	*0000*	•00004	*0000	.00003	•00003	•00003	.00003	00003	.00002	-00002	• 00002	• 00002	8	-00005	20000	10000	.00001	.00001	.00001	10000	.00001	.00001	.00001	.00001	.00001	•
_	.00013	.00011	60000	*0000	*00008	.00007	90000	90000	.00005	00000	*0000*	•0000¢	40000	.00003	.00003	.00003	£00003	00003	-00002	00000	• 00002	.00002	-00002	-00002	70000	0000		.00001	10000	10000	10000	.00001	.00001	.00001	10000	•
0	.00013	.00011	60000	0	*00008 *00007	.00007	90000	90000	• 00005	• 00005	*0000*		\$0000°	.00003	.00003	.00003	60000	00000	*00005	-00002	• 00002	*00002	.00002	*00005	20000	00000	.00001	.00001	10000	10000	00001	.00001	.00001	.00001	.00001	.00001
×	2.52		2.57				2.63	•	9.		9	•		2.73	2.74	2.75	2.76	1 ~	~	2.80	2.81	2.83	2.84	2.85	7.86	2.88	2.89	2.90	0 0	2.92	0	•	2.96	2.97	2,98	0

Table 2

Values of:

$$s = \frac{Q}{2\pi KD} \frac{1}{\sigma} \left[1 - \sqrt{1 - 2\sigma} \int_{-\sqrt{4\sigma t}}^{\infty} \frac{\bar{e}^{u^2} du}{u} \right]$$

where

$$\sigma = \frac{Q}{2\pi K D^2}$$

The following check values have been computed using values of

$$\int_{-\sqrt{4\alpha t}}^{\infty} \frac{e^{-u^2} du}{u}$$

taken from Table 1.

$(\frac{r}{\sqrt{4\alpha t}})$	$\int_{-\sqrt{4\alpha t}}^{\infty} \frac{\bar{e}^{u^2} du}{u}$	σ	Computed Value of $\frac{s}{(\frac{Q}{2\pi KD})}$	Tabular Value of $\frac{s}{(\frac{Q}{2\pi KD})}$
0.00001	11.22432	0.01	11.93676	11.93675
0.00010	8.92173	0.02	9.90228	9.90228
0.00100	6.61915	0.03	7.45217	7.45217
0.00500	5.00972	0.04	5.64764	5.64764
0.01000	4.31661	0.05	4.92235	4.92235
0.05000	2.70837	0.06	2.97365	2.97365
0.10000	2.01896	0.07	2.18625	2.18625
0.50000	0.52214	0.08	0.53353	0.53353
0.70000	0.28604	0.09	0.28982	0.28982
1.00000	0.10969	0.10	0.11030	0.11030

The above formula and its derivation are described in the paper by Glover and Bittinger. Trans. ASCE, Vol. 126, Part III, 1961. Paper No. 3142.

This development relates to the drawdowns around a well pumping at the constant rate $\,Q\,$ from an aquifer of unlimited extent where the drawdowns $\,s\,$ are not small compared to the original saturated depth $\,D\,$.

<u> </u>		Table 2.	223
010	Aport Aport	8.14208 7.44368 6.96599 6.59989 6.30301 4.03075 3.23003 2.96665 2.75169 2.75169 2.27169 2.27169 1.01042	.53653 .39503 .29026 .21207 .15370
60.	07 - 1/-	10.22893 8.43324 7.68835 6.68318 6.35993 6.09205 5.86401 4.55886 3.90684 2.70152 2.24596 1.43304 1.00481	.53502 .39422 .28982 .21185 .15358
80		9.65395 8.22956 7.45733 6.53161 6.53637 6.53637 6.53694 3.73975 5.54768 3.73975 2.85194 2.48790 2.48790 2.48790 2.48790 2.48790 2.48790 2.48790 2.48790 2.48790 2.48790 2.48790 2.48790 2.48790 2.48790	.53353 .39342 .28939 .1162 .15346
A	13.67898 12.10113	11. 45951 10. 82843 10. 82843 7. 47748 6. 89847 6. 89847 5. 29867 5. 29862 7. 45049 2. 80125 1. 41024 . 99395	.53205 .39263 .218897 .15334
90•		9.561674 9.34359 9.107676 6.98369 6.98369 6.50059 5.62120 5.62153 7.62153 2.67364 2.67364 2.67364 2.75416	.53059 .39184 .28854 .21116 .15323
SIGMA 6-05-05-17-45460	16.07510 15.23579 14.62750 14.15150 13.76171 13.78250 11.58250 10.07113 9.63138 9.28547	8.76130 8.35040 8.35040 7.23449 6.61424 6.19111 5.81162 5.61585 5.06275 7.06275 7.024991 2.33357 2.33357 2.33357 1.38885 1.38885	.52914 .39105 .28812 .21094 .15311
.04 17.01350 15.07823 14.10437	12.98934 12.61565 12.30828 12.04793 11.82254 11.82254 10.38594 9.24474 8.89456 8.61399	6.18080 6.86991 6.31917 6.3	.52771 .39028 .28770 .21071 .15299
.03 14.28541 13.10877 12.45119		7.4118 7.58823 7.58823 6.07392 6.07392 5.25629 5.25629 4.65948 4.65948 7.6269 7	.52630 .38951 .28728 .21049 .15287
217			.52490 .38874 .21687 .1527 .1527
.01 11.93675 11.15313 10.69793	000000000000000000000000000000000000000	6.09391 6.8564 6.11283 5.68196 5.3745 6.1283 6.1283 6.64775 6.64775 6.64775 6.64775 7.608 2.55935 2.55935 2.16642 2.16642 2.16642 2.16642 2.16642 2.16642 2.16642 2.16642 2.16642 2.16642 2.16642 2.16642 2.16642 2.16642 3.6602	.52351 .38798 .28645 .21005 .15264
400+ 000001 000002			.50000 .60000 .80000 .90000

Table 3

Values of:

$$\frac{y}{y_0} = \begin{bmatrix} & & & & & \\ & 1 - \sum_{n=1}^{\infty} A_n U_0 & (\beta_n r) & e^{-\frac{k^2 (\beta_n b)^2}{4}} & (\frac{4\alpha t}{a^2}) \end{bmatrix}$$

Where:

y represents the pressure drawdown produced by a flowing artesian well at the radius r and the time t . The drawdown is expressed in terms of an equivalent depth of water.

 y_0 the value of y at the well where r = a.

The condition that $U_0 = 0$ when r = a is met by the Bessel function:

$$U_0(\beta_n r) = J_0(\beta_n a) Y_0(\beta_n r) - Y_0(\beta_n a) J_0(\beta_n r)$$

If:

$$A_{n} = \frac{\frac{2k}{(\beta_{n}b)} U_{0}^{\dagger}(\beta_{n}a)}{[U_{0}(\beta_{n}b)]^{2} - k^{2}[U_{0}^{\dagger}(\beta_{n}a)]^{2}}$$

The β_n values are roots of

$$U_0^{\dagger}(\beta_n b) = 0 .$$

The development applies where a flowing artesian well, of radius a , taps a confined aquifer having an impermeable outer boundary at the finite radius b . When the well is shut down the pressure at the well rises toward the terminal value y_0 , expressed in terms of the depth of water which would exert the same pressure. The case where the outer boundary b is infinitely remote is approached by using a series of finite cases with successively greater outer radii. The finite case can represent the infinite

case until the disturbance produced by the flow of the well reaches the outer boundary b. The sequence used here is as shown below:

(b/a)	Computa Range (r/	of
25	3 to	6
50	6 to	12
100	12 to	25
200	25 to	50
400	50 to	100
800	100 to	200
1600	200 to	400
3200	400 to	800
6400	800 to	1600
12800	1600 to	3200
25600	3200 to	10000

This sequence provides an essentially exact solution until suitable approximation formulas can be used.

The β_n roots were computed by use of a root seeking digital computer program. When these roots were obtained the A_n values were computed and the computation of the tables was performed by means of the digital computer. An independent check is obtained by comparison with values for (b/a) = 100 given in Bulletin 3, Part VII of the Boulder Canyon Project Final Reports where an analogous heat flow case is treated. A comparison of independently computed and machine computed A_n values for (b/a) = 100 is shown below

	Independently Computed A	Digital Computer
n	Values* n	A Values**
1	0.399352	0.399352
2	0.092067	0.092067
3	0.069189	0.069189
4	0.059264	0.059264
5	0.053526	0.053522

	Independently	
n	Computed A _n Values*	Digital Computer A _n Values**
••	101005	
6	0.049725	0.049725
7	0.047009	0.047009
8	0.044962	0.044962

^{*} The first five values are as given in Bulletin 3. The values for n = 6,7 and 8 were computed by this writer.

**Last figure rounded.

Checks of the (y/y_0) ratio can be obtained by comparison with the approximation formulas of Chapter 4. For example, for (b/a) = 25600 (r/a) = 2000 $(\sqrt{4\alpha t}/a) = 5000$, the tabular value of (y/y_0) is 0.08969. The approximation formula gives:

$$\frac{y}{y_0} = \frac{\frac{e^{-u^2}du}{u}}{\int_{-\infty}^{\infty} \frac{e^{-u^2}du}{u}} = \frac{0.70459}{8.22859} = 0.085627$$

$$\frac{e^{-u^2}du}{u}$$

$$\frac{e^{-u^2}du}{u}$$

where

$$\left(\frac{a}{\sqrt{4\alpha t}}\right) = \frac{1}{5000} = 0.0002$$
 $\left(\frac{r}{\sqrt{4\alpha t}}\right) = \frac{2000}{5000} = 0.4$

A further check can be made by computing some (y/y_0) values using the independently derived A_n values. The following results are obtained:

For
$$(b/a) = 100 \quad (\frac{r}{\sqrt{4\alpha t}}) = 25$$

$(\frac{\mathbf{r}}{\mathbf{a}})$	Computed Values of (y/y ₀)	Tabular Values of (y/y ₀)
2.0	0.77655	0.77656
5.0	0.48248	0.48249
10.0	0.26768	0.26768
20.0	0.08802	0.08803
50.0	0.00103	0.00103
100.0	0.0000	0.00000

	Table 3.	(y/y <u>)</u>		
	.00000 .00000 .00050 .00376	10. 01195 02482 04089 05862 07689	10. 11239 12917 15917	17390 18703 19936 221094 22208 24174 25087 25951
	9. .0006 .00167 .00863	9. .02213 .04033 .06092 .08219 .10317	9. .14208 .15977 .1667	.20589 .21919 .24317 .25400 .26417 .27371 .28270 .29118
	8. .00036 .00506 .01855	8. .03929 .06365 .08891 .11353 .13679 .15842	8. •17837 •19673 •21361	24348 25674 2603 26045 29109 30103 31035 32732
	7. .00186 .01382 .03740	7. .06695 .09774 .12733 .15475 .17975 .20241	7. .22291 .24151 .25841	.28794 .30091 .32395 .33423 .34423 .35275 .36113 .3610
	6. 00795 03410 07107	6. .10991 .14637 .17935 .20459 .23446 .25746	6. .27784 .29613 .31260	.34108 .35348 .35487 .35537 .39510 .40255 .41780
<u>_</u>	5. .02817 .07640 .12785 .17451	5. •17451 •21484 •274934 •274934 •30447 •32674	5. .34631 .37914 .39308	443736 443736 443736 443736 444528 46626 46626 47617
	4. .08370 .15698 .21926 .26961	4. .26961 .31037 .34384 .37178 .39545 .41580	4. .43351 .44909 .46292	.48646 .49659 .51431 .52513 .52937 .53609 .54236 .54822
	3. .21228 .29964 .36253	3. 44935 47438 49452 49400 51776 53458	3. .54911 .56182 .57306	.59210 .60026 .61770 .61451 .62677 .63195 .64603
	2, 47392, 54433 59017 62265	2. 62266 64706 64706 68169 69454 70545	2. *71478 *7294 *73014	.74230 .74749 .75223 .75656 .76054 .76054 .77081 .77081
	1,00000 1,00000 1,00000 1,00000	1.00000 1.00000 1.00000 1.00000 1.00000	1.00000 1.00000 1.00000	1.00000 1.00000 1.00000 1.00000 1.00000 1.00000 1.00000
(ART)	2 2	۲۲ م و ۱۱۵ د د د د د د د د د د د د د د د د د د	12.0 113.0 15.0	77 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0
	52	200	(७/९) 1 _.	7001

	10.	.26768	•27544	.28281	.28982	.29650	.30288	.30896	.31479	•32036	.32571	.33084	.33577	.34051	.34508	.34948	.35372	.35781	.36177	•36559	•36959	.37287	.37634	.37971	.38297	.38614	.38922	
	.	• 59919	.30678	.31397	.32080	.32730	.33349	.33940	.34505	.35045	.35562	.36058	.36535	.36993	.37433	.37857	.38266	.38660	.39041	.39409	•39765	.40110	.40443	.40767	.41080	.41385	.41680	
	ϡ	•33508	.34241	.34935	*35594	.36220	.36415	.37383	.37924	.38442	•38938	.39413	.39869	•40306	.40727	.41132	.41523	.41899	•42262	.42613	.42953	•43281	•43288	.43907	•44205	.44495	•44776	
	7.	.37641	.38340	.39000	.39626	.40221	.40785	.41323	.41836	.42327	.42795	•43544	.43675	.44088	.44485	.44867	.45235	.45590	.45932	.46263	•46582	.46891	.47190	.47480	.47761	.48034	.48298	
		•45474	.43127	.43744	.44329	.44882	•45409	•45909	•46386	.46842	.47277	•41694	*48094	.48477	.48845	66167	.49541	69867	.50186	-50492	•50788	.51074	.51351	•51619	.51878	.52130	.52375	
-Je		.48249	.48843	*49404	76667°	.50437	.50914	.51368	.51800	.52212	.52606	.52983	.53344	.53690	.54023	.54343	.54651	.54948	.55234	.55510	•55776	.56034	.56284	.56525	.56760	.56987	.57207	
	• 4	.55371	.55889	.56376	.56837	.57273	.57687	.58080	.58455	.58812	.59153	.59479	.59792	26009*	.60379	•60656	.60922	.61179	.61426	.61665	.61895	.62118	.62334	• 62542	•62745	.62941	.63131	
	3.	.64403	91059*	•65405	. 45772	.66119	65759.	-66762	.67060	***	.67616	.67876	.68124	.68362	.68591	.68411	.69023	75569.	.69423	.69613	96269*	.69973	.70144	.70309	.70470	.70626	.7077	
	٠.	•77656	11611.	.78164	.78396	.78616	.78825	.79023	. 79211	16861	.79563	75727	. 79884	.80035	.80180	.80318	.80452	.80581	.80705	. A0A25	.80941	.81052	.81160	.81265	.81367	. P1465	.81560	
	:	1.00000	1.0000	1.00000	1.00000	1.00000	1.0000	1.0000	1.00000	1.0000	1.00000	1.0000	1.0000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.00000	1.0000	1.00000	1.00000	1.00000	
	5	0.5%	24.0	27.0	78 . n	29.0	30.0	31.0	32.0	33°u		35.0		37.0	38.0	9.0 (V				43.0	44.0	45.0	46.0	47.0	48.0	0.67	50.0	

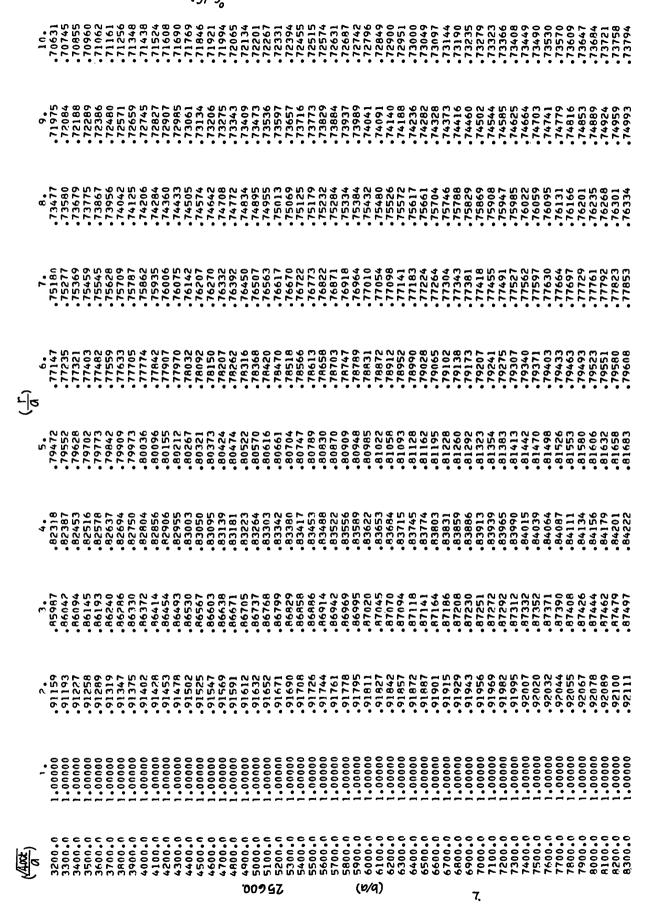
	10.	3892	3986	31586.	4007	033	4060	4085	110	134				.42261			•42894						.44223	.44399	4457	•44739	*******	.45067	•45226	.45383	15554°	45837	.45983	.46127	.46268	•46407	*40244	8,6678	17044	4707	4719	4732	744	47	768	780	791	.48030
	•6	.41680	80614	14224	CA764	643040	.43291	.43535	.43773	90044.	•44233	•44424	.44671	.44882	•45089	.45291	•45488	45682	1784.	00000	46630	046590	.46761	62694	.47093	•47254	.47413	•47568	.47721	.47871	81084.	48305	*4844	.48582	.48717	.48850	18887	60164	1964	.49483	40967	.49723	96	499	0		.50292	.50401
	æ	477	505	445315 45573	4500	507	4630	654	4676	869	.47203	_	761	•47819	•48016	.48208	.48395	.48579	48758	******	70164	4964	.49603	•49762	.49918	.50071	.50221	•50369	.50513	.50655	26/05.	51067	.51200	.51330	.51458	.51584	51708	.51830	52068	.52184	.52299	.52412	.52523	.52632	•52740	• 52846	.52951	•53055
	7.	.48298	4444.	20884°	C 8 C O 7	49513	49737	49955	.50168	.50375	.50578	.50775	• 50968	.51156	.51341	.51521	.51697	.51869	.52037	20226	50000	52678	.52830	.52979	.53125	.53268	.53409	.53547	.53683	.53816	13941	.5400	.54326	.54448	.54569	.54687	50845	71647.	55140	55249	.55357	.55462	.55566	•55669	•55770	.55870	.55968	.56065
· T-	•	.52375	52612	.52843	2000	53498	.53704	•53906	.54102	.54294	.54481	.54663	.54841	•55015	.55185	.55351	.55513	.55672	82856	00175	57075	56419	56559	.56696	.56831	•56964	.57093	.57221	•57346	.57469	065/5	.57825	.57939	•58052	.58162	.58271	.58379	• 58484 Cores	58690	.58790	.58889	.58986	.59082	.59177	.59270	-59362	.59452	•59542
-Je	5.	.57207	.5/421	62975	70000	58219	58405	.58586	.58763	.58935	.59104	.59268	.59428	.59584	.59738	.59887	.60033	9/109	•60316	60000	60209	60848	42609	.6109A	.61219	.61338	•61455	•61569	.61682	.61792	10619.	62112	.62215	•62316	•62416	.62514	01929.	•62/02 •2708	62890	62980	63069	.63157	.63243	.63328	.63412	•63464	•63576	•63656
	4	.63131	.63316	.63495	62000	40049	.64165	.64321	.64474	.64623	.64768	60679	.65048	.65183	•65315	.65444	.65570	.65693	•65814	26660	64149	.66272	66381	.66488	.66592	\$6999	96299	.66895	26699	.67087	.6/180	67363	.67451	•67539	•67625	•67709	26//9.	47874	68033	.68111	.68188	.68263	.68337	.68411	.68483	•68554	•68624	.68693
	÷.	. 70777	10923	71000	05512	71470	79517	.71722	.71843	.71961	.72076	.72188	.72298	.72405	.72510	.72612	.72712	. 72810	90627	19067	12067	69267	73356	.73440	. 73523	.73605	.73684	.73763	.73840	.73915	34,043	74134	74204	.74274	.74342	.74409	5/55/	. 74539	74666	74727	74788	.74848	74907	.74965	.75022	.75079	.75134	.75189
	ζ.	.A1560	. P1653	. R1743	00010	81998	82079	.82157	. A2233	. A2308	.82381	. 82452	. A2521	.82589	.82655	.82719	.82782	444CH.	. 8790.	*07.00	22050	45158	83188	.83242	.83294	.83346	.83396	.83446	. A3494	.83542	983539	635680	, A3724	.83768	.83811	.83853	. R3A95	.83936 92926	84015	84054	84093	. 84131		4	47	427	43	.84346
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	10.	.60194	•60360	• 60522	•60678	.60829	•60976	•61118	.61257	.61392	.61523	•61650	•61774	.61895	.62013	.62129	.62241	.62351	•62458	•62562	•62665	•62765	.62863	•62958	•63052	.63144	.63234	.63322	•63409	•63464	•63577	•63658	•63739	.63817	•63834	.63970	•64045	.64118	.64190	.64261	.64331	•64399
	•6	.62015	•62174	•62328	.62477	.62621	.62761	.62897	.63030	.63158	.63283	•63405	•63523	•63639	•63751	.63861	•63669	.64073	.64175	•64275	.64373	.64468	• 64562	.64653	.64743	.64830	•64916	.65001	.65083	.65164	•65243	•65321	•65398	•65473	.65547	•62619	•65690	•65760	•65859	•65896	.65963	602
	80	.64051	.64201	.64347	•64488	•64625	.64757	•64886	.65011	.65133	•65251	•65366	•62419	•65588	•62694	•62199	•65900	•62669	96099•	.66190	•66283	•66373	.66461	•66548	•66633	•66716	.66797	.66877	•66955	.67031	•67106	.67180	•67253	•67324	.67393	•67462	•67529	•67595	.67660	•67754	.67787	.67849
	7.	•66359	•66500	•66636	.66768	96899*	.67020	.67141	•67258	.67372	.67483	.67590	•67695	.67798	.67897	.67995	06089*	.68182	.68273	.68361	.68448	•68532	.68615	•68696	•68775	.68853	.68929	* 0069 *	.69077	.69148	•69519	.69288	•69355	.69422	.69487	.69551	•69614	•69676	.69737	.69797	•69856	•69914
←	•9	•69024	•69154	•69279	.69401	•69518	.69633	•69744	•69852	95669*	.70058	.70158	.70254	.70349	.70440	.70530	.70617	.70703	.70786	.70868	.70947	.71025	.71101	.71176	.71249	.71320	.71390	.71459	•71526	.71592	.71657	.71721	.71783	.71844	•71904	.71963	.72021	.72078	.72134	.72190	.72244	. 72297
_[¤	S	•72176	.72292	.72405	• 12514	.72620	.72723	.72822	•12919	.73014	.73105	.73194	.73281	•73366	.73448	.73529	.73607	.73684	•73759	.73832	.73903	.73973	.74042	.74109	.74174	.74239	.74302	.74363	.74424	.74483	.74541	•74598	.74654	• 14 709	.74763	.74816	.74868	.74920	.74970	.75019	.75068	•75116
	4.	.76034	.76134	.76231	•76325	.76416	.76505	.76591	. 76674	•76755	.76834	.76911	.76986	.77058	.77130	.77199	•77266	.77332	.77397	.77460	.77522	.77582	.77641	.77699	. 17755	.77810	.77865	.77918	0.1477	.78021	.78071	.78120	.78168	.78216	.78262	.78308	• 78353	.78397	.78440	.78483	.78525	92
	3.	_	.81087	~	.81238	_	_	.81448	.81515	.81579	.81641	.81702	.A1761	.81819	.A1876	.81930	. 81984	.82036	.82088	.82138	.82186	.82234	.82281	.82326	17528.	.82415	.82458	.82500	.82541	.82582	.82622	.82661	.82699	. 82736	.82773	.82809	.82845	.82880	.82914	85948	.82981	.83014
	2•	.88017	.88067	.88116	.88163	.88208	.88252	. 8R295	.88337	.88378	.88417	.88455	.88493	.88529	• AA565	.88599	.88633	.8A666	.88699	.88730	.88761	.88791	.88820	. 88849	.88878	. A8905	.88932	• AA959	.88985	.89010	.89035	.89060	.89084	.89108	.89131	.89154	. 89176	.89198	.89220	.89241	.89262	.89283
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$\left(\frac{\sqrt{4\infty t}}{a}\right)$		400.0	410.0	420.0	430.0	0.044	6.054	460.0	470.0	480.0	6.067	500.0	510.0	520.0	530.0	540.0		260.0		580.0	6.00	400.0	610.0	620.0	630.0	_	650.0	0.099	670.0	6.00.0	490.0	700.0	710.0	720.0	730.0	740.0	750.0	760.0	770.0	780.0	790.0	800.0

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	10.	•65058	.65573	.66051	•66476	•66858	•67205	.67520	•67811				10.	.67811	.68079	.68327	.68559	.68775	•68949	.69170	.69351	.69522	•69684	•69839	•69986	.70126	.70261	.70389	.70513	.70631
	9.	• 66629	.67148	•67604	.68010	•68375	•68705	.69007	•69283				.	•69284	.69539	•69776	26669*	.70204	.70398	.70581	.70753	.70917	.71072	•11219	•71359	.71493	.71621	.71744	.71862	.71975
	8. .67849	.68417	• 68409	.69341	•69725	.70070	.70383	•70668	.70930				.	.70930	.71172	.71397	•71606	.71801	.71985	.72158	.72321	.72476	.72622	.72762	.72895	.73021	.73143	•73259	.73370	.73477
	7.	.70445	• 70906	.71310	.71669	.71992	.72285	.72552	.72797				7.	72797	.73023	.73233	.73429	.73612	.73784	.73946	.74099	.74243	.74380	.74511	.74635	.74754	.74867	•74976	.75080	.75180
<u>1</u> a	.72297	.72787	.73210	.73583	.73914	.74211	.74480	.74726	.74952				•	.74952	.75160	.75354	.75534	.75702	.75861	.76010	.76150	.76284	.76410	.76530	.76645	.76754	•76858	•76958	.77054	.77146
		.75556	•75936	.76271	.76568	• 76835	.77077	.77298	.77501				5.	.77501	.77688	.77862	.78024	.78175	.78317	. 78451	.78577	.78697	.78810	.78918	.79021	.79119	. 79213	.79303	. 79389	. 19472
	4.	.78945	. 79273	. 79561	.79817	.80047	.80255	.80445	•80620				• 4	.80620	.80781	.80931	.81070	.81201	.81323	.81439	.81547	.81650	.81748	.81841	.81930	.82014	.82095	.82173	.82247	.82318
	3.	.83314	.83574	.83802	.84005	.84187	.84353	.84503	*84642				e,	-84642	.84770	.84888	66658	.85102	.85199	.85290	.85377	.85458	.85536	.85609	.85680	.85747	.85811	.85872	.85931	.85987
	2° 89283	.89473	.89636	.89780	.8990R	.90023	.90128	.90223	.90310				~	.90310	16606	.90466	.90535	.90601	.90662	.90719	*42406*	.90825	.90874	.90921	• 90965	.91007	.91048	.91086	.91123	.91159
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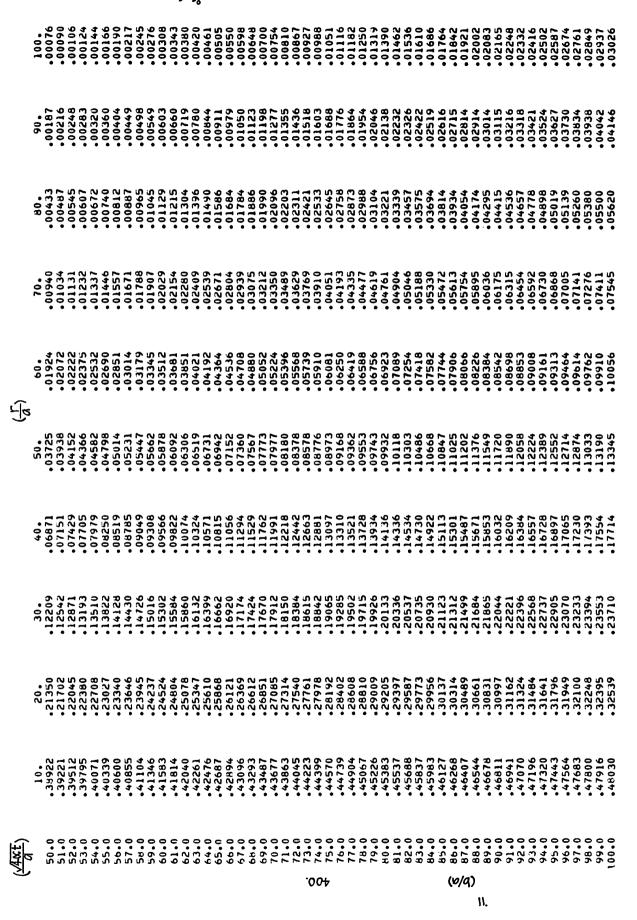
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WRP Classic Resource Edition



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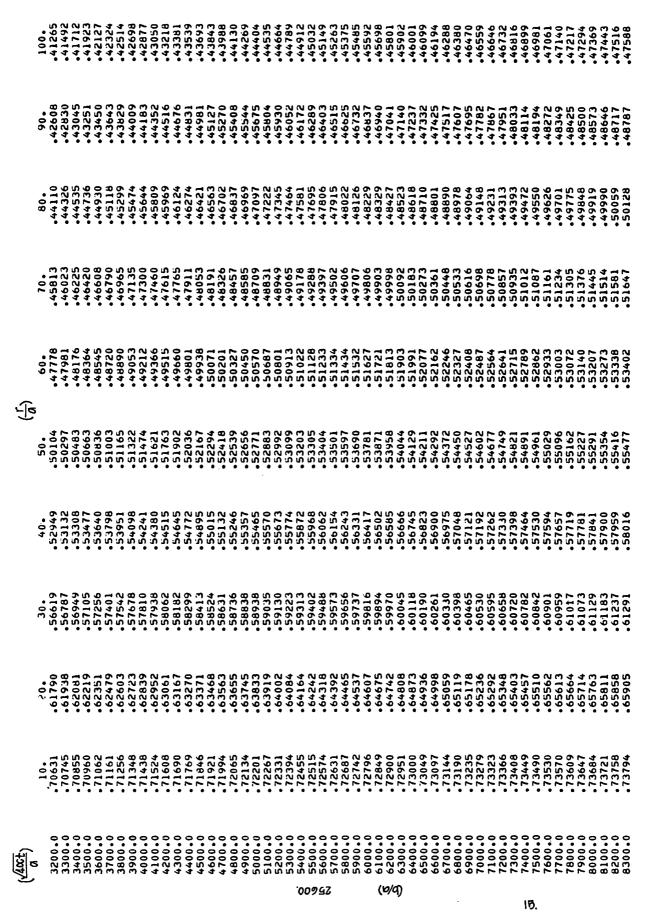
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$(\frac{\sqrt{4\kappa t}}{a})$	•	400.0	410.0	420.0	•	0.044	450.0	0.094	470.0	480.0	0.064	200.0	510.0	520.0	530.0	540.0	550.0	260.0	570.0	280.0	290.0	0.009	610.0	Ð		40.	6 50 •	0.099	670.	S 680.0	. 069	700	710.0	720.0	730.0	740.0	750.0	760.	0.022	780.	•	800.0

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	100.	.28864	.30107	.31185	.32134	.32979	•33739	34427	.35057	.35635			100.	.35635	.36169	.36664	•37126	.37559	.37965	.38347	.38708	64066.	.39374	.39682	.39976	.40257	•40525	.40782	.41028	•41265
(^r)	90•	.30479	.31696	.32752	.33680	.34507	.35250	35924	.36539	.37105			•06	.37105	.37627	.38111	.38563	.38986	.39383	•39756	60105	.40443	•40760	.41061	.41348	.41623	.41885	•42136	.42377	.42608
	80.	.32288	•33476	• 34505	.35411	.36217	.36942	37598	38198	.38749			80.	.38749	.39258	.39730	.40170	.40582	89607*	.41332	•41676	•42001	.42310	.42603	.42883	.43150	•43405	.43650	•43884	.44110
	70.	.34342	•35496	.36496	.37375	.38157	.38860	19497	62004	.40613			70.	.40613	.41107	.41565	.41992	.42391	.42766	.43119	•43452	.43768	.44067	.44352	.44623	•44882	.45129	.45367	•45294	.45812
	•09	.36717	.37830	.38795	.39643	•40398	•41076	06917	•42251	.42766			•09	.42767	•43545	.43684	.44095	.44480	•44845	•45182	.45503	.45807	96094	.46370	.46632	•46881	•47120	.47349	.47568	.47778
		•39528	•40504	.41517	.42328	•43049	•43698	.44285	.44821	.45314			50.	.45314	.45769	.46191	•46584	•46952	.47297	•47622	67624.	•48220	•48496	•48758	*49008	•49246	7. 464.	•49693	20665	.50103
	•04	.42973	643649	.44850	.45615	•46296	.46908	-47462	.47968	.48432			*0*	.48432	.48861	.49259	.49630	11664.	.50303	.50610	66805	.51173	.51433	.51681	.51916	.52141	•52356	.52562	.52760	.52949
	30.	.47417	•48345	64164.	.49855	.50483	.51047	.51558	.52025	.52453			30.	.52453	.52849	.53216	.53558	.53878	.54178	.54461	.54728	.54981	.55221	• 55449	.55666	.55873	.56072	.56261	• 56444	.56619
	20.	.53684	.54502	.55210	.55832	.56385	.56882	.57333	.57743	.58121			20•	.58121	.58469	.58793	*29094	.59376	.59640	.59890	•60125	.60347	•60229	•60159	.60951	.61134	.61308	•61476	.61636	.61790
	10.	•64366	•65028	.65573	.66051	. 56476	.66858	•67205	.67520	.67811			10.	.67811	.68079	.68327	•68226	.68775	•68919	.69170	.69351	.69522	•6969•	•68839	98669*	.70126	.70251	.70389	.70513	. 70631
$\left(\frac{\sqrt{4\kappa t}}{a}\right)$	•	800.0	0.006	1000.0	1100.0	_				1600.0		lala		1600.0	1700.0	1800.0	1900.0	2000.0	5 2100.0				2500.0	2600.0	2700.0	2800.0	2900.0	3000.0	3100.0	3200.0
							סס	/∀'	7			(19/9)	r						U	08	s <i>(</i>	1				14	¥,			



											٠,	1	0						
	100.	•47659	.47729	.47798	•47866	.47933	66624*	•48064	.48128	.48191	.48254	.48316	.48377	.48437	96585	.48555	.48613	.48670	
	•06	.48857	•48925	26684*	•49029	.49124	88167*	•49252	.49315	.49377	.49438	86767	.49558	.49616	.49675	.49732	.49788	***	
- <u> </u> <u> </u>	80.	.50195	.50262	.50327	.50392	•50456	.50518	.50580	.50641	.50702	.50761	.50820	.50878	.50935	.50992	.51048	.51103	.51157	
	70.	.51713	.51777	.51841	.51904	.51965	.52026	•52086	.52146	•52204	.52262	.52319	.52375	.52430	.52485	•52539	.52593	.52645	
	•09	•53465	.53527	.53588	.53649	.53708	.53767	.53825	.53882	.53938	.53994	.54049	.54103	•54156	•54209	.54261	.54313	•54364	
·	50•	.55537	.55596	.55655	.55713	•55769	.55826	.55881	.55935	.55989	.56042	.56095	.56147	.56198	.56248	•56298	.56347	•56396	
	•0•	.58073	.58129	.58184	.58239	.58292	.58345	.58397	.58449	.58500	.58550	.58599	.58648	•58696	.58744	.58791	.58837	• 58883	
	30.	.61343	•61395	•61445	•61496	•61545	•61594	•61642	.61689	.61736	•61782	•61828	.61873	1619.	.61961	•62004	.62047	.62090	
	20 •	•65951	.65997	.66042	•66086	•66129	.66172	•66215	•66256	•66598	•66338	.66378	.66418	.66457	•66496	.66534	•66572	60999*	
	10.	.73829	.73864	.73899	.73933	.73966	.73999	.74032	.74064	•74096	.74127	.74158	.74188	.74218	.74248	.74277	.74306	.74335	
		8400.0	8500.0	8600.0	8700.0	8800.0	8900.0			0°0026 G		0.0046		0.0096		0.0086	0.0066	10000.0	



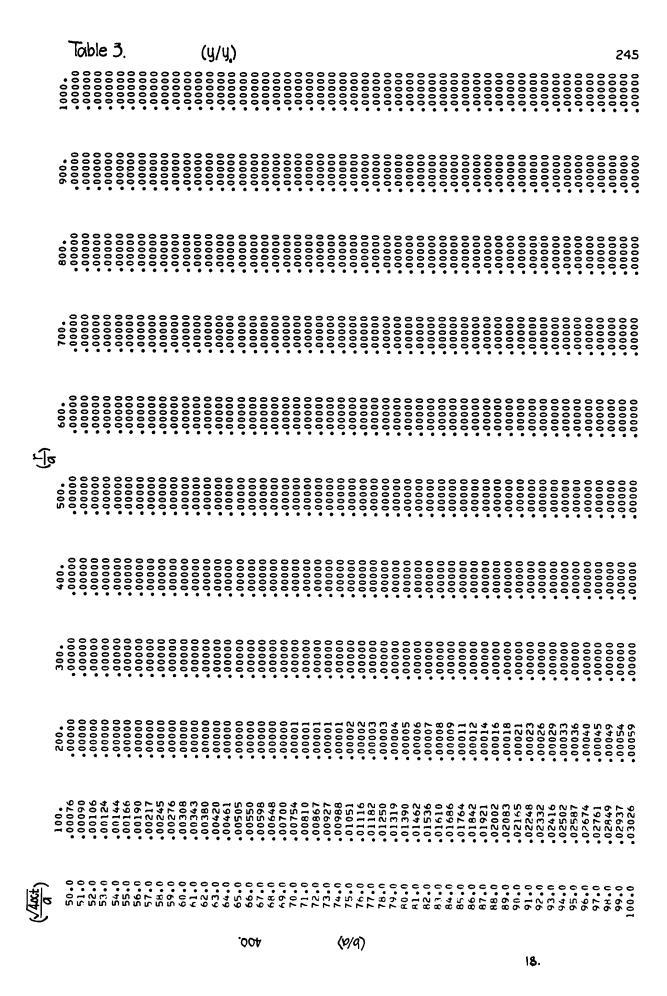
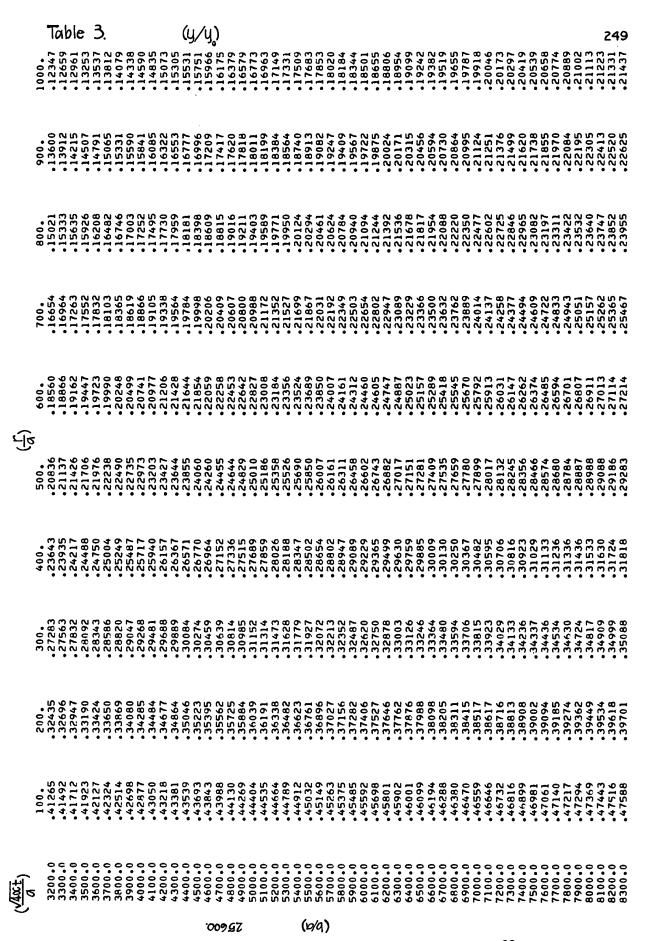


Table 3.	(y/y) e			246
	000			.00003 .00006 .00006
00000000000000000000000000000000000000	000		000001	.00014 .00019 .00025 .00033
70000	700	000000000000000000000000000000000000000	00000 00000 00000 00000 00000 00000 0000	.00055 .00087 .00107 .00107
- To	• 000	4 000000 00000 000000 0000000000000000		.00189 .00227 .00269 .00315
5000 • 00000 • 00000 • 00000 • 00000 • 00000 • 00000 • 00000	• 000	.00007 .00012 .00012 .00031 .00046	.00100 .00107 .00197 .00288 .00358	.00573 .00556 .00745 .00745
40000 00000 00000 00000 00000 00001 00000 00001	• 00	.00047 .00075 .00110 .00154 .00271 .00344	.00521 .00623 .00734 .00853 .01114 .01114	.01550 .01706 .01865 .02028
300. .00004 .00012 .00012 .00053 .00149 .00122	• 00 00 00	.00557 .00557 .00567 .00567 .01285 .01285	02327 02327 02327 02365 03350 03250	.04038 .04038 .04286 .04532
200. 00059 00132 00610 00610 00616 01150 01150	•	. 02545 . 02931 . 03325 . 04123 . 04523 . 04920	.05/04 .06088 .06467 .07206 .07565	.08601 .08601 .09257 .09257 .09575
1000 03026 03430 06849 06554 06554 08355 10659	000	11361 12032 12674 133267 14437 14975	.15988 .16465 .16924 .17365 .17791 .18201	.19348 .19348 .20051 .20386
2000 1000 1200	1	22000 23000 23000 24000 24000	009	

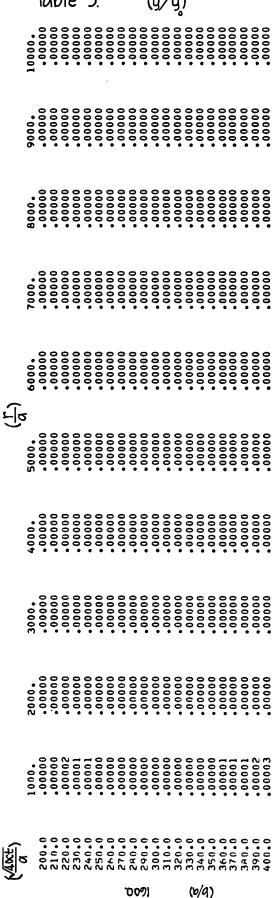


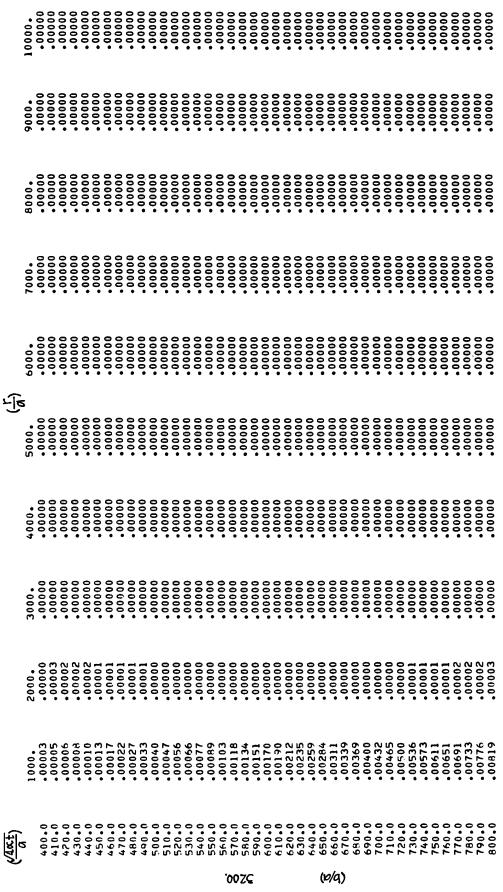
.00ZE

	7	ab	le	:	3.					(y	/y)																			
	1000.	01305	.01856	.02447	•03056	.03670	.04279	•04875	• 05456		•		1000	•05456	.06018	•06561	.07085	•07589	.08073	• 08239	.08987	.09418	.09833	.10232	.10617	.10988	.11345	.11691	.12024	.12347
	900	01890	.02558	.03248	.03939	.04621	• 05285	• 05926	• 06544				•006	• 06544	.07137	•0770	.08250	.08771	.09270	• 09748	•10205	•10644	.11065	.11470	.11859	.12233	.12594	.12941	.13276	.13600
	800.	02636	.03488	•04276	•05048	.05793	•06508	.07191	.07842				800	.07842	.08461	.09051	.09613	.10147	.10657	.11144	.11608	.12053	.12478	.12886	.13278	.13653	.14015	.14363	.14698	.15021
	700.	.03797	.04711	•05594	• 06439	.07242	.08001	•08719	.09397				200	.09397	.10038	.10644	.11218	.11763	.12280	.12772	13241	.13688	.14116	•14524	.14916	.15291	.15652	.15999	.16332	.16654
		.05289	.06316	.07285	.08194	*000	• 09839	.10583	.11281				•009	.11281	.11937	.12554	.13136	• 13685	•14206	.14700	.15169	15616	•16042	.16449	.16838	.17211	.17568	.17912	.18242	.18560
<u>, , , , , , , , , , , , , , , , , , , </u>	500.	.07308	.08433	.09471	.10429	.11313	.12132	.12893	.13602				200 •	•13602	.14265	.14885	.15468	.16018	.16536	.17027	.17493	.17935	.18357	.18758	.19142	.19510	•19862	•20199	•2025	.20836
	400.	10070	.11265	.12348	.13334	.14236	.15065	.15829	.16538				*00*	.16538	.17198	.17813	.18390	• 18932	.19442	.19924	.20381	.20814	•21226	•21619	.21993	.22352	*55694	.23023	•23339	.23643
	300.	13949	.15176	.16273	.17261	.18157	.18976	.19728	.20421				300.	.20421	.21065	.21664	. 22224	.22749	-23242	•23708	.24148	.24566	.24963	.25340	.25700	.26044	.26373	•26689	.26992	.27283
	200.	19767	.20970	.22034	.22985	.23844	.24624	.25338	• 25996				200-	•25996	.26604	.27169	•27696	.28190	.28654	.29091	*59504	.29895	.30267	.30620	.30957	.31278	.31586	.31880	.32163	.32434
	100.	.30107	.31185	.32134	,32979	.33739	.34427	.35057	•35635				100.	•35635	.36169	.36664	.37126	•37559	•37965	.38347	.38708	.39049	.39374	.39682	.39976	.40257	•40525	.40782	.41028	.41265
$(\sqrt{4\alpha t})$	× ;	0.006	1000.0	1100.0	1200.0	1300.0	1400.0	1500.0	1600.0					1600.0	1700.0	1800.0	1900.0	2000.0	2100.0	2200.0	2300.0	2400.0	2500.0	2600.0	2700.0	2800.0	2900.0	3000.0	3100.0	3200.0
							·00	149	,			(6	/9)					σ	0 9	8 2	4					2	ZL,		



		70	dk	le	;	3	5 .					ر <u>ر</u>	! /	ĵų`)			
	1000	.21542	.21645	.21747	.21848	.21947	.22045	.22141	.22237	.22331	.22423	.22515	•22605	. 22695	.22783	.22870	.22956	.23041
	•006	.22728	.22831	.22931	. 23030	.23128	.23225	.23320	.23414	.23507	•23599	•23689	.23778	•23866	.23953	•24039	.24124	•24208
	800.	.24058	.24158	.24257	.24355	.24451	•24546	.24640	.24733	-24824	.24914	.25003	.25091	.25178	•25264	.25348	.25432	.25514
	700.	.25567	•25666	.25763	• 25859	•25954	.26047	.26139	.26230	.26320	.26408	•26496	.26582	.26667	.26751	.26834	•26916	.26997
. (~		.27312	•27408	.27504	.27598	.27690	.27781	.27872	.27960	•28048	.28135	•28250	.28304	•28388	.28470	.28551	.28631	•28710
7)		.29378	.29472	.29565	•29626	•29746	.29835	•29923	•30009	•30092	.30179	•30262	•30344	• 30425	•30505	.30584	.30662	•30739
	400	.31910	.32001	.32090	.32178	.32265	.32351	.32435	•32519	.32601	.32682	.32763	.32842	.32920	.32997	.33073	.33148	•33223
	300.	.35176	.35263	.35348	.35432	.35515	.35596	.35677	.35756	,35835	.35912	•35989	.36064	.36138	.36212	.36285	•36356	.36427
	200	.39782	.39863	.39942	.40020	26004.	.40173	.40248	.40322	*4036	•40466	.40537	.40608	.40677	.40745	.40812	.40879	•40945
	100.	.47659	.47729	.47798	•47866	.47933	66624.	.48064	.48128	.48191	.48254	.48316	.48377	.48437	96484.	.48555	.48613	.48670
(<u>\4\c)</u>	5	8400.0	8500.0	8600.0	8700.0	8800.0	8900•0	0.0006	9100.0	9200.0	9300.0	0.0046	9500.0	0.0096	9700.0	9800.0	0.0066	100001
									•	00	9	GZ	;				(6	/4)





	T	dk	c	;	3.				((y,	/y)																				
	10000.	00000	00000	00000	00000	00000•	00000	00000	00000					10000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000•
	•0000	00000	00000	00000	00000	00000	00000	00000	00000					0006	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000
	8000	00000	00000	00000	00000	00000	00000	00000	00000					8000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	•00003	.0000	• 00005
	7000.	00000	00000	00000	00000	00000	00000	00000	00000					7000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	.00001	.00001	• 00005	•00003	• 00005	.00008	.00011
•		00000	00000	00000	00000	00000	00000	00000	00000					•0009	00000	00000	00000	00000	00000	00000	•00001	•00005	• 00005	+0000•	• 00000	.00010	•00014	•00051	• 0005	•00039	.00051
<u>r[a</u>		00000	00000	00000	00000	00000	00000	00000	00000					5000	00000	00000	00000	.00001	• 00005	*0000	*0000	•00013	•00019	•00059	• 00042	•00058	.00078	.00102	•00129	.00161	.00198
	4000	00000	00000	00000	00000	00000	00000	.00001	-00005					*000*	.00003	\$0000	.00010	•00019	.00031	.00047	.00070	66000	.00134	•00176	• 00225	.00281	.00343	.00413	•00488	• 00269	•00656
	3000	00000	00000	.00001	.00003	20000	•00016	• 00032	.00057					3000.	.00057	.00093	•00140	•00200	.00273	•00359	•00456	• 00565	•00685	.00814	•00951	•01096	.01247	.01404	.01566	.01732	.01901
	2000.	.00012	•00035	6.000	.00150	.00251	.00343	• 00543	•00729					2000.	.00729	•00938	•01165	.01407	.01662	.01926	•02196	.02471	.02749	•03028	.03308	.03586	.03842	.04136	.04407	.04673	.04937
	1000.	.01305	95610	.02447	.03056	.03670	•04279	.04875	.05456					1000.	.05456	.06018	.06561	•07085	.07589	.08073	.08539	.08987	.09418	.09833	.10232	.10617	.10988	.11345	.11691	.12024	.12347
$\left(\frac{\sqrt{4\infty t}}{a}\right)$		0.006	1000.0	1100.0	1200.0			_	1600.0						1600.0	1700.0	1800.0	1900.0	2000-0		2200.0				2600.0	2700.0	2800.0	2900•0	3000.0	3100.0	3200.0
						.0	01	79					(10)	/q))					•	00	9	7	I					24	; <u>.</u>	

26.

		K	OK	l	•)	•				(y,	/ L	ł)				
	10000	.00713	•00748	•00784	•00850	•00857	•0086	•00932	.00971	.01010	•0104	.01089	•01129	•01110	.01211	.01253	•01295	.01337
	•0006	•01076	.01121	.01167	.01213	.01260	.01307	.01354	.01402	.01450	•01499	•01548	•01597	.01647	.01696	.01746	.01797	.01847
	8000	•01595	•01652	.01708	•01765	.01823	.01880	.01938	•01996	•02024	.02113	.02171	• 02230	.02289	.02348	.02407	• 02466	•02525
	.0007	.02326	.02395	.02463	.02532	.02600	• 02669	.02737	•02806	.02874	.02943	.03011	.03080	.03148	.03216	.03284	.03352	.03419
	_	.03345	.03426	•03206	.03586	•03666	.03745	.03824	.03903	.03982	.04060	.04138	•04215	• 04292	.04369	• 04445	.04521	.04597
		•04759	.04851	-04942	.05033	.05123	•05212	.05301	.05389	• 05477	.05563	.05650	.05735	.05821	• 05905	•05989	• 06072	•06155
	4000	.06734	• 06835	.06936	.07035	.07133	.07231	.07327	.07423	.07517	.07611	.0770	96770.	.07888	.07978	.08068	.08157	.08245
	3000.	.09557	*9960	.09771	.09H76	61660.	.10082	.10184	.10284	.10383	.10481	.10579	.10675	.10770	.10864	.10957	.11049	.11140
	2000	.13847	13956	.14064	.14170	.14275	.14379	.14482	.14583	.14683	.14781	.14879	.14975	.15070	.15164	15257	.15349	.15440
	1000	.21542	.21645	.21747	.21848	-21947	. 22045	.22141	.22237	.22331	. 22423	.22515	• 22605	•25695	.22783	.22870	. 22956	.23041
$\left(\frac{\sqrt{4\alpha t}}{d}\right)$	S	8400.0	8500.0	8600.0	8700.0	8800.0	8900.0	0.0006					٠	0.0096	9700.0			0.00001
									Ç	סע	フ!	32				(9/	9)

Table 4

Values of:

$$G\left(\frac{\sqrt{4\alpha t}}{a}\right) = \sum_{n=1}^{n=\infty} A_n(\beta_n a) U_0'(\beta_n a) e^{-\left(\frac{\beta_n a}{4}\right) \left(\frac{4\alpha t}{a^2}\right)}$$

An independent check computation can be made by use of values given in Bulletin 3, Part VII of the Boulder Canyon Project Final Reports. These apply for (b/a) = 100. A computation for the parameter $(\sqrt{4\alpha t}/a)$ = 25 is shown below:

		$-(\frac{\beta_n^a}{4}) (\frac{4\alpha t}{a^2})$	
n	$A_n(\beta_n a) \ \bigcup_{0} (\beta_n a)$	e 4 (a ²)	Term
1	0.254236	0.992006	0.252204
2	0.058612	0.750094	0.043965
3	0.044047	0.410733	0.018092
4	0.037729	0.163466	0.006167
5	0.034078	0.047347	0.001613
6	0.03166*	0.009988	0.000316
7	0.02993*	0.001535	0.000046
8	0.02862*	0.000172	0.000005
		Total	.322408

The tabular value is 0.32241. The agreement is satisfactory. *Supplied by this writer.

<u>*</u>	$\frac{\sqrt{40ct}}{0}$	0	_	4	43	4	ß.	હ		∞	6
	m. «	.77456	.76063	.74750	.73511	.72339	.71228	.70173	.69171 .61306	.68217	.67308
	• n.	.59504	.58945	- 0	788	738	689	641	5596	5551	5508
<i>'</i> 57	.	.54668					1				•
	۱ ۍ	.5466R	.54262	.53868	.53484	.53111	.52748	5239	20	1115 4977	51.58 48.61
	• 5	. 1.068 67697	75/05.		75105.	.49866 .7362	449249	44450	6.4	7194	
	• • • •	45999	645796	45597	45402	45210	.45021	44836	44654	. 4	66244
ď		.44126	43956	œ	.43624	-7	.43303	4314	2	.42840	The second
og.	11.	. 42543	46534	4225	•45114	•41976	.41839	170	2	144	-
	٠, ٥	.41184	41058	4003	40812	10691	240572	40	.40339	- AI	_
		00000	39889	39781	.39673	.39567	.39462	.39359	.39256	.39155	•39055
	14.	. 38954	.38459	v	.38667	.38573	.38479	38	•38296	Λı	
•=	16.	.38028	.37941	rc	.37769	.37685	.37601	.37518	.37436	~ I	_
'	٦۴.	.37196	.37117	.37039	• 36962	.36886	.36810	.36735	.36661	וחו	
	17.	. 36443	.36372	.36301	.36231	.36162	.36093	.36025	.35957	on I	
	18.	. 1575A	.35693	.35629	• 35565	.35501	.35438	.35376	.35314	nu,	·
	10.	.35132	.35072	.35013	•34954	.34896	.34838	.34781	.34724	vo.	
001	20.	.34556	.34501	.34446	.34392	.34338	.34285	.34232	.34179	_	_
	2.	44024	.33973	.33922	.33872	.33822	.33773	.33724	.33675	ഹ	•
,.	22.	.33531	.33483	3343	.33389	.33343	.33297	.33251	•33206	_	_
•••	23.	.33071	.33027	.32983	.32940	.32896	.32853	.32811	.32768	_	~
•	24.	.32643	.32601	.32560	•32519	.32479	.32438	.32398	•32329	•	~
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	7 K.	.32240	.32202	.32163	212	.32087	.32049	3201	•31974	.31937	3190
(10/	۶۴.	.31863	.31827	.31790	175	.31718	3168	3164	.31612	31577	3154
	٠,٧	.31507	.31473	.31439	140	.31371	3133	2 1	.31270	.31636	2000
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٠	41.	.28017	57999	2798	.27963	.27945	2792	2	.27891	.27873	ຕ
7	42.	. 2783B	.27821	2780	•27786	.27768	.27751	2	•27716	.27699	Œ.
7	43.	. 27665	.27648	2763	.27615	•27598	.27581	.27564	.27548	.27531	5
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	45.	.27337	.27321	2730	729	.27274	.27258	7	.27227	.27212	<u> </u>
i.	44.	.27181	.27166	.27150	713	.27120	.27105	چ	.27075	.27060	7
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7	4 A •	.26883	.26869	.26855	684	•26826	.26812	۰	.26784	.26770	£ :
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0	7	-	01	56379	52	26128	26008	25890	25776	55	57	. 21	8	25247	•		•	•	•	•	-	. •	·	•	-				•	23765	23697	23565		•	•	23313		75152	23078	022	996	116	957	904	. 257	002	550
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\$\begin{array}{c} \begin{array}{c} \begin{array}{c} \begin{array}{c} \begin{array}{c} \begin{array}{c} \cdot	B	•	•56	.26	•26	•56	•26	.2591	.2579	•2568	•252	•254	.253	•252	.251	• 250	.249	847.	147.	746	245	.244	.243	•2459	.2421	147.	239	.239	.238	.237	.2371	7257	.2351	•5344	.2338	.2332	0656	2314	2.	.23	.22	.22	.228	•228	.22	.22.	22.
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Table 4.	$G\left(\frac{\sqrt{4\alpha t}}{a}\right)$
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	120.	.21715	.21677	.21639	-21402	.21566	.21529	.21493	.21458	.21423	.21388
	130.	. 21354	.21320	. 21286	.21253	.21220	.21187	.21155	.21123	.21092	.21060
	140.	.21029	66602*	.2096A	.20938	.20908	.20879	.20850	.20821	20192	.20764
.0(]50.	.20736	.20708	.20680	.20653	•20626	.20599	.20572	•20546	.20520	•50464
	140.	.2046A	.20443	.20418	.20393	.20368	.20343	.20319	.20294	.20270	.20247
	170.	. 20233	00202.	.20176	.20153	.20131	.20108	.20085	.20063	.20041	.20019
	180.	16661.	.19975	19954	.19933	.19912	.19891	.19870	.19849	.19828	.19808
	lon.	.19788	.19768	. 1974A	.19728	.19708	•196A9	.19669	.19650	.19631	.19612
	500-	.19593									
	200	.19593	19574	.19556	.19537	.19519	.19501	.19483	• 19465	.19447	.19429
•	211.	11461.	19394	.19376	.19359	.19342	.19325	.19308	.19291	.19274	.19258
	220.	19261	.19225	.1920B	.19192	.19176	.19160	.19144	.19128	.19112	.19097
ĺ	230.	.19041	.19066	.19050	.19035	.19020	.19005	.18990	.18975	.18960	.18945
	740.	.18930	.18916	19901	.18887	.18873	.18858	.18844	.18830	.18816	.18802
	250.	.18788	.18774	.18741	.18747	.18733	.18720	.18706	.18693	.18680	.18667
	260.	.18653	.18640	.18627	.18614	.18601	.18588	.18576	.18563	.18550	.18538
	270.	.18525	.18513	.18501	.18488	.18476	.18464	.18452	.18440	.18428	.18416
	2A0.	.18404	.18392	.18380	.18368	.18357	.18345	.18333	.18322	.18310	•18299
	290.	.18248	.18276	.18265	.18254	.18243	.18232	.18221	.18210	.18199	.18188
	300.	18177	.18166	.18155	.18145	.18134	.18124	.18113	.18102	.18092	.18082
9)	310.	.18071	.18061	.18051	.18040	.18030	.18020	.18010	.18000	.17990	.17980
	320.	.17970	.17960	.17950	.17940	.17931	.17921	.17911	.17901	.17892	.17882
	330.	.17873	.17863	.17854	.17844	.17835	.17826	.17816	.17807	.17798	.17789
	340.	17780	.17770	.17761	.17752	.17743	.17734	.17725	.17716	.17708	.17699
	350.	.17690	.17681	.17672	.17664	.17655	.17646	.17638	.17629	17621	.17612
	340.	17604	.17595	.17587	.17578	.17570	.17562	.17553	.17545	.17537	.17529
09	370.	.17521	.17512	.17504	.17496	.17488	.17480	.17472	.17464	.17456	.17448
	380.	.17440	.17432	.17425	17417	.17409	.17401	.17394	.17386	.17378	.17371
	300	.17343	.17355	.17348	.17340	.17333	.17325	.17318	.17310	.17303	17296
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0	.17288	97121	17078	.17012	.16948	_	.16826	.16768	.16711	.16655	.16601	.16549	.16498	.16448	•16369	.16351	.16305	.16259	.16215	.16171	.16129	.16087	.16046	.16006	.15967	15929	.15891	.15855	15919	.15783	.15748	.15714	.15681	.15648	.15615	.15583	.15552	.15521	.15491	.1546]
_	.17281	607/1.	17071	17006	.16942	.14880	.16820	.16762	.16705	.16650	16596	.16544	.16493	.16443	16394	.16346	.16300	.16255	.16210	.16167	.16125	.16083	.16042	.16002	.15963	.15925	.15484	.15851	.15815	.15780	.15745	.15711	.15677	15644	.15612	.15580	.15549	.15518	.15488	
7	.17273	21271	17065	.16999	16936	.16874	.16814	.16756	.16700	.16644	.16591	.16538	.16487	.16438	.16389	.16342	.16295	.16250	.16206	.16163	.16120	.16079	.16038	15999	.15960	.15921	.15884	.15847	.15811	.15776	.15741	.15707	.15674	.15641	.15609	.15577	,15546	.15515	.15485	
£	.17266	56171	: 2	.16993	.16930	Œ	α 0	~	.16694	.16639	•16585	m	Œ	3	.16384	.16337	.16291	.16246	.16202	.16158	.16116	.16075	.16034	.15995	15956	.15918	.15880	.15844	.15808	.15773	.15738	.15704	.15671	.15638	•15606	r	•	_	Œ	
4		7117	705	Œ	92	.16862	80	74	99	Φ	ທ	.16528	64	.16428	63	63	.16286	62	.16197	.16154	.16112	.16071	.16030	16651.	.15952	.15914	ហ	S	.15804	ຫ	U7	ຫ	S	រប	.15602	.15571	u	•15509	.15479	
Ø	17252	: ב ב	1704	16	691	.16856	.16797	.16739	.16683	.16628	.16575	.16523	.16472	.16423	Φ	œ	.16282	.16237	Φ	.16150	.16108	.16067	.16026	.15987		15910	_	\sim	580	576	.15731	•		~	•	.15568	~		.15476	
9		17.1	703	97	9	.16850	.16791	.16733	.16677	.16623	.16570	.16518	.16467	.16418	9	.16323	.16277	.16232	.16189	.16146	.16104	.16063	.16022	.15983	.15944	15906	.15869	.15833	.15797	.15762	.15728	v	99	•	S	S	S	ហ	4	
7	17237	170	20.2	69	9	684	78	672	9	Φ	Φ	.16513	Φ	Ψ	.16365	œ	.16273	ď	Φ	.16141		.16059				.15903					•15724				29	56	,15531	S	4	
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ଚ	17223	7.1.1 17.0	` ~	95	89	683	677	671	99	999	.16554	650	645	49	.16356	63	.16264	.16219	9	.16133	.16091	.16050	.16010	.15971	.15933	.15895	.15858	.15822	.15787	.15752	.15718	.15684	.15651	.15618	.15587	.15555	.15524	.15494	.15464	

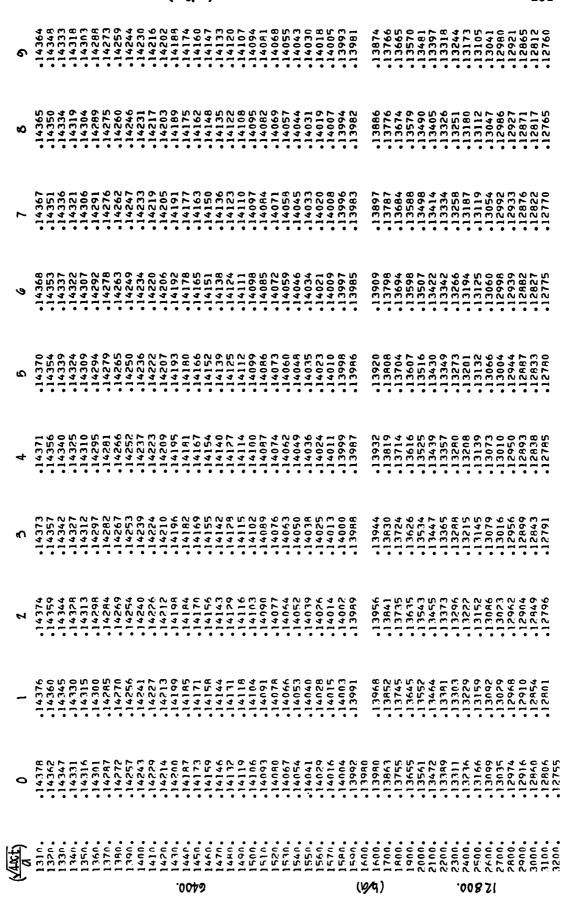
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6	.15435	2	נית	າທ	15	15	S	LO I	រូ ខេ	un (<u>., .</u>	בי ה	າທ	15	2	7	.14976	J	4	J	•	J	4	4	4	4	.14766	J	4	4	<u> </u>	₹,	<u>,</u>	Ξ;	σ,	<u> </u>		₹ ;	7 -			7	4	. 2	1,7	7	4	.14379
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6	.12710	.12662	.12616	.12572	•12529	.12487	.12447	.12408	.12371	.12334	.12299	12264	.12230	.12198	.12166	12135	.12105	.12076	.12047	.12019	.11992	•11965	.11939	.11913	.11889	.11864	.11840	.11817	.11794	.11772	.11750	.11728	11707	.11686
80	.12715	.12667	.12621	.12576	12533	.12491	.12451	.12412	.12374	.12338	12302	.12267	.12234	.12201	•12169	.12138	.12108	.12079	.12050	.12022	.11994	.11968	.11942	11916	.11891	.11867	.11843	.11819	.11796	.11774	.11752	.11730	.11709	.11688
7	.12720	.12672	12625	.12580	.12537	•12496	.12455	12416	.12378	.12341	•12306	.12271	.12237	.12204	.12172	.12141	.12111	.12081	.12053	12025	.11997	.11970	.11944	.11919	.11893	.11869	.11845	.11822	.11799	.11776	11754	.11732	.11711	.11690
9	.12725	.12676	.12630	.12585	.12542	.12500	.12459	.12420	.12382	.12345	.12309	.12274	.12240	.12208	•12176	.12144	.12114	.12084	•12056	.12027	.12000	.11973	.11947	11921	.11896	.11871	.11847	11824	.11801	.11778	11756	.11734	.11713	.11692
ĸ	.12730	.12681	.12634	.12589	.12546	.12504	.12463	12424	.12386	.12349	.12313	.12278	.12244	.12211	.12179	.12147	.12117	.12087	.12058	.12030	.12003	.11976	.11949	.11924	.11898	.11874	.11850	.11826	.11803	.11780	.11758	.11737	.11715	.11694
4	.12735	.12686	.12639	12594	.12550	.12508	.12467	.12428	.12389	.12352	.12316	.12281	.12247	.12214	.12182	.12151	.12120	.12090	.12061	.12033	.12005	.11978	.11952	11926	.11901	.11876	.11852	.11829	.11805	.11783	.11760	.11739	.11711	.11696
€	.12740	.12691	.12644	.12598	.12554	.12512	.12471	.12431	.12393	.12356	.12320	.12285	.12250	.12217	.12185	.12154	12123	.12093	.12064	.12036	.12008	.11981	.11955	.11929	.11903	.11879	.11855	.11831	.11808	.11785	.11763	.11741	.11719	.11698
7	.12745	.12696	.12648	.12603	•12559	12516	.12475	.12435	.12397	.12359	.12323	.12288	12254	12221	.12188	.12157	.12126	.12096	.12047	.12039	.12011	.11984	11957	.11931	.11906	.11881	.11857	.11833	.11810	.11787	.11765	.11743	.11722	.11700
_	.12750	.12700	.12653	.12607	.12563	.12520	.12479	.12439	.12401	.12363	.12327	15291	.12257	12224	16121•	.12160	12129	.12099	.12070	.12041	.12014	.11986	.11960	.11934	.11908	.11884	.11859	.11836	.11812	.11789	.11767	.11745	.11724	.11703
0	12755	.12705	.12658	.12612	12567	.12525	.12483	.12443	.12404	.12367	.12330	.12295	19221.	12221	.12195	.12163	.12132	.12102	.12073	.12044	.12016	.11989	.11962	.11936	.11911	.11886	.11862	.11838	.11815	.11792	.11769	.11747	11726	.11705
$\left(\frac{\sqrt{4\alpha t}}{\alpha}\right)$	3200.	3300.	3400.	3500.	3600.	3700.	3800.	3900	4000	4100.	4200	4300.			• 4600			*0067	2000	5100.	5200.	5300	2400		2600.		5800.	.0065	. 0009	6100.		.0029	6400.	.0059

$\left(\frac{\sqrt{4\infty t}}{a}\right)$	0	_	7	6	4	מ	૭	7	ಹ	6
.0099	.11684	.11682	.11680	.1167R	.11676	.11674	.11672	.11670	.11668	.11666
.0029	.11464	.11662	11659	.11657	.11655	.11653	.11651	.11650	.11648	.11646
. A00.	11644	.11642	.11640	.11638	.11636	.11634	.11632	.11630	.11628	.11626
.0064	.11624	.11622	.11620	.11618	.11616	.11614	.11612	.11610	.11608	.11607
7000	.11605	.11603	.11601	11599	11597	11595	.11593	11591	.11589	.11588
7100.	.11586	.11584	.11582	.11580	.11578	•11576	.11574	.11573	11571	.11569
7200.	.11567	.11565	.11563	.11562	.11560	.11558	11556	.11554	11552	.11551
7300.	.11549	11547	.11545	.11543	.11542	.11540	.11538	11536	11534	11533
7400	.11531	.11529	11527	.11525	11524	.11522	.11520	•11518	11511	.11515
7500.	.11513	.11511	.11510	.11508	11506	11504	.11503	11501	.11499	.11497
	.11496	11494	.11492	.11490	.11489	.11487	.11485	.11484	.11482	.11480
	.11479	.11477	.11475	.11473	.11472	.11470	.11468	.11467	.11465	.11463
7A00.	.11462	.11460	.11458	.11457	.11455	.11453	.11452	.11450	.11448	.11447
	.11445	.11443	.11442	.11440	.11438	.11437	.11435	.11434	.11432	.11430
	.11429	.11427	.11425	.11424	.11422	.11421	.11419	.11417	.11416	.11414
R100.	.11413	11411	.11409	.11408	.11406	.11405	.11403	.11401	.11400	.11398
8200.	.11397	.11395	.11394	.11392	.11390	.11389	.11387	.11386	11384	.11383
8300.	.11341	.11380	.11378	.11376	.11375	.11373	.11372	.11370	11369	.11367
R400.	.11366	11364	.11363	11361	.11360	.11358	.11357	.11355	.11354	.11352
B500.	.11351	.11349	.11348	.11346	.11345	.11343	.11342	.11340	•11339	11337
8600.	.11336	11334	.11333	.11331	.11330	11328	.11327	.11325	.11324	.11322
8700.	.11321	.11319	.11318	.11316	.11315	.11314	.11312	11311	•11309	.11308
.0088	.11306	.11305	.11303	.11302	.11301	.11299	.11298	11296	•11295	.11293
8400	.11292	11291	.11249	.11288	.11286	11285	.11283	11282	.11281	.11279
.0006	.11278	11276	.11275	11274	.11272	11271	.11269	.11268	11267	.11265
	.11264	.11263	.11261	.11260	.11258	.11257	•11256	11254	.11253	11252
0026 A	.11250	.11249	.11247	.11246	.11245	.11243	.11242	.11241	•11239	.11238
	.11237	.11235	.11234	.11233	.11231	.11230	.11228	.11227	•11226	.11224
.0076	.11223	.11222	.11220	.11219	.11218	11211.	.11215	11214	.11213	.11211
9500	11210	.11209	.11207	.11206	.11205	.11203	.11202	.11201	.11199	.11198
9600.	.11197	96111.	.11194	.11193	.11192	.11190	.11189	.11188	.11187	.11185
.0076	.111A4	.11143	.11181	.11180	.11179	.11178	.11176	.11175	.11174	.11172
.0086	111111	.11170	.11169	.11167	.11166	.11165	.11164	.11162	.11161	.11160
0066	.11159	111157	.11156	111155	.11154	.11152	.11151	.11150	.11149	.111147
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Table 5

Values of:

$$H \left(\frac{\sqrt{4\alpha t}}{a}\right) = \left(\frac{a^2}{4\alpha t}\right) \left[\left(\frac{b^2 - a^2}{2a^2}\right) - \sum_{n=1}^{n=\infty} \frac{A_n \ U_0'(\beta_n a) \ e}{(\beta_n a)} \right]$$

The whole volume produced by the artesian well out of the time t is:

$$P = 8\pi K D y_0 t H(\frac{\sqrt{4\alpha t}}{a})$$

A spot check computation for the point $(\sqrt{4\alpha t}/a) = 25$ can be made by using independently computed values for the (b/a) = 100 case.

$$(\frac{b^2-a^2}{2a^2}) = 4999.500$$
 $(\frac{4\alpha t}{a^2}) = 625$

	$\frac{A_n U_0'(\beta_n a)}{n}$	$-\left(\frac{\frac{\beta_n a}{4}}{4}\right) \left(\frac{4\alpha t}{a^2}\right)$	
n	(β _n a)	e 4 / (a ² /	Term of Series
1	4949.625	0.992006	4910.058
2	31.847	0.750097	23.888
3	7.734	0.410731	3.177
4	3.254	0.163467	0.532
5	1.745	0.047347	0.082
6	1.074	0.009988	0.011
7	0.722	0.001535	0.001
Total	4996.001	Total	4937.749

$$4999.500 - 4937.749 = 61.751 \frac{61.751}{625} = 0.098802$$

Tabular value is .098799. The correspondence is close enough to show that the program was operating properly.

A further confirmation of the validity of this table is obtained from a comparison of tabular values with those obtained from an independently derived approximate formula. A comparison for $(\sqrt{4\alpha t}/a) = 100$ is made in the text.

The comparable H values are

From the table	0.040671

From the approximation 0.04061

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100 100 100 100 100 100 100 100 100 100		.188823 .173675 .161995 .152674 .145034 .138638	.187106 .172374 .160971 .151843 .144344 .138055	.185440 .171105 .159969 .151028 .143667 .137480	838 698 589 502 430 369	.182249 .168662 .158029 .149445 .142345 .136358	72 48 08 67 70 81	.179234 .166335 .156170 .147920 .141069 .135270	.177788 .165212 .155269 .147179 .140446 .134738	.176380 .164115 .154386 .146451 .139833 .134215
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	.0987389 .097389 .097389 .094824 .093652 .092544 .091495 .090500 .089554	.098653 .097253 .097264 .094704 .093538 .092436 .091393 .090403	.098509 .097118 .094584 .093425 .093292 .091292 .091292 .091307	.098366 .096984 .095686 .093313 .09223 .091191 .090211	.098224 .096851 .095561 .093201 .092117 .091091 .09116 .089189	.098082 .096718 .095436 .093090 .093090 .092012 .090021 .089099	.097942 .096586 .095312 .094112 .091908 .091908 .0990892 .089009	.097802 .096455 .095189 .093996 .093996 .091804 .091804 .089833 .089833	.097664 .096325 .095067 .093881 .092760 .091760 .090695 .089740	.097526 .096196 .094945 .093766 .092652 .091597 .090597
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61	.078284	.077369	.076932	.076507	0.00000	.075304	.074924	.074555	•074195	•073844	•073502	901570	.072525	.072214	116120	+191/0+	*011364	140110°	264020	.070226	• 069965	.069710	097690	.069215	068739	068508	.068282	•068059	•067840	• 06/626	.067208	.067004	• 066804	• 066608	•066414	• 066224	.066037	.065853	2/9090*	• 063494	010000	064975	064808)))	
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 | 04442 | 046641 | 0.4040 | 603040 | • 040000 | 014040 | .045/43 | •045580 | .045421 | .045267 | .045116
 |
| | .062726 | .061440 | •060299 | 112650. | 057515 | 747470 | 056041 | .055387 | | .054781

 | .054216 | .053687 | .053191

 | •052724 | .052284 | .051867 | .051472 | .051097 | .050740 | •050400
 | •050074

 | •049764 | 994640• | .049180 | •048906

 | • 048642 | .048388
 | .048143 | | | 189/50 | .04/460 | 77770 | 140410
 | 146647 | 046450 | 0,4040 | 1 7040 | 660000 | 126040. | •045759 | .045596 | .045437 | •045282 | .045131
 |
| .064351 | .062864 | .061561 | .060407 | • 0593/4 | 057505 | 056821 | 056109 | .055451 | | .054839

 | .054270 | .053738 | .053239

 | •052769 | • 052326 | •05190B | .051511 | .051134 | •050775 | .050433
 | •050106

 | *049794 | • 049495 | .049208 | .048933

 | •048668 | .048413
 | .048167 | .047931 | | .047703 | 284/40. | 007/100 | 190750
 | 04666 | 046478 | 906990 | 64040 | 11040. | ***** | .045776 | .045612 | .045453 | .045297 | .045146
 |
| .064512 | .063003 | .061684 | .060516 | .059473 | .05751 | 20000 | 056177 | 05551 | | •054899

 | .054326 | .053790 | .053288

 | .052815 | .052370 | .051948 | .051549 | .051170 | .050810 | • 050466
 | .050138

 | .049825 | •049524 | .049236 | .048960

 | •048694 | .048438
 | .048192 | •047954 | , , , | 04/140 | *04/20¢ | *04/404 | 2807400
 | 046686 | 900000 | 0,4270 | 010010 | +010+0* | 104240 | 26/540 | •045628 | .045468 | .045313 | .045161
 |
| • 064675 | .063145 | .061809 | .060627 | 2/5650. | .057759 | 056970 | .056247 | .055578 | .054958 | .054958

 | .054381 | .053842 | .053336

 | .052861 | .052413 | .051990 | .051588 | .051207 | .050845 | •02020
 | .050170

 | •049855 | •049554 | •049265 | .048987

 | .048720 | .048463
 | .048216 | .047977 | 14/140 | 04//40 | 016750 | 01010 | 201/100
 | 004040 | 046515 | 155470 | 100000 | 2010+0 | 00000 | *04580 * | .045644 | .045484 | .045328 | .045176
 |
| 100. | 110. | 120. | ٠, | ٦. | - | 120 | 180 | 190. | 200. | 200-

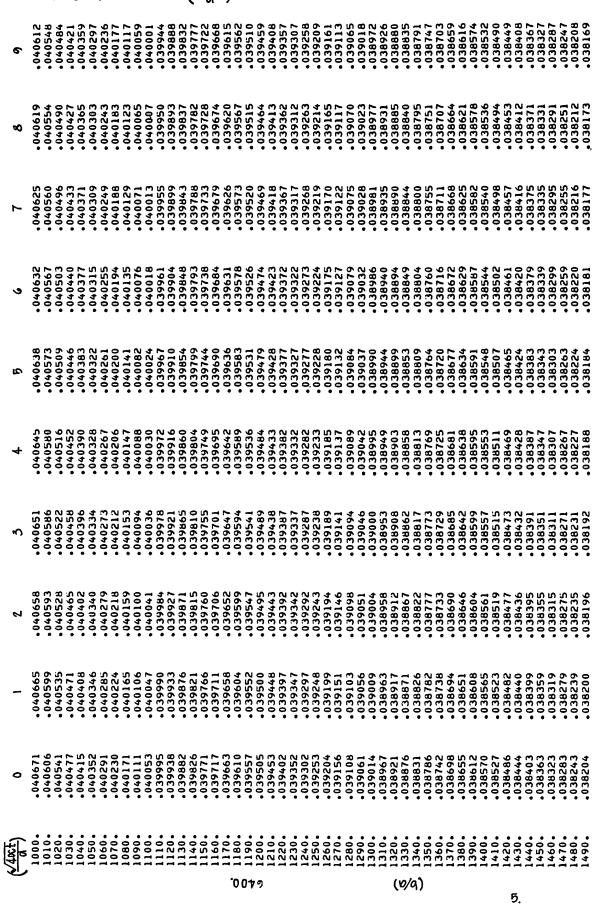
 | 210. | 220. | 230.

 | 240. | 250. | 560. | 270. | 280. | 290• |
 |

 | | 330• | 340• | 350.

 | 360. | 370.
 | 380. | 390 | • 004 | • 00+ | •10° | • • • • | 420
 | 450 | 460. | | | | | 200 | 510. | 520. | 530. | 240•
 |
| | 064675 .064512 .064351 .064193 .064037 .063883 .063731 .063582 .063434 . | .064675 .064512 .064351 .064193 .064037 .063883 .063731 .063582 .063434 .063145 .063003 .062864 .062726 .062590 .062455 .062323 .062192 .062063 | 064675 .064512 .064351 .064193 .064037 .063883 .063731 .063582 .063434
063145 .063003 .062864 .062726 .062590 .062455 .062323 .062192 .062063
061809 .061684 .061561 .061440 .061319 .061201 .061083 .060967 .060853 | 100064675 .064512 .064193 .064037 .063883 .063731 .063582 .063434 .064037 .063883 .063731 .063582 .063434 .063145 .063003 .062864 .062726 .062590 .062845 .062323 .062192 .062063 .061809 .061809 .061864 .061340 .061319 .061201 .061083 .060967 .060853 .060851 . | 100 | .064675 .064512 .064193 .064037 .063883 .063731 .063843 .063883 .063823 .062192 .062063 .06193 .063145 .063003 .062864 .062726 .062590 .062455 .062323 .062192 .062063 .06193 .061809 .06184 .061561 .061440 .061319 .061201 .061083 .060967 .060853 .06073 .060627 .060191 .060085 .059980 .059987 .059977 .05967 .059627 .059180 .059085 .058990 .058897 .058804 .05818 .05818 .05818 .05818 .05818 .05818 .05818 .05818 .05818 .05818 .05818 .05818 .05818 | 100. .064575 .064512 .064193 .064037 .063883 .063731 .063582 .062163 110. .064675 .062864 .064193 .064037 .062323 .062192 .062063 110. .063145 .062864 .062726 .062590 .062323 .062192 .062063 130. .061884 .061561 .061440 .061319 .061083 .061083 .060967 .060965 130. .060627 .060407 .060299 .060191 .060085 .059980 .059877 .059877 .0598897 .0598897 .0598804 140. .059627 .059374 .059877 .059899 .058899 .058899 .058804 .058804 150. .0587759 .058777 .058899 .058899 .058899 .057925 160. .057759 .057677 .057677 .057677 .057277 .057277 | 100. .064575 .064512 .064193 .064037 .063883 .063731 .063582 .062192 .062063 110. .063145 .062864 .064726 .062726 .062590 .062323 .062192 .062063 130. .061804 .061561 .061440 .061319 .061083 .060967 .060967 .060853 130. .060627 .060407 .060299 .060191 .061085 .059980 .059877 .059774 140. .059572 .059374 .059277 .059180 .059085 .058990 .058897 .058804 150. .058621 .058473 .059277 .059180 .058899 .058899 .058804 .058804 150. .058621 .058842 .058245 .058266 .058899 .058899 .058809 .05727 .057200 .05727 160. .0569779 .056895 .056974 .056041 .056041 .056097 .056999 .056109 .056041 .056990 .057277 .055778 .055718 | 100. .064575 .064351 .064193 .064037 .063883 .063731 .063582 .0623434 110. .0644675 .062864 .064726 .062726 .062733 .062192 .062192 .062063 110. .063145 .064364 .064440 .062726 .062590 .062323 .062192 .062063 120. .061884 .061561 .061440 .061319 .061085 .05980 .059877 .059774 130. .060627 .060407 .060299 .060191 .061085 .059980 .059877 .059804 140. .059572 .059473 .059277 .059180 .059899 .058899 .058899 .058809 150. .058621 .058642 .059277 .059180 .058899 .058809 .058809 .058809 .058809 .058809 .058809 .057721 .057220 .057723 .057220 .056978 .056978 .056978 .056458 .056458 .056458 .056458 .056 | 100. .064512 .064193 .064037 .063883 .063731 .063582 .062063 110. .064675 .062864 .064193 .064037 .062323 .062192 .062063 110. .063145 .062864 .062726 .062590 .0621201 .062192 .062063 120. .061884 .061561 .061440 .061319 .061083 .060967 .06085 130. .060627 .060407 .060299 .060191 .061085 .059980 .059877 .059877 .059880 140. .059572 .059374 .059277 .059180 .059899 .058899 .058897 .059880 150. .058621 .058742 .059276 .0598180 .058999 .058999 .058899 .05720 .058899 150. .058621 .0586821 .058747 .056401 .056401 .056401 .056401 .056401 .056401 .056459 .056459 .0564589 .0564589 .0564589 .0564589 </td <td>100. .064575 .064512 .064193 .064037 .063883 .063731 .062323 .062323 .062343 .062465 .062063 .062192 .062063 .062192 .062063 .062192 .062063 .062192 .062063 .062192 .062063 .062192 .062063 .062192 .062063 .062192 .062063 .062192 .062063 .062192 .</td> <td>100. .064575 .064351 .064193 .064037 .063883 .063731 .062323 .062323 .062343 .062465 .062343 .062323 .062192 .062063 110. .063145 .063003 .062864 .064726 .06239 .062323 .062192 .062063 120. .063145
 .061804 .061684 .061561 .060191 .0601083 .062323 .062192 .060963 120. .061809 .061684 .061644 .061440 .061919 .061008 .065997 .060967 130. .060627 .060477 .060299 .060191 .060909 .05889 .058897 .059889 150. .058621 .059473 .059826 .058180 .058899 .057877 .057925 160. .056970 .056895 .056747 .056674 .056674 .056674 .055939 .056458 .055458 .055458 .055458 .055466 .055839 .055458 .055466 .055839 .05546</td> <td>0.0 0.0<td>100. .064675 .064512 .064193 .064037 .063883 .063731 .063362 .062063 1100. .064675 .0646455 .062455 .062323 .062192 .062063 110. .063145 .062864 .062726 .062590 .062103 .062063 .062063 120. .061809 .06191 .061201 .061083 .060967 .060967 .060967 130. .060627 .060407 .060299 .06191 .060085 .059877 .059877 .059877 .059887 .059887 .059887 .059887 .059889 .059889 .059889 .059899 .055899 .055899 .055899</td><td>100. .064675 .064512 .064193 .064037 .063883 .063731 .063882 .063434 1100. .061845 .064193 .064037 .062455 .062323 .062192 .062043 1100. .061844 .061644 .061319 .061201 .061083 .062092 .062063 120. .061809 .061644 .061440 .061319 .061083 .060967 .060853 130. .060627 .060616 .061440 .06191 .061083 .061097 .060853 130. .060627 .060407 .060299 .06191 .061899 .055897 .059877 .059817 .059817 .059817 .059817 .059817 .059817 .059817 .059817 .059817 .051817 .056428 .055829 .056458 .055817 .055108 .055108 .056458 .056458 .055458 .056458 .055817 .055108 .055109 .056458 .055458 .055458 .055458 .055458 .055458</td><td>U0. .064675 .064512 .064193 .064037 .063883 .063323 .063382 .063343 1100. .061865 .062864 .062726 .06299 .061201 .061083 .062962 .062063 1100. .061809 .061801 .061809</td><td>100. .064675 .064812 .064193 .064037 .063883 .063731 .063382 .063343 1100. .06145 .062864 .062726 .062590 .062455 .062323 .062192 .062043 120. .061809 .061864 .061240 .061319 .061083 .062323 .062192 .062045 120. .061809 .061844 .061440 .061319 .061809 .060967 .060967 .060967 .060853 130. .060627 .060299 .06191 .061809 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059887 .059887 .059877 .059887 .059877 .059877 .059877 .059877 .059887 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .055773 .055773 .055773 .055773 .055773</td><td>U. .064675 .064512 .064493 .064037 .063883 .063731 .063582 .063434 1100 .064675 .064612 .064493 .064693 .064455 .062723 .06292 .062465 .062731 .063434 .062864 .062786 .06259 .062455 .06292 .062999 .06299 .06299 <</td><td>U.0. .064675 .064512 .064131 .063883 .063731 .063882 .063434 1100 .061465 .064512 .064435 .062864 .062726 .062695 .062323 .062192 .062094 1100 .061809 .061804 .061319 .061801 .061833 .062192 .062092 1130 .061809 .061804 .066191 .0601085 .065980 .065987 .060967 1130 .06087 .060191 .060088 .059980 .058987 .065974 140 .059572 .059180 .058989 .059887 .059871 150 .059621 .058180 .058990 .058897 .059874 150 .058621 .058474 .058926 .058990 .058987 .059874 150 .058621 .058474 .058474 .058644 .058094 .058094 .058094 .058094 .058094 .058094 .058094 .058094 .058094 .058094 .058094 .0</td><td>0.064675 .064512 .064454 .064493 .064037 .064351 .064354 .064363 .064363 .062455 .062323 .062362 .062363 .062455 .062323 .062452 .062455 .062323 .062452 .062465 .062465 .062455 .062323 .062192 .062463 .062463 .062455 .062323 .062192 .062465 .062465
 .062465 .062465 .062465 .062469 .0610191 .0610191 .0610193 .06</td><td>100 .064675 .064512 .064193 .064037 .063883 .063731 .065362 .062063 110 .063145 .062164 .064526 .062390 .062455 .062192 .062063 110 .063164 .061561 .061440 .061319 .062192 .062192 .062063 120 .0661807 .061584 .061561 .061440 .061319 .062192 .062192 .062063 130 .060517 .061584 .061581 .061083 .062192 .062192 .062063 130 .060527 .069517 .061899 .05847 .05840 .05848 .05846 .05846 .05846 .05846 .05846 .05846 .05846 .05846 .05846 .05846 .05846 .05847 .05848 .05846 .05847 .05848 .05846 .05846 .05846 .05846 .05848 .05846 .05848 .05848 .05848 .05848 .05848 .05848 .05848 .05848 .05848<td>100 .064675 .064512 .064493 .064037 .062455 .062323 .062192 .062063 1100 .063145 .062864 .062726 .062590 .062455 .062192 .062102 .062063 1100 .061345 .062864 .062726 .06299 .061083 .062192 .062063 1100 .061340 .061083 .062192 .060191 .062190 .062191 .062192 .062191 .062192 .062067 .062192 .062192 .062069 .060191 .062192 .062192 .062193 .062192 .062193 .062192 .062193 .0621</td><td>10.0 .064675 .06458 .06403 .06403 .06403 .06403 .06403 .06249 .062455 .06233 .062192 .062093 .062192 .062063 .062063 .062063 .062063 .062192 .062063 .062063 .062192 .062063 .062063 .062192 .062063 .062063 .062063 .062063 .062063 .062063 .062063 .062063 .062063 .062063 .062063 .062063 .062063 .0620647 .062063 .0620647 .062063 .0620647 .062063 .0620647 .062063 .0620647 .062063 .0620647 .062063 .0620647 .062064 .0</td><td>100 .064675 .064612 .064951 .064037 .063883 .063331 .063882 .063343 110 .063145 .064604 .062726 .062590 .062455 .062323 .062192 .062063 120 .061809 .062726 .066191 .061809 .062192 .062363 130 .060627 .060407 .066299 .066191 .061908 .058900 .062897 .062064 140 .059972 .066191 .060192 .061998 .058900 .058897 .062192 .066197 140 .059972 .066191 .060191 .060199 .061998 .058900 .058897 .0</td><td>0.00 0.064675 0.064512 0.064931 0.064675 0.064675 0.064513 0.064313 0.064331 0.063145 0.062864 0.062890 0.062893 0.062192 <th< td=""><td>100 .064675 .064612 .0649193 .064037 .062855 .062132 .062192 .062693 .062192 .062093 .062192 .062093 .062192 .062093 .062192 .062093 .062192 .062093 .062192 .062093 .062192 .062192 .062093 .062192 .062192 .062192 .062192 .062192 .062193 .</td><td> 10. 0.64675 0.64512 0.64493 0.64037 0.62883 0.63731 0.62382 0.62334 0.63145 0.63303 0.62864 0.62226 0.62295 0.62455 0.62455 0.62323 0.62192 0.62063 0.63145 0.603163 0.61564 0.61493 0.61319 0.610183 0.62192 0.62063 0.63145 0.60316 0.61664 0.61464 0.61464 0.61464 0.61464 0.61464 0.61464 0.61464 0.61464 0.61319 0.610183 0.62083
0.62083 0.620</td><td>100. 0.64675 0.64312 0.64493 0.64493 0.64493 0.64493 0.64493 0.64493 0.64493 0.64594 0.62590 0.62492 0.62393 0.62392 0.66392 0.66394 0.66694 0.62590 0.62495 0.66184 0.62690 0.66184 0.66292 0.66394 0.66994 0.66184 0.62590 0.66184 0.66297 0.66394 0.66184 0.66297 0.66394 0.66184 0.66297 0.66394 0.66184 0.66297 0.66394 0.66184 0.66297 0.66394 0.66984 0.66297 0.66394 0.66984 0.66297 0.66394 0.66984 0.66297 0.66394 0.66984 0.66297 0.66394 0.66984 0.66294 0.66394 0.66984 0.66294 0.66984 0.66294 0.66984 0.66984 0.66584 0.66584 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0</td><td>0.064675 0.064675 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.06299 0.062495 0.06299 0.06299 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.06299 0.061881 0.06299</td><td>10. .064675 .064512 .064193 .064037 .063863 .063731 .063582 .063194 110. .063145 .062764 .06276 .062793 .062723 .062792 .062693 110. .063145 .061564 .061404 .061319 .061683 .062792 .062693 130. .06487 .061684 .06144 .061219 .061083 .062994 .060894 130. .06487 .06087 .060894 .08347 .083847 .069894 160. .05567 .065821 .062847 .06681 .055899 .057877 .058847 .055899 170. .05587 .056821 .055474 .055899 .057277 .057279 .05789 180. .05687 .066821 .055474 .055899 .055899 .055479 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .</td><td> 10. 0.064675 0.064512 0.064931 0.064037 0.063883 0.063323 0.063194 0.063184 0.063184 0.063184 0.063184 0.063184 0.063184 0.063184 0.063184 0.063184 0.06484 0.</td><td> 10. 0.064675 0.064312 0.064321 0.064324 0.064313 0.063824 0.063824 0.062323 0.062323 0.062302 0.06232 0.062302 0.062302 0.062302 0.062302 0.062302 0.062302 0.063311 0.062322 0.062322 0.062302 0.063311 0.063311 0.062322 0.062302 0.063311 0.063311 0.062322 0.063311 0.06</td><td> 100</td><td>100 .064675 .064512 .064451 .064453 .064453 .064453 .064455 .06232 .062392 .062393 .062393 .062393 .062393 .062393 .062393 .062393 .062393 .062893 .062893 .062893 .062893 .062893 .062893 .062893 .062893 .062893 .062893 .062893 .062894 .06</td><td> 10. </td><td> 10.</td><td> 10. 0.064675 0.064512 0.064451 0.064493 0.064493 0.062495 0.062323 0.062322 0.062192 0.062192 0.062192
0.062192 0.062192 0.062192 0.062192 0.062192 0.062093 0.063644 0.061564 0.061664 0.061564 0.061664 0.0</td><td> 10.</td><td> 100</td><td> 100</td><td> 100 064675 064512 064432 064403 064403 064403 066242 0662</td><td> 10. 1064675 1064512 1064512 1064613 1064613 1062455 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062652 1062452 1062652 1062452 1062652 1062452 1062652 1062452 1062652</td><td> 10. 1064675 1064512 1064513 1064037 1062459 1062659 1062459 1062459 1062459 1062459 1062459 1062459</td><td> 10. </td><td> 10. 1064675 1064512 1064451 1064413
 1064413 1064413</td></th<></td></td></td> | 100. .064575 .064512 .064193 .064037 .063883 .063731 .062323 .062323 .062343 .062465 .062063 .062192 .062063 .062192 .062063 .062192 .062063 .062192 .062063 .062192 .062063 .062192 .062063 .062192 .062063 .062192 .062063 .062192 .062063 .062192 . | 100. .064575 .064351 .064193 .064037 .063883 .063731 .062323 .062323 .062343 .062465 .062343 .062323 .062192 .062063 110. .063145 .063003 .062864 .064726 .06239 .062323 .062192 .062063 120. .063145 .061804 .061684 .061561 .060191 .0601083 .062323 .062192 .060963 120. .061809 .061684 .061644 .061440 .061919 .061008 .065997 .060967 130. .060627 .060477 .060299 .060191 .060909 .05889 .058897 .059889 150. .058621 .059473 .059826 .058180 .058899 .057877 .057925 160. .056970 .056895 .056747 .056674 .056674 .056674 .055939 .056458 .055458 .055458 .055458 .055466 .055839 .055458 .055466 .055839 .05546 | 0.0 0.0 <td>100. .064675 .064512 .064193 .064037 .063883 .063731 .063362 .062063 1100. .064675 .0646455 .062455 .062323 .062192 .062063 110. .063145 .062864 .062726 .062590 .062103 .062063 .062063 120. .061809 .06191 .061201 .061083 .060967 .060967 .060967 130. .060627 .060407 .060299 .06191 .060085 .059877 .059877 .059877 .059887 .059887 .059887 .059887 .059889 .059889 .059889 .059899 .055899 .055899 .055899</td> <td>100. .064675 .064512 .064193 .064037 .063883 .063731 .063882 .063434 1100. .061845 .064193 .064037 .062455 .062323 .062192 .062043 1100. .061844 .061644 .061319 .061201 .061083 .062092 .062063 120. .061809 .061644 .061440 .061319 .061083 .060967 .060853 130. .060627 .060616 .061440 .06191 .061083 .061097 .060853 130. .060627 .060407 .060299 .06191 .061899 .055897 .059877 .059817 .059817 .059817 .059817 .059817 .059817 .059817 .059817 .059817 .051817 .056428 .055829 .056458 .055817 .055108 .055108 .056458 .056458 .055458 .056458 .055817 .055108 .055109 .056458 .055458 .055458 .055458 .055458 .055458</td> <td>U0. .064675 .064512 .064193 .064037 .063883 .063323 .063382 .063343 1100. .061865 .062864 .062726 .06299 .061201 .061083 .062962 .062063 1100. .061809 .061801 .061809</td> <td>100. .064675 .064812 .064193 .064037 .063883 .063731 .063382 .063343 1100. .06145 .062864 .062726 .062590 .062455 .062323 .062192 .062043 120. .061809 .061864 .061240 .061319 .061083 .062323 .062192 .062045 120. .061809 .061844 .061440 .061319 .061809 .060967 .060967 .060967 .060853 130. .060627 .060299 .06191 .061809 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059887 .059887 .059877 .059887 .059877 .059877 .059877 .059877 .059887 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .055773 .055773 .055773 .055773 .055773</td> <td>U. .064675 .064512 .064493 .064037 .063883 .063731 .063582 .063434 1100 .064675 .064612 .064493 .064693 .064455 .062723 .06292 .062465 .062731 .063434 .062864 .062786 .06259 .062455 .06292 .06299
.06299 .06299 .06299 .06299 .06299 .06299 .06299 .06299 .06299 .06299 .06299 .06299 .06299 .062999 .06299 .06299 <</td> <td>U.0. .064675 .064512 .064131 .063883 .063731 .063882 .063434 1100 .061465 .064512 .064435 .062864 .062726 .062695 .062323 .062192 .062094 1100 .061809 .061804 .061319 .061801 .061833 .062192 .062092 1130 .061809 .061804 .066191 .0601085 .065980 .065987 .060967 1130 .06087 .060191 .060088 .059980 .058987 .065974 140 .059572 .059180 .058989 .059887 .059871 150 .059621 .058180 .058990 .058897 .059874 150 .058621 .058474 .058926 .058990 .058987 .059874 150 .058621 .058474 .058474 .058644 .058094 .058094 .058094 .058094 .058094 .058094 .058094 .058094 .058094 .058094 .058094 .0</td> <td>0.064675 .064512 .064454 .064493 .064037 .064351 .064354 .064363 .064363 .062455 .062323 .062362 .062363 .062455 .062323 .062452 .062455 .062323 .062452 .062465 .062465 .062455 .062323 .062192 .062463 .062463 .062455 .062323 .062192 .062465 .062465 .062465 .062465 .062465 .062469 .0610191 .0610191 .0610193 .06</td> <td>100 .064675 .064512 .064193 .064037 .063883 .063731 .065362 .062063 110 .063145 .062164 .064526 .062390 .062455 .062192 .062063 110 .063164 .061561 .061440 .061319 .062192 .062192 .062063 120 .0661807 .061584 .061561 .061440 .061319 .062192 .062192 .062063 130 .060517 .061584 .061581 .061083 .062192 .062192 .062063 130 .060527 .069517 .061899 .05847 .05840 .05848 .05846 .05846 .05846 .05846 .05846 .05846 .05846 .05846 .05846 .05846 .05846 .05847 .05848 .05846 .05847 .05848 .05846 .05846 .05846 .05846 .05848 .05846 .05848 .05848 .05848 .05848 .05848 .05848 .05848 .05848 .05848<td>100 .064675 .064512 .064493 .064037 .062455 .062323 .062192 .062063 1100 .063145 .062864 .062726 .062590 .062455 .062192 .062102 .062063 1100 .061345 .062864 .062726 .06299 .061083 .062192 .062063 1100 .061340 .061083 .062192 .060191 .062190 .062191 .062192 .062191 .062192 .062067 .062192 .062192 .062069 .060191 .062192 .062192 .062193 .062192 .062193 .062192 .062193 .0621</td><td>10.0 .064675 .06458 .06403 .06403 .06403 .06403 .06403 .06249 .062455 .06233 .062192 .062093 .062192 .062063 .062063 .062063 .062063 .062192 .062063 .062063 .062192 .062063 .062063 .062192 .062063 .062063 .062063 .062063 .062063 .062063 .062063 .062063 .062063 .062063 .062063 .062063 .062063 .0620647 .062063 .0620647 .062063 .0620647 .062063 .0620647 .062063 .0620647 .062063 .0620647 .062063 .0620647 .062064 .0</td><td>100 .064675 .064612 .064951 .064037 .063883 .063331 .063882 .063343 110 .063145 .064604 .062726 .062590 .062455 .062323 .062192 .062063 120 .061809 .062726 .066191 .061809 .062192 .062363 130 .060627 .060407 .066299 .066191 .061908 .058900 .062897 .062064 140 .059972 .066191 .060192 .061998 .058900 .058897 .062192 .066197 140 .059972 .066191 .060191 .060199 .061998 .058900 .058897 .0</td><td>0.00 0.064675 0.064512 0.064931 0.064675 0.064675 0.064513 0.064313 0.064331 0.063145 0.062864 0.062890 0.062893 0.062192 <th< td=""><td>100 .064675 .064612 .0649193 .064037 .062855 .062132 .062192 .062693 .062192 .062093 .062192 .062093 .062192 .062093 .062192 .062093 .062192 .062093 .062192 .062093 .062192 .062192 .062093 .062192 .062192 .062192 .062192 .062192 .062193
 .062193 .</td><td> 10. 0.64675 0.64512 0.64493 0.64037 0.62883 0.63731 0.62382 0.62334 0.63145 0.63303 0.62864 0.62226 0.62295 0.62455 0.62455 0.62323 0.62192 0.62063 0.63145 0.603163 0.61564 0.61493 0.61319 0.610183 0.62192 0.62063 0.63145 0.60316 0.61664 0.61464 0.61464 0.61464 0.61464 0.61464 0.61464 0.61464 0.61464 0.61319 0.610183 0.620</td><td>100. 0.64675 0.64312 0.64493 0.64493 0.64493 0.64493 0.64493 0.64493 0.64493 0.64594 0.62590 0.62492 0.62393 0.62392 0.66392 0.66394 0.66694 0.62590 0.62495 0.66184 0.62690 0.66184 0.66292 0.66394 0.66994 0.66184 0.62590 0.66184 0.66297 0.66394 0.66184 0.66297 0.66394 0.66184 0.66297 0.66394 0.66184 0.66297 0.66394 0.66184 0.66297 0.66394 0.66984 0.66297 0.66394 0.66984 0.66297 0.66394 0.66984 0.66297 0.66394 0.66984 0.66297 0.66394 0.66984 0.66294 0.66394 0.66984 0.66294 0.66984 0.66294 0.66984 0.66984 0.66584 0.66584 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0</td><td>0.064675 0.064675 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.06299 0.062495 0.06299 0.06299 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.06299 0.061881 0.06299</td><td>10. .064675 .064512 .064193 .064037 .063863 .063731 .063582 .063194 110. .063145 .062764 .06276 .062793 .062723 .062792 .062693 110. .063145 .061564 .061404 .061319 .061683 .062792 .062693 130. .06487 .061684 .06144 .061219 .061083 .062994 .060894 130. .06487 .06087 .060894 .08347 .083847 .069894 160. .05567 .065821 .062847 .06681 .055899 .057877 .058847 .055899 170. .05587 .056821 .055474 .055899 .057277 .057279 .05789 180. .05687 .066821 .055474 .055899 .055899 .055479 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .</td><td> 10. 0.064675 0.064512 0.064931 0.064037 0.063883 0.063323 0.063194 0.063184 0.063184 0.063184 0.063184 0.063184 0.063184 0.063184 0.063184 0.063184 0.06484 0.</td><td> 10. 0.064675 0.064312 0.064321 0.064324 0.064313 0.063824 0.063824 0.062323 0.062323 0.062302 0.06232 0.062302 0.062302 0.062302 0.062302 0.062302 0.062302 0.063311 0.062322 0.062322 0.062302 0.063311 0.063311 0.062322 0.062302 0.063311 0.063311 0.062322 0.063311
0.063311 0.06</td><td> 100</td><td>100 .064675 .064512 .064451 .064453 .064453 .064453 .064455 .06232 .062392 .062393 .062393 .062393 .062393 .062393 .062393 .062393 .062393 .062893 .062893 .062893 .062893 .062893 .062893 .062893 .062893 .062893 .062893 .062893 .062894 .06</td><td> 10. </td><td> 10.</td><td> 10. 0.064675 0.064512 0.064451 0.064493 0.064493 0.062495 0.062323 0.062322 0.062192 0.062192 0.062192 0.062192 0.062192 0.062192 0.062192 0.062192 0.062093 0.063644 0.061564 0.061664 0.061564 0.061664 0.0</td><td> 10.</td><td> 100</td><td> 100</td><td> 100 064675 064512 064432 064403 064403 064403 066242 0662</td><td> 10. 1064675 1064512 1064512 1064613 1064613 1062455 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062652 1062452 1062652 1062452 1062652 1062452 1062652 1062452 1062652</td><td> 10. 1064675 1064512 1064513 1064037 1062459
1062459 1062659 1062459 1062459 1062459 1062459 1062459 1062459</td><td> 10. </td><td> 10. 1064675 1064512 1064451 1064413</td></th<></td></td> | 100. .064675 .064512 .064193 .064037 .063883 .063731 .063362 .062063 1100. .064675 .0646455 .062455 .062323 .062192 .062063 110. .063145 .062864 .062726 .062590 .062103 .062063 .062063 120. .061809 .06191 .061201 .061083 .060967 .060967 .060967 130. .060627 .060407 .060299 .06191 .060085 .059877 .059877 .059877 .059887 .059887 .059887 .059887 .059889 .059889 .059889 .059899 .055899 .055899 .055899 | 100. .064675 .064512 .064193 .064037 .063883 .063731 .063882 .063434 1100. .061845 .064193 .064037 .062455 .062323 .062192 .062043 1100. .061844 .061644 .061319 .061201 .061083 .062092 .062063 120. .061809 .061644 .061440 .061319 .061083 .060967 .060853 130. .060627 .060616 .061440 .06191 .061083 .061097 .060853 130. .060627 .060407 .060299 .06191 .061899 .055897 .059877 .059817 .059817 .059817 .059817 .059817 .059817 .059817 .059817 .059817 .051817 .056428 .055829 .056458 .055817 .055108 .055108 .056458 .056458 .055458 .056458 .055817 .055108 .055109 .056458 .055458 .055458 .055458 .055458 .055458 | U0. .064675 .064512 .064193 .064037 .063883 .063323 .063382 .063343 1100. .061865 .062864 .062726 .06299 .061201 .061083 .062962 .062063 1100. .061809 .061801 .061809 | 100. .064675 .064812 .064193 .064037 .063883 .063731 .063382 .063343 1100. .06145 .062864 .062726 .062590 .062455 .062323 .062192 .062043 120. .061809 .061864 .061240 .061319 .061083 .062323 .062192 .062045 120. .061809 .061844 .061440 .061319 .061809 .060967 .060967 .060967 .060853 130. .060627 .060299 .06191 .061809 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059887 .059887 .059877 .059887 .059877 .059877 .059877 .059877 .059887 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .059877 .055773 .055773 .055773 .055773 .055773 | U. .064675 .064512 .064493 .064037 .063883 .063731 .063582 .063434 1100 .064675 .064612 .064493 .064693 .064455 .062723 .06292 .062465 .062731 .063434 .062864 .062786 .06259 .062455 .06292 .062999 .06299 .06299 < | U.0. .064675 .064512 .064131 .063883 .063731 .063882 .063434 1100 .061465 .064512 .064435 .062864 .062726 .062695 .062323 .062192 .062094 1100 .061809 .061804 .061319 .061801 .061833 .062192 .062092 1130 .061809 .061804 .066191 .0601085 .065980 .065987 .060967 1130 .06087 .060191 .060088 .059980 .058987 .065974 140 .059572 .059180 .058989 .059887 .059871 150 .059621 .058180 .058990 .058897 .059874 150 .058621 .058474 .058926 .058990 .058987 .059874 150 .058621 .058474 .058474 .058644 .058094 .058094 .058094 .058094 .058094 .058094 .058094 .058094 .058094 .058094 .058094 .0 | 0.064675 .064512 .064454 .064493 .064037 .064351 .064354 .064363 .064363 .062455 .062323 .062362 .062363 .062455 .062323 .062452 .062455 .062323 .062452 .062465
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0.061664 0.0</td><td> 10.</td><td> 100</td><td> 100</td><td> 100 064675 064512 064432 064403 064403 064403 066242 0662</td><td> 10. 1064675 1064512 1064512 1064613 1064613 1062455 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062652 1062452 1062652 1062452 1062652 1062452 1062652 1062452 1062652</td><td> 10. 1064675 1064512 1064513 1064037 1062459 1062659 1062459 1062459 1062459 1062459 1062459 1062459</td><td> 10. </td><td> 10. 1064675 1064512 1064451 1064413</td></th<> | 100 .064675 .064612 .0649193 .064037 .062855 .062132 .062192 .062693 .062192 .062093 .062192 .062093 .062192 .062093 .062192 .062093 .062192 .062093 .062192 .062093 .062192 .062192 .062093 .062192 .062192 .062192 .062192 .062192 .062193 . | 10. 0.64675 0.64512 0.64493 0.64037 0.62883 0.63731 0.62382 0.62334 0.63145 0.63303 0.62864 0.62226
0.62295 0.62455 0.62455 0.62323 0.62192 0.62063 0.63145 0.603163 0.61564 0.61493 0.61319 0.610183 0.62192 0.62063 0.63145 0.60316 0.61664 0.61464 0.61464 0.61464 0.61464 0.61464 0.61464 0.61464 0.61464 0.61319 0.610183 0.620 | 100. 0.64675 0.64312 0.64493 0.64493 0.64493 0.64493 0.64493 0.64493 0.64493 0.64594 0.62590 0.62492 0.62393 0.62392 0.66392 0.66394 0.66694 0.62590 0.62495 0.66184 0.62690 0.66184 0.66292 0.66394 0.66994 0.66184 0.62590 0.66184 0.66297 0.66394 0.66184 0.66297 0.66394 0.66184 0.66297 0.66394 0.66184 0.66297 0.66394 0.66184 0.66297 0.66394 0.66984 0.66297 0.66394 0.66984 0.66297 0.66394 0.66984 0.66297 0.66394 0.66984 0.66297 0.66394 0.66984 0.66294 0.66394 0.66984 0.66294 0.66984 0.66294 0.66984 0.66984 0.66584 0.66584 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0.66884 0 | 0.064675 0.064675 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.064875 0.06299 0.062495 0.06299 0.06299 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.061881 0.06299 0.06299 0.061881 0.06299 | 10. .064675 .064512 .064193 .064037 .063863 .063731 .063582 .063194 110. .063145 .062764 .06276 .062793 .062723 .062792 .062693 110. .063145 .061564 .061404 .061319 .061683 .062792 .062693 130. .06487 .061684 .06144 .061219 .061083 .062994 .060894 130. .06487 .06087 .060894 .08347 .083847 .069894 160. .05567 .065821 .062847 .06681 .055899 .057877 .058847 .055899 170. .05587 .056821 .055474 .055899 .057277 .057279 .05789 180. .05687 .066821 .055474 .055899 .055899 .055479 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 .055899 . | 10. 0.064675 0.064512 0.064931 0.064037 0.063883 0.063323 0.063194 0.063184 0.063184 0.063184 0.063184 0.063184 0.063184 0.063184 0.063184 0.063184 0.06484 0. | 10. 0.064675 0.064312 0.064321 0.064324 0.064313 0.063824 0.063824 0.062323 0.062323 0.062302 0.06232 0.062302 0.062302 0.062302 0.062302 0.062302 0.062302 0.063311 0.062322 0.062322 0.062302 0.063311 0.063311 0.062322 0.062302 0.063311 0.063311 0.062322 0.063311 0.06 | 100 | 100 .064675 .064512 .064451 .064453 .064453 .064453 .064455 .06232 .062392
.062393 .062393 .062393 .062393 .062393 .062393 .062393 .062393 .062893 .062893 .062893 .062893 .062893 .062893 .062893 .062893 .062893 .062893 .062893 .062894 .06 | 10. | 10. | 10. 0.064675 0.064512 0.064451 0.064493 0.064493 0.062495 0.062323 0.062322 0.062192 0.062192 0.062192 0.062192 0.062192 0.062192 0.062192 0.062192 0.062093 0.063644 0.061564 0.061664 0.061564 0.061664 0.0 | 10. | 100 | 100 | 100 064675 064512 064432 064403 064403 064403 066242 0662 | 10. 1064675 1064512 1064512 1064613 1064613 1062455 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062452 1062652 1062452 1062652 1062452 1062652 1062452 1062652 1062452 1062652 | 10. 1064675 1064512 1064513 1064037 1062459 1062659 1062459 1062459 1062459 1062459 1062459 1062459 | 10. | 10. 1064675 1064512 1064451 1064413
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6	.044897	.044755	.044617	V8+++0+	.044350	122450	*60**0	.043971	.043850	.043731	.043615	• 043502	.043390	.043281	.043174	.043069	•042966	• 042865	•042765	.042668	.042572	• 042478	.042385	•042294	•042205		.042116	•042029	•041944	.041860	.041778	.041697	.041617	.041538	.041461	1861170	V02140.	.041235	•041162	.041090	.041019	676070	.040880	.040811	*04044	•040678
×	.044911	.044769	169440.	0.4440.	.044303	• 04423	.044107	.043983	.043862	.043743	.043627	.043513	.043401	.043292	.043185	•043079	• 042976	• 042875	•042775	.042677	.042581	.042487	•042394	.042303	.042214		.042125	.042038	.041953	•041869	.041786		•041625	•041546	.041468	266140	15140.	242140	•041169	.041097	•041026	•040926	•040886	.040818	.040751	•040684
7	•044926	.044783	*04404	*00440°	9/5440.	947440	.044119	• 043995	•043874	.043755	.043638	.043524	.043412	.043303	.043195	.043090	.042986	.042885	.042785	.042687	.042591	•045496	•04240•	.042312	•04222		.042133	.042047	.041961	.041877	.041794	.041713	.041633	.041554	•041416	004140	*041364 010130	041420	•041176	.041104	.041033	.040963	.040893	•040825	.040757	.040691
9	046340	.044797	.044658	2044D.	787440°	607440.	.044132	•044008	.043886	.043767	.043650	.043536	•043454	.043314	.043206	.043100	.042997	.042895	.042795	.042697	.042600	•042506	7	.042321	.042231		•042145	•042055	.041970	•041885	.041803	.041721	.041641	.041562	•041484	104140	*04133K	162140.	•041184	.041111	•041040	.040970	•040300	.040832	•040164	•040698
R	.044955	.044812	2/9440.	070110	704440.	2/2550.	.044145	.044020	.043898	•043779	.043662	.043547	.043435	.043325	.043217	.043111	.043007	.042905	.042805	.042707	.042610	.042515	• 042422	.042330	.042240		.042151	•042064	.041978	•041864	.041811	•041729	•041649	.041570	265150	CI+1+0•	7 7	*07T*0*	.041191	=	.041047	.040977	.040907	.040838	.04071	•04010
4	696440	.044826	•044686	M+0++0•	.044415	• 044785	.044157	.044032	.043910	.043790	.043673	.043558	• 043446	.043335	.043227	.043121	.043017	.042915	.042815	.042716	.042620	.042525	.042431	.042339	•042540		.042160	•042072	.041987	.041902	.041819	.041737	.041657	.041577	041466	224140	3 6	2/2/50.	•041198	.041126	.041054	•040984	•040914	•040845	•040778	•040711
ഹ	•044984	044840	00/4400	200440	625550	• 044298	.044170	• 044045	.043922	.043802	.043685	.043570	.043457	.043346	.043238	.043132	.043027	.042925	.042825	.042726	•042629	.042534	•045440	.042349	• 042258		.042168	.042081	•041995	.041911	.041827	•041745	•041665	• 041585	.041507	054140.	*041354 041354	·041219	•041506	.041133	.041061	.040991	.040921	•040852	•040784	.040717
7	866440.	• 044854	.044713	0/6++0.	255550	115440.	.044183	.044057	.043934	.043814	.043696	.043581	.043468	.043357	.043249	.043142	.043038	.042935	.042835	.042736	.042639	.042543	.042450	.042358	.042267		.042177	.042090	•042004	.041919	.041836	.041754	.041673	.041593	.041515	0041400	-04136Z	182150	.041213	.041140	.041068	•040998	•040928		~	•040724
_	.045013	.044868	.044727	050440	.044455	.044324	.044195	•044069	•043946	.043826	.043708	.043593	.043479	.043368	.043260	.043153	.043048	• 042945	.042845	•042746	.042648	.042553	.042459	.042367	.042276		.042186	.042098	•042012	.041927	.041844	•041762	.041681	.041601	.041523	.04140.	*041369	*62T*0*	•041520	.041147	.041075	.041005	.040935	•040866	•040798	.040731
0	.045027	.044883	.044741	500440.	694440	.044337	•044508	.044082	•043959	.043838	.043720	•043604	.043491	.043379	.043270	.043163	.043059	.042956	.042855	.042755	.042658	• 042562	.042468	.042376	.042285	•042196	•042195	.042107	.042021	.041936	.041852	.041770	•041689	•041609	.041530	04140	.041377	• 041302	.041228	•041155	.041083	.041012	.040942	.040873	•040805	.040737
$\left(\frac{\sqrt{4\kappa c_{\pm}}}{a}\right)$	550.	560.	570.	280	590	•009	610.	620.	630.	.049	6 50.			089			710.	720.	730.	740	750	760•	770.	780.	790.			810.	820.	830.	840.	850.	860.	870.			• 006 • 006		920•	930.	076	920	•096	940	980•	•066



	101010 5.	''(<i>a</i> /		277	2
6	.038130 .038054 .038054 .0379807 .037943 .037943 .037870	.037485 .037166 .036869 .036592 .036592 .036592	0000000	.034094 .033956 .033957 .033570 .033234 .0331126 .033008 .032906 .032617 .032525 .032525	• 03556
80	.038134 .038058 .038058 .038021 .037983 .037873 .037873	.037518 .037197 .036898 .036619 .036357 .036112	.035454 .035257 .035070 .034720 .034557 .034401	.034108 .033845 .033845 .033846 .033846 .033846 .033833 .03319 .032916 .032916 .032916 .032845 .032845	*17750.
2	.038138 .038100 .038062 .038024 .037987 .037913 .037841	.037551 .037228 .036927 .036946 .035383 .035303	.035474 .035276 .035088 .034908 .03473 .034416	.034122 .033983 .033719 .033719 .033874 .033135 .032926 .032926 .032926 .032926 .032926 .032926 .032926 .032928	002700
૭	.038142 .038104 .038066 .038028 .037991 .037917 .037864	.037585 .037259 .036956 .036673 .036408 .035925	.035495 .035296 .035106 .034926 .034589 .034589	.034136 .033862 .033862 .033486 .033146 .032936 .032836 .0328936 .0328936 .0328936 .0328936	147760.
מ	.038146 .038107 .038069 .037994 .037957 .037864 .037884	.037619 .037290 .036985 .036700 .036434 .035948	.03515 .03515 .035125 .034944 .034605 .034447	.034151 .033875 .033875 .033875 .033815 .033889 .033867 .032847 .032654 .032654	• 0.36677
4	115 003 003 003 003 003 003 003 003 003 00	.037653 .037322 .037015 .036728 .036208 .035971	.03536 .03536 .035144 .034961 .03462 .03462	.034165 .033888 .033758 .033510 .033510 .033951 .033168 .032856 .032856 .032856 .032881 .032881	000700
٣	.038154 .038115 .038077 .038039 .037965 .037891 .037891	.037688 .037354 .037044 .036756 .036486 .035233	.03556 .035162 .034979 .034805 .034478	.034179 .033902 .033902 .033544 .033522 .033178 .032967 .032666 .032666	010300
7	.038157 .038119 .038081 .038064 .037968 .037958 .037895	.037723 .037386 .037074 .036784 .036512 .036257	.035577 .035374 .035374 .034997 .034654 .034694	.034194 .034052 .033915 .033564 .033584 .033189 .032877 .032897 .032682 .032682	
_	.038161 .038123 .038085 .038047 .037972 .037935	.037758 .037419 .037105 .036839 .036239 .036241	.035598 .035200 .035200 .034839 .034671 .034509	.034208 .033929 .033796 .033569 .033569 .033200 .032988 .0327886 .0327886 .032598	
0	.038165 .038127 .038081 .038051 .037976 .037992 .037896	.037793 .037135 .037135 .036840 .036565 .036307	.035619 .035414 .035219 .034856 .034687 .034525	034223 034080 033681 033881 033823 033823 033823 032898 032898 032896 032797 032607	*075746
(40ct)	6400 1500 1510 1520 1550 1550 1590 1590	1600 1700 1800 1900 2200 2300	2800 2800 2800 2800 2800 3900 3000 3000	(b/d)	• > 0 6 5
-	1500 038165 038161 1510 038127 038123 1520 038088 038085 1530 038051 038047 1540 038013 038047 1550 037976 037972 1550 037939 037899 1580 037866 037862 1590 037830 037826	.037793 .037758 .037452 .037419 .037135 .037105 .036840 .036812 .036565 .036539 .036307 .036282 .036064 .036041	2400. 035619 035598 2500. 035414 035394 2600. 035219 03520 2700. 035033 035015 2900. 03487 034871 3000. 034525 034535 3200. 034370	3200 3200 3200 3200 3200 3200 3200 3300 3300 3300 3300 3300 3300 3300 3300 3300 3300 3300 3300 3300 3300 4200 3323 4200 3323 4200 3320 4200 32298 4200 32200	

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6	781250	.032104	3202	3194	3187	3180	າ,	3166	03150	20150	03145	75150	55150	וכוכט	21150	031092	03103	03097	3092	3087	3081	3076	3071	.030663	3061	3056	3051	.030469	3042	.030376	55050	82050	47050	3015	.030115	03007	3003	.029993	5662	2991	298	2983	2	2976	297	5968	965	.029619
•<	ָר רַ בּי	.032111	032	3	3	3	031737	7 7		7	7 (7	7	7 ?		031098	031	030	.030930	8	.030822	8	8	.030668	8	•030269	ຂ	.030474	8	.030381	פאס	9 0	2050	300	030	0300	300	66	568	568	298	2984	98	297	2973	96	2962	• 029623
^	00000	.032119	.032041	.031964	.031889	.031816	031744	•031674	•031605	.031538	.031472	.031408	.051545	.031203	031162	031106	031047	.030990	.030935	.030881	.030828	.030775	.030724	.030673	.030623	.030574	.030526	.030478	.030432	.030386	• 030340	030208	202050	030706	.030124	.030082	.030041	.030001	.029961	• 029922	2988	5884	298	116	297	5969	596	962
9	0000	.032127	03204	3197	3	3182	<u>ر</u> ر	3168	03161	103154	19160	3141	700	03150	03116	3.5	03105	03099	03094	308	oo o	307	~	0306	306	ທ	8	.030483	304	303	5050	2020	2050	305	.030128	0300	00	300	588	5	2988	2984	2981	2977	5	970	5966	963
ĸ	1005	.032135	3205	3197	3190	3183	.031758	316	.031619	.031551	.031485	124150				031116			•030946	• 030892	.030838	.030785	.030734	.030683	.030633	.030584	3053	e e	8	• 030395	030	03030	5 C	770	0301	03009	3004	3000	2996	2993	2989	2982		2977	2974	2970	2 96	.029633
4	66650		03206	031	0	031	750	5		150	9	500	100	102120	36	,	0310	031	0308	308	308	307	3073	030	306	•030289	30	304	304	030	9	200	602050	3 8	030	03009	0300	3001	2662	299	83	2985	298	2978	2974	970	8	963
ĸ	3223	.032151	3207	.031994	3	3184	3177	0/15	03163	93120	03149	24120	15150	02120	91150	.031127	031070	3101	95	3090	94	•030796	•030744	03069	.030643	.030594	ຂ	3049	3045	3040	3035	250	02050	30 CE	03014	3009	05	3001	Q.	2993	2989	•029860	• 029822	• 029785	.029748	2971	2967	.029640
0	176660	.032159	03208	0	.031926	.031852	.031780	607150	03163	75150	05150	044150	10100	10100	91150	031133	03107	03101	.030963	•030908	.030854	.030801	.030749	.030698		3059	.030550	.030502		.030408		.030318	0.00c0	١ ٣	03014	0	9	Δı	2998	5668	2990	986	2982	978	2975	2971	2967	• 029644
	676660	.032168	03208	3201	.031934	.031859	031787	031/16	•031646		_ ,	0.051446	.		, ,	031139	, ,		•030968	.030913	.030859	•030806	.030754	•	•030653	.030603	.030555	.030507	.030460	.030413	9	9 0	,	030534	.030149	0	•030066	.030025	• 029985	• 029945	• 059906	986	2983	616	6	2971	2968	964
0	_	3217	03209	.032018	.031941	3186	6/16	37.75	970	ה מ	12120	7150	7	֓֞֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֡֓֓֡֓֓֓֓֡֓֓֡	֚֡֜֝֜֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓	031145	031	031	030	.030919	.030865	.030812	.030760	.030708	.030658	.030608	.030559	.030511	•030464	.030418	.030372	12030367	0805050	030535	.030153	.030111	.030070	•030059	• 029989	5667	2991	987	298	616	297	2972	2968	.029651
(<u>v#k</u>)	. 200	5100.	5200.	5300.	5400	5500	2000	•00/5	2800	0066	3 8	•0010	•0000	0200	6500	• 00099 • 00099	700	800	•0069	2000		_	_	.007 7400		7600.	7700	7800.	1900	8000	8100		/q	8500	8600	8700.	8800	8900	•0006	9100.	9200	9300	•0056	·0056	σ.	9200	9800	9900° 10000°

Table 6

Values of:

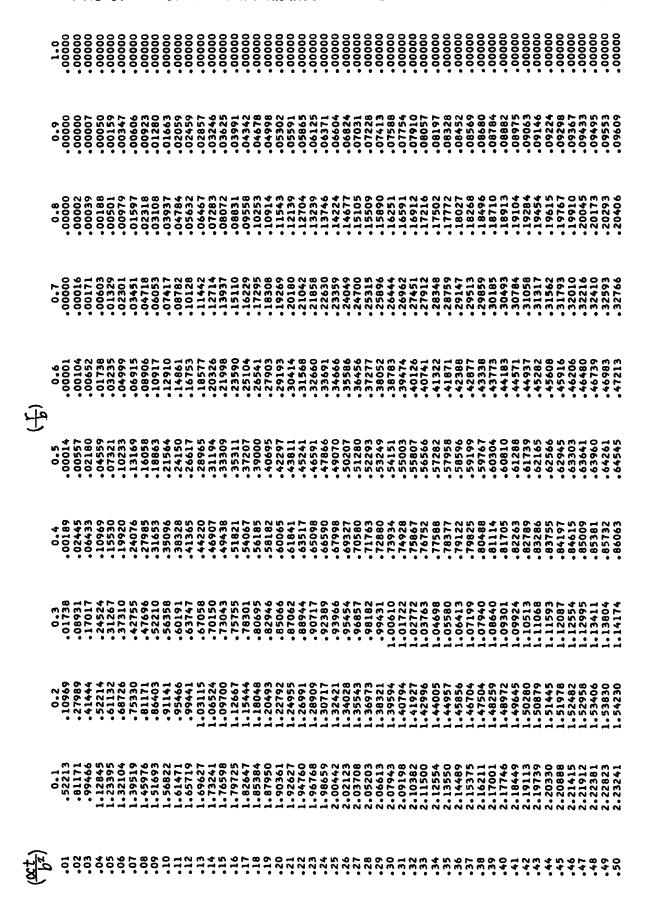
$$\frac{s}{(\frac{Q}{2\pi KD})} = \left[\log_{e}(\frac{b}{r}) - \sum_{n=1}^{n=\infty} \frac{2 J_{0}(\beta_{n}r) e^{-(\beta_{n}b)^{2} (\frac{\alpha t}{b^{2}})}}{(\beta_{n}b)^{2} J_{1}^{2} (\beta_{n}b)} \right]$$

This solution applies where a well at the center of a circular aquifer delivers water at the steady rate Q. The water table level is maintained at the outer boundary r = b. The $(\beta_n b)$ values are roots of $J_0(\beta_n b) = 0$.

To provide a check, the value of $s/(\frac{Q}{2\pi KD})$ will be computed for $(\frac{\alpha t}{b^2}) = 0.20$ $(\frac{r}{b}) = 0.5$ $\log_e(\frac{b}{r}) = 0.693147$

$$\frac{s}{(\frac{Q}{2\pi KD})} = 0.693147 - 0.270178 = 0.422969$$

The tabular value is 0.42297. This is a satisfactory check.



	1.0	00000	00000	00000	0000	00000	00000	00000	00000	00000	00000	00000		00000	.00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	0000	00000	00000	00000	00000	00000	00000	0000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	000000
	0.9	.09661	.09710	•09756	0,000	0880	11660.	*09952	\$8660	10016	.10045	1000	10123	1014	_	.10189	• 10209	.10227	-10244	10261	10276	.10291	• 10305	.10318	.10330	10341		10372	.10382	.10390	.10398	10406	.10414	10450	10433	.10439	.10444	.10449	.10454	.10459	.10463	.10467	.10471	-10475	.10478	.10482	.10485
	0.8	.20514	.20615	2071	20802	.20966	.21042	.21113	.21181	. 21244	.21304	10517	21465	21513	.21558	.21601	.21641	.21679	.21714	.21748	.21780	.21810	85812.	.21865	.21890	21914	21958	21978	.21997	.22015	.22031	. 22047	. 22062	22000	.22102	.22114	. 22126	.22136	.22146	.22156	.22165	. 22173	.22181	.22188	\sim	.22202	. 22208
	0.7	.32929	.33083	.33228	4046	.33616	.33732	.33840	.33943	34040	15145.	94246	37535	34449	.34517	.34582	.34643	.34700	.34755	48	.34854	.34900	54545	.34984	.35022	95056	35125	35156	.35184	.35211	.35237	.35261	.35284	45426	35345	.35363	S.	.35397	S	.35426	.35440	.35452	.35465	.35476	548	S	• 35506
<u>(</u> 2	9.0	.47431	•47636	£ :	40016	.48348	.48501	.48646	.48783	.48912	46064.	047120	49341	49457	.49549	.49635	•49716	.49793	.49865	.49934	.49998	.50059	71106.	.50171	.50222	n u	50359		S	.50475	• 50509	.50541	50571	ľ	50653	5067	.50700	.50721	.50742	076	.50779	• 50796	.50812	.50827	.50842	.50855	. 50868
•	0.5	.64813	• 65066	. 65305	9 4	. 65943	v	.66312	.66480	9	ο,	76600	, 4	v	.67424	75	.67630	.67725	.67814	789	.67978	.68053	47189.	819	825	\$1589°	68423	684	.68521	.68565	.68607	864	.68685	68783	.68785	6881	.68843	.68869	.68894	891	.68940	.68961	.68981	900	*69018	.69034	• 69050
	7. 0	.86376	.86671		o a	.87695	.87916	.88125	.88322	. 88508	x	. 88844 80005	, a		œ	.89547	*89664	.89774	.89878	.89977	. 90070	.90157	04206.	.90318	.90392	.90461	90589	74906	.90702	.90754	.90804	.90850	40894	47000	.91011	.91046	.91078	.91109	.91139	.91166	.91192	.91217	.91240	.91262	.91282	.91302	.91320
	0•3	7	7	1.15165	; -	: :	7	.1647	٦,	1.16907	٦.	1-17663	! -	: :	7	_	₹	1.18323	7	-	٦,	•		7	٦.	~ -	: -	: -	5	v	5	÷:	1.19575			.1974	~	1981	.1984	• 18	• 16	• 18	ຫ		20	1.20031	002
	0.2	• 54	.5496	5529	5501		.5646	.5671	.5695	.5717	62139	1.57778	G706	5812	.5828	.5843	.5857	.5870	.5883	. 5895	. 5906	.5916	9266.	. 5936	. 5945	. 2953 5041	5968.	5975	5	.5988	• 599	009	1.60057		. 601	9	99.	•	9.	9.	99.	9	.6047	• 605	-6052	.6054	•
	0.1	.2363	. 2400	2.24359	2500	2529	.2557	.2584	. 2608	7	.2654	2.26653	27.22	2731	.274	.2763	.2778	.2792	.2805	.2817	.2829	•28	. 2850	.2860	.2869	.2878	2806	2002	. 2	7	.2921	.2927		2062	2947	.2952	.2956	~	-2964	.2967	.2970	.2973	.2976	.2979	.2982	. 298	. 2986
$(\frac{\alpha t}{bt})$		•51	• 52	.53	* "	95	.57	• 58	• 59	09.	.61	79.	77	59.	99•	.67	89•	69•	.70	.71	.72	•73	* i	.75	•7• •1		0 2	80	. 81	-82	•83	• 84	. 85	• a	88	68°	• •	.91	-92	2	•	• 95	96.	.97	86.	•	1.00

Table 7

Values of:

$$\frac{y}{(\frac{Q}{2\pi KD})} = \left[\log_{e}(\frac{b}{r}) + \frac{r^{2}}{2b^{2}} - \frac{3}{4} + \frac{2\alpha_{1}t}{b^{2}} - \sum_{n=1}^{n=\infty} \frac{2 J_{0}(\beta_{n}r) e^{-(\beta_{n}b)^{2}(\frac{\alpha_{1}t}{b^{2}})}}{(\beta_{n}b)^{2} J_{0}^{2}(\beta_{n}b)} \right]$$

With $J_1(\beta_n b) = 0$.

This solution applies where a well at the center of a circular aquifer, of outer radius b, lifts water at the steady rate Q. An impermeable boundary is assumed to be present at the radius b.

To provide a check the value of $y/(Q/2\pi KD)$ will be computed for $(\frac{\alpha_1 t}{b^2}) = 0.20$ where (r/b) = 0.5.

$$\log_{e}(\frac{b}{r}) = 0.693147 \quad \frac{r^2}{2b^2} = 0.125000 \quad 2 \quad (\frac{\alpha_1 t}{b^2}) = 0.40$$

The terms preceding the summation total 0.468147.

$$y/(\frac{Q}{2\pi KD}) = 0.468147 - 0.012137 = 0.45601$$

The tabular value is 0.45601. This is a satisfactory check.

.050000	1.12845	1.45976	1.79862	1.90864	1.99876	2.07510	2.14131	2.19977	2.25213	2.29954	2.34290	2.38288	2.42001	2.45473	2.48739	2.51830	2.54771	2.57582	2.60281	2.62885	2.65406	2.67856	2.70244	2.72579	2.74869	2.77119	2.79334	17618.7	189681	2.87840	2-90046						3.02427	3.04464	990,	085	~	125	145	166	~	206	N	2466
.010000	.7083	3.05432	3.40058	3.51209	3.60321	3.68026	3.74700	3.80588	3.85857	3.90625	3.94983	3.99000	4.02729	4.06214	4.09492	4.12593	4.15542	4-18360	4.21066	4.23675	4.26201	4.28654	4.31046	4.33384	4.35676	4.37928	4.40146	+6674.	4.44496	4.48757		4.52952	ູ	ŝ	'n	٠	4.63248	٠	ŝ	÷	-	۲.	•	۲.		.814	4.83459	730
.005000	41		Ö	ž	.2	4.37327	4.	4	Š	2	3	9	. 7	ř	2	8	8	8	4.90376	4.92985	4.95511	4.97965	5.00356	5.02695	5.04987	5.07239	5.09457	**********	5.15807	5.1804B	5.20172	5.22263	5.24340	5.26408	5.28466	5.30516	5.32559	5.34597	5.36629	5.38657	5.40681	5.42702	5.44720	5.46735	5.48748	5.50760	5.52770	044770
<u>r)</u> 5.001000	7600	5.35629								6.20873	6.25232	6.29249	6.32978	6.36464	6.39743	6.42844	6.45793	6.48612	6.51318	6.53927	6.56453	6.58907	6.61299	6.63637	6.65929	6.68181	6.70399	00(7)0	0.14/49	6.79010	6-81115	6.83205	6.85283	6.87350	6.89408	6.91459	6.93502	6.95539	6.97571	6.99599	7.01623	7.03644	7.05662	7.07678	7.09691	7.11703	7,13713	7 16731
(<u>†)</u> .0005000.	5.70286	6.04943	.3960	6.50758	5987	6758	7425	8014	8541	9018	9454	9856	7.02293	7.05779	7.09057	7.12159	7.15108	7.17926	7.20632	7-23242	7.25768	7.28222	7.30613	7.32952	7.35244	7.37496	7.39714	7061+-1	7.44004	7.48325	7.50430	7.52520	7.54598	7.56665	7.58723	7.60773	7.62816	7.64854	7.66886	7.68914	2	1295	7	6692	2006	310	3302	200
001000	122	7.65887	8.00544	8.11701	8.20817	8.28525	8.35202	8.41092	8.46361	8.51131	8.55490	8.59507	8.63237	8.66723	8.70001	8.73102	8.76051	8.78870	w	w	Ф '	₩.	8.91557	or 1	v (•	0 (1020	2000	7000	9,11373	1346	~	9.17609	-	7	?	9.25798	?	?	÷	ŗ.	•	ů.	.3994	196	.43	
.000050	8.00544	8.35202	8.69859	8.81016	8.90132	8.97840	9.04516	9.10406	9.15676	9.50446	9.24805	9.28822	9.32551	9.36037	9-39316	9.42417	9.45366	9.48185	9.50891	9.53500	9.56026	9.58480	9.60872	9.63210	9.65502	9.67754	9.69972	00171.6	9.14322	0.78586	9.80688	9.82778	9.84856	9.86923	9.88982	9.91032	9.93075	9.95112	9.97145	9.99173	10.01197	ö	ċ	ö	10.09264	ċ	10.13286	10.36.01
010000.	9.61488	•	10.30803	10.41960	10.51076	10.58784	10.65460	10.71350	10.76620	10.81390	10.85748	10.89766	_	·	_	_	11.06310	_	_	_	_	_			_	_	11.30916	- '	_,	11.395.77	11.41632	11.43722	ä	11.47867	ä	ä	11.54019	ä	-	11.60116	ä.	541	1.661	.681	702	11.72220	42	743
.00000g	10,30803	10.65460	11.00117	11.11275	11.20391	11.28098	11.34775	11.40665	11.45935	11.50704	11.55063	11.59080	11.62810	11.66296	11.69574	11.72676	11.75625	11.78443	11.81149	11.83759	11.86285	11.88739	11.91130	11.93469	11.95761	11.98013	12.00231	61470.21	12.04581	12.0862	12,10947	12,13037	12.15115	12.17182	12.19240	12.21290	12.23333	12.25371	12.27403	12.29431	12.31455	12.33476	12.35494	12.37509	12.39523	12.41534	12.43544	12 45552
.00000	.9174	12.26404	. 9	2.7221	12.81334	2.8904	2.95	•	3.06	m	•	m,	m	'n	m,	m	•	'n,	'n	ี ค่า	•	m	m (•	m,	ď,		٠,	•	, ,		'n	•	•	.801	3.822	.842	3.863	3.883	•903	.92	3.944	•964	.9845	•0046	.0247	14.04488	0770
(Oct.)	. 20	20.	40.	•05	90•	.07	80 •	60.	01:	. 1.	.12		.14	•15	•16	.17	.1.	•16	• 50	.21	•22	•23	•24	•25	• 26	.27	• 28	67.	.30	100	20°	.34	•35	•36	.37	• 38	•39	07.	.41	.42	• 43	74.	.45	94.	.47		64.	ď

.050000	3.26652	3066	3266	34	3667	3867	ŝ.	4268	4468	•	7000	5269	ເຄ	5669	S	6909	6569	3.64695	6999	3.68696	3.70696	3.72697	٠,	•	٠,	•	2.8629.c	8669	.8869	-9069	.9269	-9469	6996.	.9869	. 000	• °	7040	0000	1069	1269	1469	.1669	.1869	690	• 22	469
000010*	4.87475	4.91487	9349	•	5	5.	9	5.03507	5.05509	114/0-4	5.11516	5,13515	5.15516	5.17517	5.19518	5.21518	5.23519	5.25519	5.27520	5.29520	5.31520	5.33520	5.35521	5.37521	5.39521	1241521	5-43521 R. 45521	5.47521	5.49522	5.51522	5.53522	5.55522	5.57522	5.59522	27619.6	2,63522	2.65522	20126	5.71522	5.73522	5.75522	5.77522	5.79522	5.81522	5.83522	5.85522
,005000	5.56786		5.62803	5.64807	5.66810	5.68814	5.70816	5.72818	5.74820	27897.5	5. AOR25	5.82826	5.84827	5.86828	5.88828	5.90829	5.92830	5.94830	5.96830	5.98831	6.00831	6.02831	6.04832	6.06832	6.08832	25801.9	56821-9	5.8	6.18832	208	28	248	268	288	208	275	0.54833	ם ס ס	9 6	20.0	448	468	488	8	528	48
(1) (1) (1) (1) (1) (1) (1) (1) (1) (1)	7.17729	• 1975 • 2174	2374	•	•	•			7.35763	1.36765	7.41768	7.43769	7.45770	7.47770	7.49771	7.51772	7.53772	7.55773	7.57773	7.59773	7.61774	7.63774	7.65774	7.67774	7.69775	(1111)	7.151.1	7.7775	7.79775	7.81775	7.83775	7.85775	7.87775	7.89775	6//16-/	7.057.75	01106-1	27.700 -	8-01775	R-03775	8.05776	.0777	.0977	٦.	13	.1577
-)	7.87043	7.91055	7.93060	9206	.9706	•	.0107	8.03076	.0507	2020	8-11082	1308	.1508	7	∹	•	7	8.25087	.2708	.2908	3108	.3308	.3508	.3708	٠,	*4108	8.45089	4709	4909	.5109	.5309	. 5509	5709	.5909	6010.	•	4000	•	8.71090	•	7509	.7709	1909	.8109	8.83090	.8509
001000	9.47987	9.51999	9.54004	9.56008	5801	9.60015	9.62017	6402	2099	200	9, 72026	9.74027	9.76028	9.78029	9.80030	9.82030	9.84031	9.86031	9.88032	9.90032	9.92032	9.94032	9.96033	;	10.00033	ໍ ເ	10.04033	;		0.1	10.14034		1803	ο,	5027	2047.0	\$6097 OT		900	0.3403	3603	0.3803	.4003	•	10.44034	•
090000	10-17302	- 7									10.61361		10.45343	•	•	10.51345		10.55346	10.57346	10.59347	10.61347	10.63347	10.65347	10.67348	10.69348	10.71348	10.78348	10.77348	10.79348	10.81348	10.83348	10.85348	10.87348	10.89349	10.91349	10.95549	10-42549	4600	11.01349	9660	0534	1.0734	.0934	.1134	11.13349	.1534
000000	11.78246	1.8	1.8	8	1.8	1.9	1.9227	1.9427	1.9628	9797.	12,02284		12.06287			12.12289	12.14289	12.16290	12.18290	12.20290	12,22291	12.24291	12.26291	1528231	12.30292	76776 21	12.34292	12,38292	12.40292	12.42292	12.44292	12.46292	12.48292		v 1	76746*71	4"	12 40303	• •	. ~	12.66292	~	12.70292	12.72292	12.74293	12.76293
*000000	12.47560	12.51572	12.53577	12.55581	12.57585	12.59588	12.61590	12.63593	12.65595	12.01390	12,71599	12.73600	12,75601	12.77602	12.79603	12.81603	12.83604	12.85604	12.87605	12.89605	12.91605	12.93606	12.95606	12.97606	12.99606	13.01606	13.05607	13.07607	13.09607	13.11607	13.13607	13.15607	13.17607	13.19607	10017-61	10062.61	10967-61	13 20607	13,31607	13,33607	13.35607	13.37607	13.39607	13.41607	13.43607	4
100000-	14.08504	4.1251	.1452	4.1652	4.1852	4-2053	4.2253	4.2453	4.2053	#•<007#	14.32543	4.3454	4.3654	4.3854	4.4054	4.4254	4.4454	4.4654	4.4854	4.5054	5254	.5454	5655	. 2875	.6055	CC70.	14.66550	.6855	.7055	.725	7455	.7655	.7855	.8055	6670	2422		4400	9255	9455	.9655	.9855	.0055	•0255	045	•0655
$\frac{(\alpha t)}{b^2}$.5.	53.	•54	• 55	•56	.57	e. 8	65.	00.	10.	63	79.	•65	99.	.67	89.	69.	. 10	.71	•72	.73	* :	•75	9	- 62			. 81	-82	•83	₩.	-85	98.	100	•	•	9.0		2.	•	.95	96.	.97	• 98	66.	1.00

	•	8	00000	Э,	.00028	• **	• •	-	œ	•	.03667	.04771	.05993	.07317	16780.	٠,	13405	. ~		_	N	.22155	.23997	.25861	.27744		.31554	. 33479	354))) (.41266	432	3	7107	4112	311	5509	708	0	2	305	8	703	8	~ 1	20	202
		00000		5	.00057		Š		.02259		.04212			8	66	777	14261	159	176	2	212	3	G.	89	8	8	25	.34479	364	400	22.	~	3	100	25	:3	3	8	5	유	\$	3	8	0	720	9	• (00)•
	8.	00000	-00005	• 6003	.00190	92010	.01722	.02587	.03601	.04747	• 00008	.07369	•08816	10339	11926	15363	16996	.18766	.20568	.22397	.24249	.26121	.28011	-29916	.31834	.33763	.35702	.37649	.39603	43630	45500	.47475	.49453	*2416 52410	55404	.57391	.59381	.61372	.63364	.65357	.67351	.69346	.71342	. 73338	. 75335	76677	. 17350
	۲.	00000	5	7	.00603	3 6	0320	0483	2	8	ຫ	<u> </u>	285	3	1642	106	21973	2386	2576	8	2961	5	8	ũ	3	8	32	32	4530	4027	. 51256	2	23	2/2/ 8021	777	ខ្ល	5	719	919	118	œ	518	718	_:	8117		210
	÷	8	001	9	.01738	3,5	690	089	110	3	152	2	194	214	235	210	26	317	338	358	378	66	19	39	29	•	88	2	240 240 340		909	620	9	0 0	Ć	720	0	0	6	2	207	6 04	607	.88078	9007	6/076	•
<u>1</u>	Š	8	800	770	.04559	10,7	131	160	189	16	ů,	69	294	319	345		11,	434	456	477	499	20	‡ 1	2	583	6	24	***	000	7007	. 72635	746	766	000	2 -	847	867	87	.90767	27	~	67	.987	700	.027	500	200
•		8	.02445	2	2;	,,	2407	2799	3167	2	3	8	449	4733	5005	7601	.97681	6008	6243	6473	6699	_	7140	_	.75718	-		28028	8412	0017 8825	. 90303	9234	438	1406	7900	249	.0451	.0652	.0854	.1055	,	.1457	.1658	1.18587	.2059	6677	047.
		5	680	2	.24524		427	476	522	563	602	38	672	707	734	0 0	7	643	867	892	915	D	961	984	.0061	.0278	.0493	1010.	.0918	9221.	1.15437	.17	٦,	17.		27	.29	.31	.33	• 35	•37	.39	41	4	T	- *	• • • •
		0		_	.52214	4 6	5	-	8	,,,,,	.95492	*6966*	1.03212	1.06688	1.09958	1 15005	1.18810	1.21512	1.24117	1-26640	1.29092	1.31481	1.33818	1.36108	1,38359	1.40575	1.42762	1.44923	1.47063	1.51287	1.53377	1.55454	1.57521	1.545.4	1.63672	1.65709	1.67741	1.69769	1.71792	1.73813	1.75831	1.77846	1.79860	1.81871	1.83881	1.85890	1.010.1
	٦.	.52214	117	946	1.12845	3210	3951	4597	.5169	.5682	•	.6575	969.	. 7335	.7678		9556 95569	88	9146	6.	96.	96	.0137	.03	9	9	∹,	٦,	~ -	7 -	2.21122	7	7	4612.	3165	'n	.355	.37	395	.416	.4363	.4564	•4766	965.	.5168	. 5369	. 55 (
	.01	.7083	.0543	. 2568	99	6030	6802	.74	.8058	.8585	-9062	.9498	0066	.0272	.0621	V4V0•	4-15542	183	210	.236	-262	86	4.31046	4.33384	.356	.379	.401	.423	444	004	į	529	.550	5 / C	617	632	.652	•	4.69346	13	.733	.754	.1742	.794	.8144	68345	o
	•001	ŝ	35	Š	<u>آ</u> ،		8	9	110	•16	• 20	• 25	53	32	.3646	•	++074-9 Y 65794	•				•	•	6.63637	•	•	•	•	•	2 6		8	.85	200	6 0		.95	16	6.99599	7.01623	364	•0566	.0767		.1170	.1371	12121
(R.t.)	À	.01	-02	•03	40.	5	20.	80.	60.	-	.11		•13	•1•	•15	٠,	81.	-	.20		•22	.23		•25	•26	.27	•28	•26	930	16.	4 E		•35		7	9 00	04.	.41	.42	.43	44.	.45	• 46			64.	. 20

Table 7. 281

	•	201	66	21014	501	200	900	100	93006	200	9700	•	0	50.	.07	60.	==	=	- 15	•17	-:	12.	4 6 6 6	1.27000	, ,	3.	8	8	3	o.	1.41000	4500	۱ ۲ -	.490		230	. 550	200		70.) C	670	9	22.0	7300	1.75000
	6.	. 78054	œ (84047		₩	.90043	.92042	.94041	.96041	•	1 02040	ָר נָי י	9	3	_	7	7	7	•		7	• .	,,	•		-	"	e,	•	4203	4403	803	.5003	1.52036	403	.5603	5803		6070	40.4	4803	7003	720	7403	9
		132	8332	87324	8932	9132	9332	531	3	.9931	.0131	1660.	0731	.0931	.1131	.1331	.1531	.1731	.1931	.2131	.2331	.2531	2021	1615.	1441	.3531	.3731	.3931	.4131	.4331	1.45314	1604	51	.5331	1.55314	.5731	931	.6131	1000.	1000		7131	7331	1	7731	.7931
	٠.	717	8917	21116.	9517	9717	.9917	117	.0316	.0516	9116	1116	1316	516	.1716	1916	.2116	2316	.2516	2716	91620	3116	3175	1.37168	3016	4116	.4316	.4516	116	.4916	5116	5516	.5716	5916	6116	.6316	6516	6716	9160	7177	7516 7516	712	7016	8116	8316	8516
		0	. 9808	800	.0408	.0608	.0808	.1008	.1208	.1408	9091.	9000	8	2408	.2608	œ	.3008	3208	.3408	.3608	9086	4008	2074.	44 CB	4808	.5008	.5208	608	.5608	.5808	1.60083	. 640B	608	.6808	7008	.7208	7408	. 7608	9000		8608	86.48	3 6	9006	9208	9408
Ţļs	s.	.0880	. 1080	1-14806	1680	.1880	.2080	.2281	.2481	.2681	2881	2201	481	.3681	.3881	.4081	4281	4481	4681	468	2061	5281	1040	1.58814	6081	.6281	.6481	8	8	7081	1.72815	7481	81	8081	8281	.8481	8681	.8881	1006.	1076	1046	080	5 6	0281	0481	0681
1	4.	.2660	-2860	1.32614	.3461	.3661	.3861	•4062	. 4262	.4462	7995	2004.	5262	.5462	.5662	. 5862	• 6062	.6262	.6462	.6662	2989-	7367	7971.	7662	7862	8062	.8262	.8462	.8662	.8862	9062	9467	62	.9862	-0062	.0262	0462	2990•	7000.	7001.	1662	1462	1862	2062	2262	2.24629
	(1)	. 518	538	1.57876	.598	.618	.638	.658	•678	969	73.0	76.0	7,8	. 798	.818	.838	. 858	878	898	918	200	200	000	200	9.6	058	.078	.09	118		2.13897	90	218	. 238	258	.278	298	31B	ט ט ט מ	,,,	ט פ פ	418	4.4	458	478	498
	•5	1.89903	1.91909	1.95918	1.97921	1.99924	2.01927	2.03929	2.05931	2.07933	2.09934	7 1 2 0 2 7	2,15938	2.17939	2.19939	2.21940	2.23940	2.25941	2.27941	2.29942	2+616-2	2.33942	2 27942	2,39943	2,41943	2.43943	2.45943	2.47943	2.49943	2.51943	2.53943	2.57944	2.59944	2.61944	2.63944	2.65944	2.67944	4000	1001	7 0	7704	7007	8196	8394	8594	8794
		5771	5971	2.63729	5573	5773	5974	7174	1374	7574	2.1.148	1714 175	,	3575	3775	n	1175	9375	9575	3775	 	27.60	7575	3.07757	3.09758	3,11758	3.13758	3.15758	3.17758	3.19758	3.21758	3.25758	3.27758	3.29758	3.31758		3575	3.37758	27.75	27.67	י ע	77.5	976	175	375	3.55758
	.01	.8747	8948	934	.9549	.9749	• 9950	.0150	.0350	.0550	1420.	1151	1351	.1551	.1751	.1951	.2151	.2351	.2551	.2752	2067.	2016.	3557 3552	3752	3952	4152	:4352	.4552	.4752	4952	ろしてて	5557	5752	.5952	.6152	.6352	.6552	26/04	7162	7267	7557	7752	7952	8152	8352	8552
	.001	~	•	7.23745	~	7	~	e. (<u>ا</u>	٦,		, 4				7.49771							6577	6777	6977	7117	.7377	.7577	.7777	. 7977	7.81175	8577	8777	1168.	7.91775	.9377	.9577	7776.	0177	7440	7750	7770	7760	1177	13	1577
(<u>&</u>	a	.51	5 c s	 	•55	• 56	.57	.58	•59	Ō٧	104		ø	• 65	99•	.67	89•	69.	2;	7.	21.	27.	5	.76	.77	.78	•79	90	.81	28.	200	8.5	.86	.87	88.	68.	06.	••	20.		0.0	96	265	86	66.	1.00

Table 8

Values of:

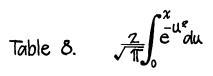
$$\frac{2}{\sqrt{\pi}} \int_{0}^{x} e^{-u^{2}} du$$
 This is the "Probability integral."

This integral table was made by Simpson's rule procedures. The table was prepared in this way to make it fit into the format of the other tables and to avoid the tedious and expensive typesetting and checking operations which would have been required if it were to be made up from values to be found in the excellent existing tables.

The following spot comparisons are made with values from the U.S. Department of Commerce Applied Mathematics Series No. 41.

x	These Tables	Vol. 41*
0	0	0
0.5	0.52050	0.520499
1.0	0.84270	0.842700
1.5	0.96611	0.966105
2.0	0.99532	0.995322
2.5	0.99959	0.999593
3.0	0.99998	0.999977

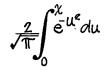
^{*}Last figure not rounded.



•	•01010	-	~ (•	0,	00000	- a	10016	-	_		.14476	~	Ð	.17791	•	О.	~	N	m.	•	S.	·	23	.28584	8	9	_	27	3	•	200	37838	3883	39		_	~	ED.	ş	33	65	45	837	928	.50185	101	196	283
•	0	.02031	m,	.04280	0 (בי כ	56	40660		12	1	Z	n	•	.17681	878	987	960	202	313	421	528	635	742	847	953	051	162	265	368	3470	4 1 C	25105.	3873	3972	070	168	264	9	t S	20	643	736	878	916	96005	860	187	275
_	~	•01918	о.	7	50		7	36	109	120	131	142	153	3	S	œ	•19766	0	~	.23028	∢	.25181	ø	~	æ	σ,	•	-	N (ы ,	7 1	מ ל		38	3	***************************************	2	23	8	7	∢.	9	75	8	5	8	Õ	1	3
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ĸ	5	•	-02820	•	9	5 6	58	9 57		7	12	ĭ	S.	Φ	~	∞ -	or.	.20639	~	N	.23891	.24967	.26037	.27102	.28162	.29217	.30266	.31309	.32346	.33378	80446°	17466.	4764B	.38436	.39428	.40412	-	~	•	•	.45219	•	•	₩.	∞	•	0	_	.52488
4	ĉ	28	0	93	940	• 0000	1710	09456	57	169	12	39	2	Φ	.17241	.18341	.19437	.20530	.21619	.22704	.23784	.24859	.25930	•26996	.28056	.29111	.30161	.31205	.32243	.33275	00646.	¥1556.	3550C+	.38337	.39329	.40314	-	~	m	•	515	909	669	161	883	•49736	063	152	240
e	33	9	.02595	7	048 010	7 6	5 6	, m	2	=	12	m	.14919	•	~	∞ .	œ	0	~	N	m	•	in.	•	~	ъ.	О.	~ (N 1	m i		7	16706.	38	39	0	-	N	312	408	503	297	900	782	873	94964.	.50543	143	231
7	0	5	Ň	Ň.	6	5 6	0 0	0	2		7	3	14	.15916	.17020	.18121	.19218	.20312	.21402	.22487	.23568	.24645	.25716	.26783	.27845	.28901	-29952	.30997	•32036	E C		9 6	75175	38	39	-40117	•41096	*42068	.43031	.43988	Ť			.47732	.48648	ō.	.50454	_	Ñ
	.00113	.01241	.02369	16460	•04624	ה ס	56	09120	101	=	12	13	7	S	•	œ	Or .	0	~	N	m	❖	S.	Ð	~	∞ :	С,	0	~ 1	N		2 4	7500C+	38	39	0	0	_	N	m	.44841	S	•	~	Ø	.49465	0	~	~
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×	00•	.01	-05	• 03	40.	0 0	90.	80	60•	•10	.11	•12	.13	-14	•15	•16	-17	•18	-19	-20	.21	.22	.23	•24	•25	•26	.27	•28	62.	930	 	25.	0 40 0 40 0 40	35	•36	.37	•38	•39	-40	.41	-42	.43	**	.45	•46	.47	.48	64.	• 20

Table 8 284

6	40752	v	.55410	54749	87079		. 5	· LO	60307	0	6186	Ň	+6338¢	•64129	•64865	.65591	-66307	.67014	.67711	•68338	•69076	• 69744	.70403	.71051	.71690	.72320	.72940	.73550	.74151	.74742	.75323	. 75896	.76459	.77012	.77556	16081	.78617	.79133	14961	.80139	.80628	110	158	.82043	249	.82943	338	380	45	6
80	•	3			, ч	, ,	58631	, ,	.60228	•61012	3	.62593	60269	.64055	.64791	.65519	•66236	v	.67642	.68330	60069.	.69678	.70337	.70987	.71627	.72257	.72878	. 73489	.74091	.74683	.75266	.75839	.76403	.76957	.77502	.78038	.78565	. 79082	. 79590	.80090	.80580	.81061	.81534	.81997	.82452	.82899	.83337	.83766	.84187	.84600
~	.53531	54390	55241	56082	56914	47737	15850	.59354	-60149	.60934	.61710	.62477	63233	.63981	.64718	.65446	•66165	.66873	.67572	.68262	.68941	.69611	.70272	.70922	.71563	.72195	.72816	. 73429	.74031	.74624	.75208	.75782	.76347	.76902	.77448	.77985	.78512	.79031	.79540	.80040	8	2	7	13	24	28	32	37	41	Ž.
૭	346	5430	. 6	5500	5683	576	58469	5927		•60856	6	3	63158	G	.64645	S.	•66093	•	_	.68193	•68874	. 69545	.70206	•70858	.71500	.72132	.72755	.73368	.73971	.74565	.75150	.75725	.76291	.76847	.77394	.77932	.78460	. 78979	. 79489	19990	.80482	99608*	₩.	. 81905	N	N	m	3	4	4
R	53358	56210	55071	4552	84748	87873	48.00	59194	16665	•60778	•61556	.62324	.63083	.63832	•64572	.65301	.66022	.66732	.67433	.68129	•68806	.69478	.70140	.70793	.71436	.72069	.72693	. 73307	.73911	.74506	.75092	.75668	.76234	. 76792	.77340	. 77878	• 78408	.78928	. 19439	14661	.80434	80618	.81393	.81859	.82317	.82766	. 83206	.83638	404	447
4	64272	46148	.54987	ď	18	57491	. 6	.59114	2	•	9	·	300	6375	.64498	22	595	99	136	802	873	_	~	•	~	\sim	.72631	.73246	.73851	.74447	.75034	.75611	.76178	.76736	.77285	.77825	• 78355	. 78876	.79388	8	8	.80870	.81346	.81813	.82271	.82721	.83162	.83595	.84020	.84435
m	.53185	15404B	54902	485746	86882	87400	58226	59034	.59832	•60621	.61401	.62171	.62932	.63683	.64424	.65156	.65878	.66591	•67294	.67987	.68671	. 69344	. 10009	.70663	.71308	.71943	.72569	.73185	. 73791	.74388	.74976	. 75553	.76122	.76681	.77231	17777.	.78302	-78824	1684	.79841	.80336	.80822	.81299	.81767	.82226	.82677	.83119	.83552	.83977	-84394
7	69048	53962	54817	55662	86469	47734	58166	58953	.59753	.60543	.61323	.62095	6295 <u>6</u>	.63608	-64351	.65083	-65807	.66520	.67224	•67918	.68603	.69278	.69943	.70598	.71244	.71880	.72507	.73124	.73731	.74329	.74917	.75496	• 16066	.76626	.77176	.77718	.78250	.78773	• 19286	.79791	.80287	.80773	.81251	.01720	.82180	.82632	.83075	•83509	.83935	.84353
-	11055	47878	54732	47.878	56616	57244	58063	58873	.59673	*909*	.61246	.62018	.62780	EF.559*	.64277	.65011	•65735	•66449	.67154	.67849	.68935	.69210	.69877	.70533	.71180	.71817	.72444	.73062	.73671	.74270	.74859	. 75439	.76009	.76570	.77122	.77664	618	872	626	974	023	072	120	167	213	258	303	83	389	431
0	900	100	24446	540	7	1	57982	879	959	6038	116	.61941	270	345	\$	493	566	637	108	778	846	914	981	940	111	175	238	300	361	421	480	538	595	651	708	761	814	998	918	696	910	067	115	162	208	254	298	83	385	8427
×	.8.		, ,	4	, t	1 4	7	60	520	09	.61	-62	Egr	19.	• 65	99.	.67	99.	69.	2.	.71	.72	.73	.74	.75	.76	.77	.78	.79	980	.81	-82	• 83	•84	. 85	•86	.87	88	.89	06•	16.	.92	.93	• 94	.95	• 96	.97	96•	66.	1.00



6	50	.85439	58	2	65	69	72	2	19	83	86	8	92	95	86	.90171	9	6	2	.91269	.91528	.91780	.92026	22	25	27	.92951	.93168	93	35	3	0	.94173	m	S.	~	8	0	N	m	n	w	•	Or .	.96092	22	635	•	53	•
80	500	8540	578	16	654	9	726	.87611	.87953	28	.88614	.88934	.89247	.89552	.89851	.90142	.90428	90206	87606.	.91243	.91502	.91755	.92002	.92243	.92477	.92706	.92929	.93147	.93359	.93566	.93767	.93963	.94154	Ę,	S	.94697	•	20	.95197	53	22	56	ø	2	9	.96211	.96340	2	65	.96705
2	.84964	.85361	.85750	.86131	.86504	. 86869	.87227	.87577	.87919	.88254	.88582	.88902	.89216	.89522	.89821	.90114	.90399	90678	.90951	.91217	.91477	σ		ᢐ	σ	.92684	.92907	.93126	.93338	.93545	.93747	.93944	.94135	.94321	.94503	.94679	.94851	.95018	.95181	.95339	.95492	.95642	.95787	.95928	.96065	86196	.96327	.96453	.96575	.96693
૭	.84924	.85322	.85711	.86093	.86467	.86833	.87191	.87542	.87885	.88221	.88549	.88871	.89185	.89492	.89792	.90085	.90371	.90651	.90924	.91191	.91451	.91705	.91953	σ	o	Ţ	.92885	.93104	.93317	.93525	.93727	.93924	.94116	.94303	.94485	.94662	.94834	.95002	.95165	.95323	.95477	.95627	.95773	.95914	.96051	.96185	.96315	.96440	656	8
Ŋ	.84883	.85282	.85673	.86055	.86430	.86797	.87156	.87507	.87851	.88188	.88517	.88839	.89154	.89461	.89762	95006.	.90343	.90623	.90897	.91164	.91425	.91680	.91929	.92171	.92408	.92638	.92863	.93082	.93296	.93504	.93707	.93905	.94097	45	4	46	48	6	-4	53	54	56	957	.95900	.96038	9	63	642	65	~
4	.84843	.85243	.85634	.86017	.86393	.86760	.87120	.87473	.87817	.88155	.88484	.88807	.89122	.89431	.89732	.90027	.90314	.90595	.90870	.91138	.91399	v	.91904	o.	U.	.92615	.92841	.93061	.93275	.93484	.93687	.93885	.94078	.94266	.94449	ø	8	49	.95132	25	•	55	~	.95886	.96024	615	628	641	653	.96658
'n	480	.85203	.85595	.85979	.86356	.86724	.87085	.87438	.87783	.88121	.88452	.88775	.89091	.89400	.89702	16668.	• 90286	.90568	084	==	37	63	8	12	36	59	.92819	8	23	3	9	86	.94059	2	.94431	8	78	95	Ξ	27	£	58	572	.95872	501	614	627	3	52	9
7	4	851	m	D.	m	866	0	v.	~	•	•	887	0	m	·	899	N	Tax	908	0	m	916	918	920	923	925	927	0	N	•	•	æ	0	~	•		~	GD.	-	N	•	ın	-	•	~	-	N	963	10	•
_	472	.85124	551	290	628	665	707	136	111	905	838	871	902	933	964	993	022	051	078	105	132	9157	9183	9207	9231	9254	277	599	321	345	362	382	102	‡ 21	1 39	157	\$7	161	308	524	540	555	570	584	969	511	525	•	550	662
0	468	Ö	241	586	624	ø	•	~	~	8	œ	.88679	œ	0	On.	0	0	8	5	2	2	5	3	920	2	2	2	٤	931	ĭ	8	8	3	3	Ē.	ŝ	2	5	ö	2	ũ	Ñ	5	8	2	961	62	96	64	.96611
ĸ	•	•	•	1.04	•	•	•	•	•	•	•	•	•	•	•	•	•	•	•	•	•	•		•	•		1.27	•	•		•	•	•	•	•	•	•	•	•	•	•	•	•	-	•	•	1.47		•	•

6	.96830 .96841 .97048 .97152 .97253	91626 91626 91712 91712 91712 9176 9176 9176		· · · · · · · · · · · · · · · · · · ·	4.44 <i>000</i>
×	000000	97528 97617 97617 97787 97787 977947 98096 98167	.98302 .98366 .98366 .98546 .9857 .98557 .98808	· ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~ ~	. 99463 . 99562 . 99528 . 99528
7	.96808 .96919 .97027 .97131 .97233	. W @ @ P @ & O O U U I	.98296 .98360 .98483 .98541 .98597 .98704 .98803	98896 98940 98982 99023 99137 99137 99206 99238 99329 99329 99329	99699999999999999999999999999999999999
૭	.96796 .96908 .97016 .97121 .97322			\ ~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	
æ	.96785 .96897 .97006 .97111 .97213		~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~		444888 444888
4		. O B P B B B B B B B B	88888888888888888888888888888888888888	, 4 3 3 3 3 5 5 5 6 6 6 6 6 6 6 6 6 6 6 6 6	.99428 .99452 .99476 .99498 .99520
ĸ	9676 9687 9698 9709 9719	2224222222	2004224444 30042444444444444444444444444		3444CE
7	9999999 9997777		~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	. 998918 . 998918 . 998081 . 999184 . 999189 . 999189 . 999185 . 999185	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~
_	.96739 .96853 .96962 .97108 .97172	- A IA - A - A - A - A - A	10 ~ 10 ~ m ~ 0 ~ m to 0	99999 999999 999999 999999 999157 999185 999185 99989	~400~~
0		24547886688	.98249 .98315 .98641 .98560 .98558 .98613 .98613		444400
×	សល់សល់សល់	<i> </i>			4. 1.99 1.98 1.98 2.00

Table 8. $\sqrt{2\pi} \int_{0}^{\pi} e^{-u^{2}} du$

െ	.99570	928	96	Ó	Ò	962	96	968	970	971	.99727	973	.99751	916	977	978	979	0	981	82	983	983	984	985	86	986	987	987	86	16866.	989	066	66	7.7	1666	1666	776	7666	444	7 (7 (7	*	966	966	466	9	9	995	. 99959	•
80	S	.99587	96	8	8	Ð	8	8	96	9	6	6	.99750	97	6	9	97	86	86	86	8	.99837	86	86		86	8	.99879	8	06866*	8	66	D (7 (J (5	, (7 6	, 8	, (5 8	5	6	ው	6	6	8	σ	8	
٢	956	. 99585	960	962	963	Š	996	89	696	971	.99725	973	4	916	977	978	919	.99802	981	82	982	98866*	4	985	985	986	987	.99878	988	8	989	8	66	166	5	166	766		0.000	2	993	4	994	994	994	995	995	• 98656	995	6
૭	956	95	9	961	963	965	996	Ø	696	971	.99723	973	.99748	975	116	978	2	980	981	8	982	983	984	985	S	986	987	~	988	88	989	8	66	990	6	166	76	266	0000	566	999	466	994	4	994	95	995	Ō	995	Ō
ħ	986	0	960	961	S.	965	996	œ	696	910	.99722	973	974	975	92	978	19	980	980	981	985	983	84	985	985	986	987	987	88	988	989	8	66	066	166	166	266	7	7 6	200	ان ان ان	466	6	994	994	σ	995		95	966
4	926	5	959	196	96	964	996	967	696	970	.99721	973	974	975	916	977	978	919	980	981	982	983	984	984	985	986	987	987		988	989	989	0	066	166	166	266	7 (776	200	200	5	994	994	994	995	995	93666	995	966
ĸ	955	957	959	196	.99631	3	996	67	696	970	971	973	974	975	ው	977	978	97	980	186	982	983	984	984	O	986	98	98	86	8	98	98	9	66	66	66	6	5	2	2	200	993	994	994	994	995	995	Φ	995	
7	6	.99576	6	8	8	w	8	w	8	~	5	6	6	6	_	6	~	6	8	86	8	86	8	86	8	9	8	86	∞ .	86	8	86	Ò	5 (.99912	D (7 (*2666°		,	~ (56555	0	8	8	ø.	66	66	8	•
-	955	95	959	.99610	962	96	996	9	968	970	.99717	973	•	975	916	116	97	97	98	98	98	98	98	98	98	98	98	98	8	86	98	∞ .	66	66	66	66	7	7 (7566	2.0	3	2.5	966	4	966	9	995	995	995	36
0	955	9	959	096		964	965	.99673	896	970	97	972	974	975	•	977	978	.99795	980	981	985	983	983	984	8	8	8	.99874	86	86	8	Φ,	9	Z (5	5 6	07666	7 (7 (7 0 0	2 C	5 C		466	4	994	995	995	σ	995	95
×	0		۰.	9		9	٥.	٠	٦	7	7	╗	7	∹	∹	⇁	∹	∹	~	Ň	Ň	Ň	Ņ	N	Ň	Ň	Ň	Ň	Ň	ωj (Ū.	ů٠	2.33	J.	ω,	J. (u (J.	4	۲,	t٠	t ·	•	2.44	7.4		4	4	•	2.50

6	2,000	~ (69666	19666*	69666.	0.4865	.99972	£1666°	.99975	ு	v	•	œ	o		.99983	.99984	.99985	96	966	98	966	966	69666	06666.	6666	16666*	* 99992	-99992	* 99992	.99993	66666*	*6666*	999	6	56666	56666.	on.	6	666	9		.99997	.99997	•	16666.	666	•	999	86666*	
80	7000	C 0.000	69886	966	69666.	02666	_	o	•	•	66	61666	6	•	.99982	•	48666.	68666*	98666	0	.99987	9	ø	68666	06666*	06666*	o	16666*	₽.	.99992	ው	o.	666	999	999	\$6666	999	666	999	999	666	666	16666.	16666.	666	666	16666.	666	666	86666*	
-	000073	7000	69666	19666	89666.	02666*	.99972	.99973	.99975	92666.	17666.	499979	.99980	.99981	.99982	.99983	.99984	.99985	98666	98666*	966	866	866	866	06666.	666	666	G	666	666	666	66	666	8	666	Φ.	666	8	6	8	8	8	.99997	16666*	16666.	8	666	16666.	8	86666*	
૭	0000		60555	966	966	166	.99972	σ	+1666 .	σ	.99977	B1666*	8	œ	*99982	ው	*8666*	.99985	\$8666*	•	.99987	6	88666*	68666.	6	06666*	66	66	99	8	99	99	9	666	666	6	666	666	999	999	66	666	96666*	.99997	16666.	.99997	16666.	99	99	86666*	
re	07000		9	96	89666	œ	.99971	.99973	41666.	92666*	11666.	87666.	.99979	7	*99985	.99983	*8666	98666	. 99985	98666*	.99987	98666*	88666*	68666*	06666*	99	666	9	666	666	666	999	666	666	8	Ō	6	•	•	666	666	ው	96666*	.99997	16666.	16666.	σ	16666.	8	86666*	
4	0000	• (97.0	966	ᡐ	o	.99971	97	. 99974	92666	11666.	87999a	6	œ	.99982	ው	GA.	.99984	.99985	98666*	.99987	Q.	88666*	68666*	06666	06666*	Φ	9	9	8	9	9	99	666	999	6	666	666	999	666	666	6	96666*	26666	.99997	96	9	66	666	86666*	
'n	67000	7044	96	966	966	σ	.99971	σ	41666.	766	Ð	82666°	6	966	.99981	* 99982	.99983	*8666*	.99985	ø	.99987	866	88666*	68666*	06666*	06666.	666	O	666	666	999	666	666	666	666	666	666	9	666	666	66	6	96666*	16666.	16666.	666	16666.	666	666	86666*	
7		, ,	49666	9666	966	69666*	.99971	21666.	41666 °	1666	o	87999.	6	666	18666*	66	66	66	9	6	6	6	6	966	6	666	16666*	.99991	.99992	.99992	.99993	.99993	66666*	*6666*	*6666	\$6666	\$6666	\$6666.	\$6666	666	999	96	666	666	666	666	666	666	666	86666*	
- 4	7000) 	966	96	966	966	997	O	997	7666	66	147	166	6	18666*	Q.	ው	ው	σ	•	ው	18666	ው	σ	O	ው	666	σ	666	666	666	666	999	8	8	666	6	6	9	666	666	ው	ው	ᢐ	6	ው	9	6	Ò	ው	
0	7000	9 2 6	966	966	966	9	•	96	41666 °	9	9	-9997B	.99979	o	.99981	66	96	966	9	66	66	66	9	9	68666*	06666*	9	66	66	6	8	ው	66	666	999	666	999	8	666	666	666	96666*	96666*	66		96	ď	66	6666	86666*	666
×	u	•	24.52	2.53	2.54	2.55	2.56	2.57	2.58	2.59	2.60	2461	2.62	2.63	•	2.65	2.66	2.67	2.68	5.69	2.70	2.71	2.72	2.73	2.74	2.75	2.76	2.17	2.78	2.79	2.80	2.81	2.82	2.83	2.84	2.85	2.86	2.87	2.88		•	2.91	•	2	•	•			2.98	•	•

Table 9

Values of:

$$\int_{x}^{\infty} \frac{e^{-u^2} du}{u^2}$$

This integral can be evaluated as

$$\int_{x}^{\infty} \frac{e^{-u^{2}} du}{u^{2}} = \sqrt{\pi} \left[\frac{e^{-x^{2}}}{x\sqrt{\pi}} - 1 + \frac{2}{\sqrt{\pi}} \int_{0}^{x} e^{-u^{2}} du \right]$$

A comparison of independent evaluations made, by use of this expression, and tabular values is as shown below:

x	Computed Value	Tabular Value
0.0001	9998.2276	9998.228
0.001	998.2285	998.229
0.01	98.2377	98.238
0.1	8.32738	8.32738
1.0	0.08907	0.08907

A graphical determination for x = 1.0 yields 0.090. This compares with the tabular value 0.08907.

Table	9.	$\frac{\int_{e^{-U^{z}}du}^{\infty}du}{u^{z}}$
9 5261.336 3446.504 2562.331 2939.044	1693-143 1447-594 1264-651 1121-324 1008-330	524.563 343.076 254.660 202.332 167.743 163.180 110.614
5553.783 3569.656 2629.407 2081.561	1722, 366 1468, 316 1250, 230 1134, 592 1013, 637	553,803 355,391 261,407 206,584 170,665 145,311 126,459 111,391
7 5880.531 3701.932 2700.931 2125.868	1752.614 1490.766 1296.930 1147.654 1029.156	586.483 368.619 268.520 211.016 173.690 147.506 128.124 113.197
6248.228 3844.382 2776.006 2172.141	1783,942 1513,380 1314,018 1161,019 1039,395	623.247 382.864 276.027 215.642 176.823 149.767 1129.832 114.533
5 6664.894 3998.228 2855.371 2220.450	1816.410 1536.690 1331.562 1174.699 1050.860	664.914 398.248 283.964 220.472 180.069 131.587 115.901
4 7141.085 4164.894 2939.404 2270.955	1850.080 1560.728 1349.580 1188.705 1062.058	712.533 414.915 292.367 225.523 183.436 154.502 117.302 104.638
3 7690.535 4346.054 3028.531 2323.809	1885.021 1585.530 1368.091 1203.048 1073.497	767.478 433.031 301.279 230.808 186.930 156.982 1156.239
2 8331.561 4543.682 3123.228 2379.180	1921,305 1611,131 1387,117 1217,741 1085,185	831.580 452.793 310.749 236.345 190.559 137.142 120.205
1 9089.137 4760.133 3224.034 2437.252	1959,012 1637,572 1406,679 1232,796 1097,130	907.338 474.438 320.829 242.152 194.329 162.186 139.098 121.711
0 9998.228 4998.228 3331.561 2498.228	1998.228 1664.895 1426.800 1248.228 1109.340 998.229	998.229 498.248 331.582 248.250 198.251 164.918 123.254 109.366 98.238
. 0001 . 0002 . 0003		0003 0003 0004 0005 0009

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ဇ	50.87812				$\overline{}$	9.55238	8.427.39	+9110°1	6.10313	5.56034	5.03741	4.67519	4.51231	3.70643	3,45037	3.21973	3.01102	2.82138	2.64840	2.49907	7. 14404 7. 14404	2.01082	1.97280	1.86665	1.75797	1.67604	1.59025	1.01005	1.45450	1.29342	1.23625	1.17772	1-12255	1.07050	1.02134	97430	88066	226020		077100	76271	71083	68089
80	53.80110	4.5813	9.1080 5.5268	3.0013	1.1259	6790	4670	0100	7191	6114	1317	,7140	17 40 4	7220	1725	.2417	.0309	8395	.6650	5052	. 5580	1627	9838	8769	.777	.5849	3660	۰. ۱۲۶	2744.	306	245	.1834	.127	.075	.0261	2.5	11666.	9.00	0.0	0.6010.	1087.	70517	.68323
7	9 0	5.29	7.00	3.21	1.29	8	6	5 0	0 0	99	-1	5	กั	2 7	4	3	0	8	9	3	ŋ,	7 -	ŏ	ĕ	7.	9	9	3	4 0	9 6	2	~	Ξ	õ	Ċ.								.68628
9	7	6.0413	0.0126 6.1406	3.4450	1.4613	9413	7400	1613	7897	7160	.2223	7931	4168	1004	5243	2864	.0715	8764	9869	5361	3870	1223	0062	8976	.7968	.7029	6153	5335	4565	2000.	יאיני	.1949	.1387	•0858	.0358	~	8	9014	22.5	``	787		.68930
B	64.90921 38.25254	6-8339	0.494 <i>1</i> 6.4643	3.6771	1.6358	0.0771	8487	8561	7527	7695	2685	8335	4524	70710	5495	3091	.0920	8951	71157	5517	4013	2630	0175	? 0.	8065	.7120	6238	5414	4644	1766.	2606	.2007	.1442	.0909	0406	9931	481	056	552	2 / 0	7907	700	.69233
4	69.67012 39.91821	6733	9988	9165	8149	2162	9597	9467	4152	8238	3154	.8744	4883	8/4T.	5749	3321	1128	.9139	.7329	.5675	4158	.2764	0280	9187	8164	.7212	6324	5494	4719	1666.	2668	2065	.1497	.0961	.0455	9977	9525	.90977	8692	*83076 -	. 79427	19601	.69537
60	75.16362 41.72880	ω,				o.																															869 <u>5</u> 6*	.91394	87317	14458°	. 79783	10000	• 69843
7	81.57288 43.70409	9.5095	2.0790 7.5102	4.4185	2.1883	0.5045	1889	1332	.26/8 5450	9349	.4113	.9579	.5616	.2126	6267	3786	.1549	.9522	.7678	.5994	.4451	3033	1711.	9401	.8363	7397	.6496	.5656	4870	-4133 2663	2793	.2183	.1608	.1066	.0554	.0071	614	181	8771	387	8014	1004	331 015
	4 0	0.516	2.658 7.886	4.681	2.382	0.654	307	229	547	991	460	000	599	242	20 Y .	402	176	971	785	615	460	.317	167	950	846	749	.658	573	494	250	7 8 5	224	166	111	090	.01	65	922	881	47	9	201	. 70460
0	98.23755	1.590	3.267	4.954	2.583	0.807	• 428	.327	874.	046	.509	•043	•636	5/2	206.	426	197	.991	.803	•631	•474	. 330	071	196	.856	.758	.667	581	502	1750	797	230	.172	117	•065	•016	2	26	ສຸ	5	80 6	- 6	20
×	.01	•03	• 0 • • 0	90	.07	80.	60.	.10	•11	13	•14	•15	•16	1.	01.	20	.21	•22	•23	•24	•25	•26	77.	50	30	•31	•32	•33	• 34	3.5	37	•38	•39	04.	.41	•42	2.	•	•45	94.	24.	4.	

	•	Ta	bl	е	9	•										• • <u>e</u>) - (ار الرا	di	<u>u</u>																												
e	.65123	.62355	81706.	54806	61865	.00336	.40251	66795	.44356	. 42536	96194	16168.	. 17539	.35014	34554	.31157	.31318	.30335	.29306	.28128	.26998	591	.24876	.23890	.22924	.22007	.21127	.20233	.19472	.18694	146/1.	.17230	15261	05830	64741	1,1069	13487	.12346	.12427	.11928	.11449	.10989	.10546	.10122	.09713	.09321	or .	.08582
Ø	.65407	.62625	67996.	. 55041	27	0.5	. 48455	.46454	.44542	.42714	9604.	.39294	.37695	.36164	. 34694	.33294	.3194	19908.	.29426	.20243	.27109	.26021	.24978	.23978	•23013	.22097	.21214	.20366	.19552	.1877	.18020	.17300	60991.	6 1 9 4 5	12693	14107	.1354.	.1299"	.12470	.11977	.11496	.11034	.10590	.10163	• 09753	• 09360	.08981	.08618
7	.65693	O 1	.60235	٠.	5246	076	.48660	.46653	.44729	.42593	.41138	•39458	.37852	63	.34841	.33431	.320E1	.30757	• 2954 B	.28359	.27220	.25123	.25081	.24076	.23112	.22183	.21300	.20449	.19032	.18347	18094	37	11991.	01041.	16755	14165	13597	.13053	.12523	.12026	.11543	101	.10633	.10205	*6260*	36860	•09018	•08653
9	.65980	.63171	- 60495 - 57065	.55514	313	960	•48866	.46847	.44918	.43073	.41310	.39623	•38009	.36464	.34986	.33570	.32214	.30914	• 29669	.28476	.27332	.26235	.25183	.24174	.23207	.22278	.21387	.20532	.19712	.18924	.18168	⊸.	_	16076	14815	_	-	20	.12581	.12076	.11591	111125	.10677	02	98	.09437	80	• 08689
Ð	626	4	58195	.55752	.53421	.51197	.49073	•47045	.45107	.43254	•41483	.39788	.38167	.36616	•35131	•33709	.32347	.31042	.29791	.28593	•27444	.26343	.25287	.24273	• 23302	• 22369	-21475	20616	.19792	.19001	-18242	┙,	٥、	16701.	14876		.13709	.13160	.12632	.12125	.11638	.11171	072	028	987	746	.09093	• 08725
4	9	φ,	58446	, ru	.53649	$\overline{}$.49281	.47243	•45296	.43436	.41656	.39954	.38326	.36768	• 35276	.33848	.32480	.31170	.29914	87	75	.26451	.25390	. 24373	.23397	.22461	.21563	0/0	987	5 :	┙,	3,700		10201.	, 6 , 6	•	.13765	.13214	.12684	.12175	.11686	.11217	.10765	•10332	3	- (- 1	.08/61
6	•66849	66669	65710.	.56231	.53879	.51634	646	.47443	.45487	.43612	.41531	.40121	.38486	.36921	.35423	.3365.	.32614	•31298	.30037	.28329	.27670	.26559	.25494	.24473	.23493	.22553	16917.	50702	J (19157	26691.	1,021	16691.		14995	~	.13922	.13268	.12736	.12225	.11734	126	081	.10374	995	ζ,	916	96/80°
2	9	1759	58951	5647	410	185	970	~	ഗ	.43801	20	0	86	.37074	•35569	.34129	.32749	.31427	.30161	.28947	•27784	• 26668	•25599	.24573	235	226	717	208	2007	761	181 181	111	2 7 7	1667	ľ	44	.13878	3	27	22	~	~	80	5	666	200		g
_			.59205		.54340	.52074	.49911	•47845	.45871	.43985	.42182	.40457	.38807	.37228	.35717	.34270	.32884	.31557	.30285	• 29066	•27898	.26778		. 246 /4			ν,		81107.					15751	, –	_	.13935	.13377	.12841	.12326	.11831	11355	10899	•10460	003	200	.09245	88
0	63	644	.59461	569	45	22	5	8	•46065	.44170	.42358	.40626	.38969	.37383	.35865	. 34412	.33020	.3916.	.30410	•29186	-28012	-26888	• 25809	. 24 / /5	237	228	717	017	202	2,43	120	178	771	1 2 2	51	45	39	34	28	23	18	14	60	020	200	900	.09283	200
×	.51	55.	5.5	.55	• 56	.57	• 58	• 59	09•	.61	• 62	•63	• 64	• 65	99•	/9•	.68	69•	• 10	.71	2/5	•73	51°	٠,5	9,6		9.0	. .	2.5	Τ ρ •	, az	000	•	70.	.97	.88	68.	96•	.91	-92	• 93	• 94	.95	96.	/6°	7 C	66.	00.1

Table 9. 293

6	.08234	·	727	69	.05689	.06153	00650*	.05658	.05425	10250	0.00	.04582	.04391	N	.04055	.03702	.03547	.03398	.03254	.03117	220	.02737	. ^	.02508	.02401	.02298	.02104	.02013	.01926	.01942	29110	01611			.01408	34	12	2	۲:	•01122	200	260
શ્	.08268	.07611	~	2	.06443	61	.05925	56	.05448	200	, 4 5 CJ	046	.04410	.04226	04040	-03718	•03562	•03412	.0326	.03130	2 5	3720°	.02631	.02519	.02411	• 02308	.02203	.02022	.01934	.01850	7	010	5 4	0148	141	135	29	123	118	17110.	֓֞֜֜֜֜֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֓֡֓֓֓֡֓֓֓֓֡֓	9 0
2	.08303	.07542	073	2	.06745	52	059	70	.05471	.05245	.04820	.04621	.04429	.04244	.04067	.03734	.03577	.03427	.03282	.03144	11050.	.02760	.02643	.02530	.02422	.02318	• 02218	.02031	.01943	•01859	•01778	01100	01555	.01486	• 01421	0135	53	124	20 (26110.	2 6	.01035 .00987
૭	.08337	767	073	706	• 06774 • 06497	623	059	•05730	• 05494	•05267	.05050	.04640	.04447	.04262	.04085	03750	.03593	•03442	.03297	.03157	*03054 03004	.02772	.02654	.02541	.02432	.02328	• 02228	-02040	.01952	.01867	.01786	*01/08	01561	.01493	.01427	.01364	m	.01246	_	.01137	2	.00991
ß	.08372	•07706	.07392	.07091	.06802	9	00090	.05754	.05517	.05290	.05071	.04660	• 04466	.04280	20150	03766	.03608	.03456	.03311	.03171	.03037	00230	•02666	.02552	.02443	.02338	.02238	02049	.01960	.01875	•01794	• 01/15	01569	.01500	.01434	•01370	31	25	13	.01143	Š	96600.
4	40	ס ~	0742	7	т. и	62	0602	577	. .	0531	.04882	0468	~	•04299	0412	78780	.03624	.03471	.03325	.03185	.03050	12620	.02678	.02563	.02454	.02349	.02248	02058	•01969	.01883	.01802	.01723	01576	.01506	.01440	37	31	N	η,	.01148	Э (.01000
'n	.08441	07770	.07454	.07150	.06859	- 0	റ	.05802	•05564	.05334	.05114	.04700	• 04 5 0 4	.04317	.04137	03798	•03639	.03486	• 03339	.03199	• 03063	.02809	.02689	.02575	•02464	•02359	02750	02067	.01978	•01892	.01810	.01731	01533	.01513	.01447	.01383	•01322	.01263	.01207	.01153	20110.	.01005
64	.08476	-04133	.07485	.07180	. 06887 06606	.06336	•06076	.05827	•05587	.05357	.05136	.04719	•04524	.04335	.04155	.03815	.03655	.03501	.03354	.03212	.03077	.02821	.02701	.02586	.02475	•02369	.02267	-02076	.01986	•01900	.01818	.01/38	01590	.01520	.01453	.01389	.01328	.01269	•01212	.01158	10110.	.01010
-	085	9 8	0751	721	0691	636	0190	in	0561	0537	.04944	.04739	.04543	.04354	.04173	03831	.03671	.03516	.03368	.03226	03030	46420 •	.02713	.02597	.02486	.02380	• 02277	.02085	.01995	•01909	•01826	•01/46	01597	.01527	.01460	•01395	.01334	.01275	•01218	.01164	21110.	.01062
0	0854	-07867	0754	•	990	o cc	0612	~	.05634	.	.05179	·	•04562	.04372	9	03848	, 0	v	.03383	.03240	\circ	<i>-</i>	.02725	0	.02497	•02390	96	.02095	0	.01917	.01834	ગ (, ,	.01534	.01466	140	34	128	122	91	117	.01019
×	1.01	1.03	Ò	0	1.06	? ?	•	7	∵.	7.	1.14	7	7	ີ '	∹ -	: `	. 7	.2	.2	?	۲,	, ,	7		m	<u>ش</u> (1.32	. "	ຕຸ	u.	ů,	. u	. 4	. 4	4	4	4.	4.	٠,	•	•	1.50

	Table	9.		$\int_{a}^{e^{-u^2}du} du$ 29)4
6	.00933 .00691 .00851	.00740	.00673 .00642 .00642 .00584 .00557 .00591 .00483 .00483	000079 000379 000379 000379 000379 000200 000200 000100 000100 000100 000100 000100 000100	.00083
80	.0093 .00893 .00855 .00816	0074	.00645 .00645 .00616 .00587 .00509 .00509 .00485	.00440 .00363 .00363 .00363 .00329 .00234 .00231 .00231 .00100 .001160 .001160 .001160 .001160 .001160 .001160	•00083
1	.00942 .00900 .00859	.00713	.00680 .00649 .00649 .00590 .00583 .00536 .00487 .00465	.00383 .00383 .00385 .003815 .00331 .00285 .002846 .002846 .002846 .002846 .002846 .002846 .002846 .002846 .00183 .00183 .00184 .00183 .00103 .00103 .00103 .00103	• 00084
9	.00947 .00904 .00863 .00824	.00750	.00683 .00652 .00622 .00563 .00565 .00539 .00514 .00490	00349 00349 00332 00332 00332 003316 00287 00287 00287 00287 00287 00287 00287 00287 00287 00287 00287 00192 00192 00194 00115 00104 00104 00104	-00084
R	.00951 .00908 .00867 .00827	.00719	.00686 .00655 .00624 .00568 .00542 .00542 .00492 .00469	00000000000000000000000000000000000000	•00084
4	.00955 .00912 .00871 .00831	.00723	.00689 .00688 .00627 .00598 .00571 .00519 .00495	.00408 .00352 .00352 .00336 .00336 .00326 .00276 .00276 .00276 .00276 .00276 .00175 .00175 .00175 .00176 .00176 .00176 .00176 .00176 .00176 .00176	•00085
ĸ	. 00960 . 00916 . 00875 . 00835	.00726	.00693 .00661 .00630 .00573 .00577 .00497 .00497	00354 003372 003374 003374 003374 00231 00251 00251 00255 00105 00156 00156 00157 00159 00159 00151 00117 00105 00105	• 00085
7	.00964 .00921 .00879 .00839	.00729	.00696 .00664 .006633 .00604 .00576 .00524 .00499		•0008
₩.	.00929 .00925 .00883 .00843	0076 0073 0073	.00699 .00667 .00636 .00579 .00526 .00526 .00458		• 00086
0	.00973 .00929 .00887	.00772	.00703 .00670 .00639 .00539 .00555 .00529 .00504 .00488	00000000000000000000000000000000000000	.00087
×	1.51 1.52 1.53 1.54	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	1.58 1.660 1.662 1.662 1.664 1.666	5	2•00

	To	ibl	е	9	١.																																			;	295
6	.00078	02000	.00067	09000	15000	.00054	15000.	.00048	.00046	600049	60034	.00037	. 60035	.00033	.00031	62000	.00028	00025	.00023	*00055	.00021	02000.	81000	.00017	•00016	.00015	*1000°	.00012	.00012	.00011	01000	60000	60000	.00008	. CC008	70000	10000	90000	90000	.0000	.00005
80	.00075	12000	79000.	09000	15000	.00054	.00051	. 0004	.00046	******	.0004	76000.	.00035	.00033	.00031	.0003	2000	2000	.00024	*C0025	.00021	02000.	51000	1000.	91000*	.00015	400014	.00013	.00012	.00011	11000.	21200	0000	÷0000°	-0000	.0000	10000.	90000	90000.	.00005	. 00005 . 00005
1	.00079	12000	.00063		.00057	.00054	.00057	.0004)	.00046	\$\$000°	.00041	16000	.00035	.00033	.00031	• 000 30	.0002×	0000	.00024	• 00022	.00021	02000	81300	.00017	.00016	.00015	•00014	.00013	.00012	.00011	.00011	01000	60000	80000	£0000°	.00007	20000	30000	90000	90000	.00000
e	.00080	.00072	*0000	• 00064	.00053	• 00055	*00052	.00049	• 00046	• 00044	2,0004.2	00037	.00035	.00033	.00032	• 00030	.00028		.00024	.00023	.00021	000050	91000	0001	.00016	.00015	.00014	.00013	0001	.00011	.00011	01000	50000			10000	10000	.0000	90000	90000	.00005
B	.00080	.00072	89000	50000	.00058	.00055	.00052	.00049	.00047	• 00044	000042	8000	• 00036	.00034	•00032	.00030	.00028	30000	.00024	.00023	.00021	• 00020	81000	.00017	•00016	.00015	.00014	.00014	.00012	.00011	.00011	00010	60000	*0000	80000°	80000·	10000	20000	90000	90000	.00005
4	.00080	.00072	69000	C9009•	.00058	.00055	.00052	• 00020	.00047	• 00044	2,000,0	00038	• 00036	.00034	.00032	.00030	. 00029	7000	.00024	.00023	• 00022	.00020	• 00019	.00017	.00016	.00015	.00014	.00014	.00012	.00011	.00011	01000	60000	60000	.00008	• 00008	10000	,0000	90000	90000	.00005
ሒ	18000.	.000	69000	69000	65000.	95300	• 00053	35000°	.00047	• 00045	.00042	2000	96000	.00034	•00032	• 00030	.00029	40000	. 00024	.00023	.00022	02000	• 00013	-00017	00000	.00015	• 00015	.00013	.00012	.00012	.00011	\$1000°	01000	60000	30000	*0000*	4.0000	/0000	90000	.0000	•00000
7	.00081	.00073	69000	9000	65000°		£8000°	25000°	.00048	• 00045	• 00043	5000	•00036		.00032	.00031	.0002	2000	.00024	.00023	•00022	.00021	• 00019	.00017		.00015	.00015	4000	.00012	.00012	.00011	000010	5000	60000	• 0000E	• 0000B	10000	40000	00000	90000	.00005
71	• 00082	.00074	.00070	• 00066	00059		• 00053	•000020	.00048	• 00045	.00043	85000	• 00036		.00032	•00031	• 00029	• 0002	.00025	.00023	.00022	.00021	07000	00001	• 00016	•00016	.00015	.00014	0001	• 00012	.00011	000010	5000			80000	.00007	70000	90000	0000	.00005
0	.00082	.00074	00000	*00066	00000	.00057	.00054	.00051	.00048	• 00045	.00043	1 t 000	.00037	.00035	.00033	•00031	• 00029	* OCOC	.00025	.00023	.00022	.00021	07000	.00018	.00017	.00016	.00015	• 00014 • 00013	.00012	.00012	.00011	.00010	60000	60000	.00008	• 000¢3	20000	10000	90000	90000	.00005
×	2.01		0	2.05			•	7	7	7	2.13	: -		7	•	٦.	•	•		•	•		•			2.31	•			•	•				2.42	•	4.	2.45	4	4	2.49 2.50

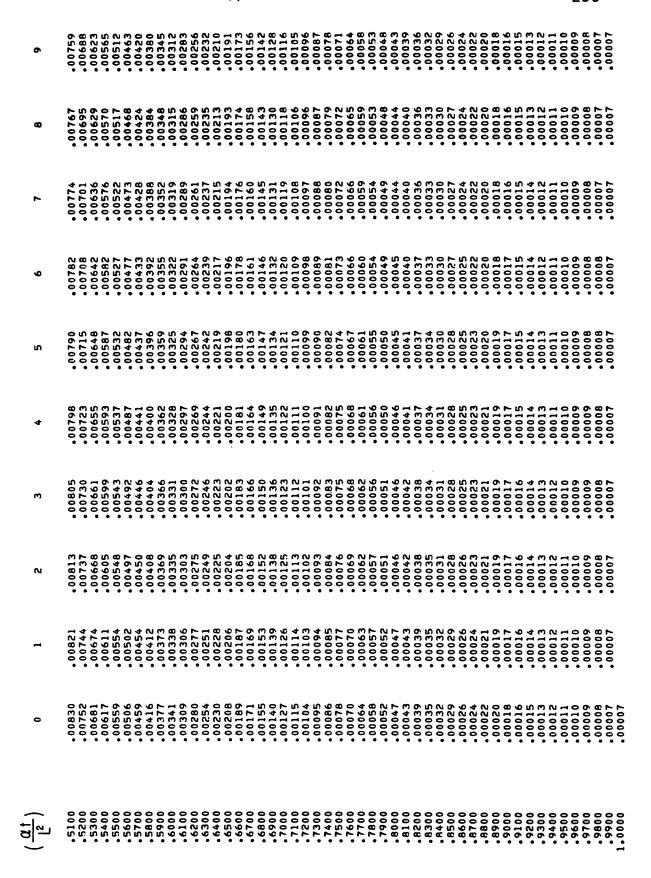
e	• 00005	40000	•0000	*0000*	.00003	£0000°	.00003	•00003	-00005	20000	20000	•00005	.00002	-00002	00005	.00001	10000	10000	.00001	.00001	.00001	.00001	.00001	10000	10000	.00001	.0000	.00001	10000	10000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000
8 0	.00005	0000	• 0000	• 00004	.00003	.00003	.00003	• 00003	• 00005	20000-	20000	-00005	-00002	-00003	• 00002	.00001	10000	1000	00001	.00001	.00001	.00001	.00001	.00001	. 00001	.00001	.00001	.00001	10000	10000	00000	00000*	20000	60000	00000	00000	00000	2000	00000	00000	00000	00000°
7	.00005	40000	• 00004	+0000+	•00003	£0000°	•00003	• 00003	• 00002	-00002	20000°	• 00002	•00005	*0000	• 00002	.00001	10000	10000	.0000	.00001	10000	.00001	• 00001	10000	00001	.00001	.00001	.00001	10000	10000	00000	00000	00000	00000	00000	00000	00000	0000	00000	00000	00000	00000•
૭	.00005	00000	• 00000	• 00004	• 00003	.00003	• 00003	• 00003	-00005	• 00002	20000	• 00002	• 00005	*0000	-00002	.00001	10000	10000	.00001	.00001	.00001	.00001	.00001	.0000	0000	.00001	.00001	.00001	10000	00000	00000	00000	00000	00000	00000	00000	0000	00000	00000	00000	00000	00000
æ.	• 00005	*0000*	, 0000	*0000	£0000°	50000	• 00003	• 00003	.00003	-00002	20000	.00002	• 00002	*0000	-00002	20000	10000	10000	00001	.00001	• 00001	.00001	• 00001	.00001	00001	.00001	.00001	.00001	10000	10000	00000	00000	00000•	00000	00000	00000	20000	0000	00000	00000	00000	00000•
4	.00005	40000	*0000	*0000	£00003	.00003	.00003	• 00003	• 00003	200005	00005	• 00002	• 00002	*0000	-00002	70000	10000	10000	.00001	.00001	.00001	.00001	• 00001	100001	0000	.00001	.0000	.00001	.00001	00000	00000	00000	00000	00000	00000	00000	0000	0000	00000	00000	00000	00000•
'n	.00005	40000	• 00004	.00004	.00003	.00003	• 00003	• 00003	• 00003	- 00002	20000	• 00002	-00002	*0000	• 00002	20000	10000	.00001	.00001	.00001	.00001	.00001	.00001	.00001	10000	.00001	.00001	.00001	10000	10000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000•
7	• 00005	00004	. 00004	*0000	• 00003	00003	8	•00003	•00003	20000	20000	.00002	• 00002	• 00005	• 00002	20000	10000	10000	00001	.00001	.00001	.00001	.00001	.00001	10000	.00001	.00001	.00001	.00001	10000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000
4	• 00005	40000	• 00004	+00000	•00004	00003	0000	• 00003	• 00003	-00002	20000	.00002	-00002	- 00002	• 00005	200005	.00001	10000	00001	.00001		.00001	.00001	.00001	10000	.00001	.00001	.00001	.00001	10000	00000	00000	00000	00000	00000	00000	00000	0000	00000	00000	00000	00000•
0	• 00005	40000	•0000	*0000*	•00004	00003	•00003	•00003	• 00003	•00005	20000	• 00002	-00005	• 00002	*00005	20000	.0000	10000	00001	.00001	.00001	•00001	.00001	• 00001	10000	.00001	.00001	.00001	.00001	10000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000	00000.
×	2.51	2.53	Š	2.55	2.56	2.58	2.59	2.60	2.61	2.62	2.63	2.65	2.66	2.67	2.68	2.69	2.70	2 7 2	2.73	2.74	2.75	2.76	2.77	2.78	2.80	2.81	2.82	2.83	2.84	2.86	2.87	2.88	2.89	2.90	2.91	2,92	2.93		96.2	2.97	2.98	2.99 3.00

Values of:

$$\frac{h_c}{H} = \frac{4}{\pi} \sum_{n=1,3,5,\dots}^{n=\infty} \frac{e^{-n^2\pi^2(\frac{\alpha\tau}{L^2})}}{e^{-n^2\pi^2(\frac{\alpha\tau}{L^2})}} \frac{\sin(\frac{n\pi}{L^2})}{n}$$

The drainable depth at the point midway between drains can be obtained by use of this table.

Φ	.99961	.92424		.70899	.58346			.43419	.39339			26509		.21760	.19715	.17862		79941	12036	.10905		.08951		845.00		.05465		.04486	•04064	•03682	•	.02739			.02037	.01845	•01672	.01515	.01372	.01243	.01127	7000	00000	07000
œ	.99985	. 93078 . 86055	.78684	.71584	58971	.53404	.48394	.43849	.39729	35996	2054	.26771	.24255	.21976	19911	.18039	4 1 6 3 4 4	01361	12155	11013	.09978	.09040	.08191	12470.	26090	.05519	.05000	.04530	.04105	.03719	40550.	.02766	02506	.02270	.02057	.01864	.01689	.01530	01386	.01256	.01138	1010	4000) } •
7	.99995	.93715 .86789	.79415	.72274	.59502	.53932	.48873	•44284	.40123	.36353	20841	.27037	.24496	.22194	.20108	91281.	16506	13560	12276	.11122	.10077	.09130	.08272	404	06150	.05574	.05050	.04575	.04145	.03756	50460	02793	.02531	.02293	.02077	.01882	.01705	.01545	004100	.01268	.01149	1 0 0 0	00843)
v	99999	.94333	6	Ž,	68009	546	49	2	.40521	35.0	הליל הליל	, i-	247	2	0	2	9	13684	2	115	.10177	.09220	083	E 7 0		056	053		4.	63	֡֝֜֝֜֜֜֜֝֜֝֜֜֜֝֓֜֜֜֜֜֝֓֜֜֜֜֜֓֓֓֜֜֜֜֓֓֓֜֜֜֜֓֓֡֓֡֓֜֜֜֜֓֡֓֡֡֡֡֡֓֜֜֡֡֡֡֡֓֡֡֡֡֡֡	28.	025	023	2	5	7	<u>:</u>	3 :	7:	.01160	20	9	;
w	1.00000			- 1		Š			.40923						7				, ,,	_	_	.09312	ö	5 8	.06275	0	9	ч	4 1	5 6	, i	.02849	8	9	w	~	~	5	-	- ,	~ -	• •		•
4	9943	ე გ	8162	7437 4755	2 2	5554	.50340	9	35	ع د د	7 6	22	2	.22861	25	֓֞֜֞֜֜֝֓֓֓֓֓֓֓֜֝֜֜֜֓֓֓֓֓֡֜֜֝֡֓֓֓֡֜֜֝֓֡֓֜֝֡֡֡֡֡֡֡֡	ׅׅׅׅ֝֝֝֝֝֡֝֝֝֝֝֡֡֝֝֡֡֝֡֡֡֝֡֡֡֝	13956	12	1	.10380	-	085		.06337	057	52	47	.04270	200	היים היים	.02877	026	023	2	193	175	0159	4 6	0510	01184	100	088	
ю	00	90	8236	7508	188	5609	083	909	ო.	3676		2812	an .	.23088	2091	1937	7 7 7 7	400	1277	1157	048	949	0860	7770	.06400	579	0525	476	.04312	3450	100	0520	0263	0238	216	195	177	0160	140	1510	20110	800	989)
N	1.00000	.90379	.83100	.75793	.62489	.56651	.51341	.46522	.42152	38196	.31351	28405	.25735	.23317	N •	٠.	- ۲	14235	-	~	.10587	26260	08690	40100	.06463	.05856	• 05305	.04807	.04355	04450. 04450	0.550	.02935	.02659	•05409	•02182	.01977	.01792	.01623	11410	7610.	0100	10000	86800) } }
-	1.00000	.91072	83	9 4	63103	57	.51850	.46983	42	2 4	31662	8	.25990	23	27	, . , .	15867	: 4	<u> </u>	1	2	50	2 6	2 6	.06527	9	9	.04854	.04398	ייי סכ	9 6	.02964	02	9	N.	—	┛,	3 3	-	3 6	3 6	• ~	• 0	•
0	6 0 0	.91755	8		3 (*)	,-	"	4	σι	ט מ	7	6	4	78	2154	2 9	1602	3.5	1315	1191	2	9260	0.000	7277	.06592	6	7	8	.04442	2040	100	0299	0271	0245	222	5	182	0165	2010	0110	֓֞֓֓֓֓֓֓֓֓֓֟֝֟֓֓֓֓֓֓֓֓֓֓֟֝֟֓֓֓֓֡֟֝֡֡֟֝֟֝֡֡֡֡֡֝֡֡֡֡֝֡֡֡֡֝֡֡֡֡֡֡֝֡֡֡֡֡֡֡֡	1010	160	•
$\left(\frac{a!}{\lfloor 2}\right)$.0100	.0300	040	050	0200	080	0060	.1000	.1100	0021	1400	1500	.1600	.1700	180	2 4 6	0000	220	230	240	• 2500	260	2 0 0		3000	310	320	.3300	.3400	350	0025	380	390	400	.4100	.4200	• 4300	440	0004	9 6		067	ស	



Values of:

$$p = \frac{8}{\pi^2} \sum_{n=1,3,5...}^{n=\infty} \frac{e^{-n^2\pi^2(\frac{\alpha t}{L^2})}}{e^{n^2}}$$

The part of the original drainable volume which remains at the time $\,t$, of water moving to drains at the spacing $\,L$, where the aquifer constant is α , can be obtained by use of this table.

Φ	.9322972 .9016302 .8784698 .8590655 .8266551	. 7870978 . 7870978 . 7754554 . 6889274 . 6156948	.5009189 .4532679 .4104329 .3717626	.3051157 .27664336 .2269150 .2055859 .1862643 .1528985	.1137139 .1037139 .0933442 .0845715 .0766233
ω	.9361692 .9042538 .8608836 .8608841 .8436472	.80159059 .80106885 .7765973 .6972241 .6223770	.5059835 .4578032 .4145201 .3754566	.3081431 .2791758 .2591637 .2076257 .1881118 .1704326 .1544150	.1646418 .1040401 .0942700 .0854103 .0773833
۲	.9402918 .9069515 .8827354 .8627267 .8452844 .8296185	.8019703 .8019703 .7895036 .7777351 .7057547 .6291801	.5111073 .4623872 .4186493 .3791879	.3112006 .2819453 .2854442 .2314357 .2096844 .1899776 .1721230 .1559466	.1159808 .1050807 .0952050 .0862575 .0781508
Φ	.9447209 .9097297 .8649275 .8645945 .8311197	.8032604 .8032604 .7788837 .7145401 .5718600	.5162917 .4670206 .4228211 .3829568	.3142886 .2847424 .2337313 .2117642 .1918619 .1574933 .1574933	.1171312 .1061230 .0961493 .0871130 .0799260
ហ	.9495373 .9125961 .8871621 .8664884 .8486120 .8326343	.8045590 .8045590 .7800384 .7236047 .6431766	.5215383 .4717044 .4270361 .3867639	2875672 26875672 26875672 2386497 2138646 1937649 1755544 1590554	.1182930 .1071756 .0971030 .0879771 .0797088
4	.9548648 .9155598 .8894419 .8684095 .8503036	.8058663 .7931647 .7329798 .5503854	.5268485 .4764395 .4312949 .3906096	.3205572 .2904201 .23631215 .2159858 .1956866 .172956 .1606330	.1194662 .1082486 .0980661 .0888497 .0804994
m	.9609118 .9186314 .8917697 .8703591 .8520145	.8071825 .7823995 .7823661 .7426900 .6577462	.5322242 .4812267 .4355980 .3944943	.3237384 .26533013 .2667315 .2181281 .1976277 .1790541 .1622262	.1206512 .1093121 .0990388 .0897309 .0812978
N	.9680846 .9218236 .8941486 .8723385 .8537453	.805077 .8056418 .7835393 .7527845 .6652689	.5376672 .4860672 .4399462 .3984185	.3269513 .2962112 .26431437 .2202917 .1995879 .1638353	.1218478 .1103963 .1000211 .0906209 .0821042 .0743879
1	.9774323 .9251518 .8965823 .8743490 .8554970	.809422 .8088422 .7968917 .7633092 .6729648	.5431795 .4909620 .4443401 .4023826	.3301963 .2391500 .2291500 .2224767 .2015675 .1656603 .1654603	.1230564 .1114913 .1010131 .0915197 .0829185
o	1.0000000 .9286350 .8990747 .8753923 .8572701	. 8111861 . 8111861 . 7981494 . 7859051 . 6808463	.5487632 .4959122 .4487805 .4063873	.3334736 .334736 .274933 .2246834 .2035668 .1844350 .1513968	.125769 .1125971 .1020150 .0924275 .0837409
$(\frac{a_1}{\lfloor 2})$	0.000 0.000 0.001 0.002 0.003 0.005 0.005 0.005		0.000 0.000 0.000 0.000 0.000 0.000	1000 11.000 11.000 11.000 11.000 10.000	22000 22000 22000 22000 2500

٥	0.056985 0.038839 0.038839 0.038839 0.038839 0.03885 0	.0021943
ω	.0575517 .0521428 .0472424 .0487424 .03514352 .03514352 .025144 .02514308 .0251446 .0176945 .01369528 .01369528 .01369528 .01369528 .01369528 .01369528 .01369528 .0046535 .0059625 .0059625 .0059625	.0022160
٢	0581225 0474109 0474109 03912270 0391244 03514884 02812490 02812490 02812490 0281289 0117882 0117882 0117882 0117882 0117882 0117882 0117882 0117882 0117882 0117882 0117882 0117882 0117882 0117882 0117882 0117882 0117882 011782 0117882 011882 01	.0022380
v	05386990 0531873 0345557 0358557 0358557 0358559 03586517 02665117 0179566 0179566 0179566 0179586 0179586 0179586 0179586 0179586 01796933 00606643 00606643 0060663 00606643 0060663 0060663 0060663	.0022602 .0020478
ស	0592812 0440887 0440887 0349452 03599455 0220946 0220946 0122210 0110724 010318 0050273 0050273 0050273 0050273	.0022826
4	0598692 06942447 0645260 0345560 0335560 0335560 02241830 02253137 0155952 0155952 0165952 0165952 017242 0172422 017242 0172422 0172422 0172422 0172422 0172422 0172422 0172422 0172422 0172422 0172422 0172422 0172422 0172422 0172422 0172422 0172422 0172422 017242 017242 017242 017242 017242	.0023053
m	0604630 0647805 06496372 06496372 06696373 067696373 0676983 0676963 0676965	.0023281
æ	0610627 0553239 0653239 0611244 0611246 0611249 06277249 06277249 06277249 06277249 06277249 06277249 06277249 06277249 06277249 06277249 06277249 06277249 06277249 06277249 06277249 06277249 06277249 0627724 0627	.0023512
1	.0616683 .0558726 .0558726 .0558641 .0376484 .0379999 .0229843 .0229843 .0229843 .0127131 .015789 .0167131 .0167131 .0167131 .0167131 .0167131 .0167131 .0167131 .0167131 .0167131 .0167131	.0023745
0	.0622800 .0564268 .0511237 .0511237 .0419458 .0312109 .0236201 .0236201 .0172634 .0172634 .0172634 .0163393 .0163393 .0163393 .0163393 .0163393 .0163393 .01638393 .01638393 .01638393 .01638393 .01638393 .01638393 .01638393	.0023981
(at)	64400000000000000000000000000000000000	.5900

$\left(\frac{a_1}{L^2}\right)$	00019685	.0019492	.0019300	3.0019111	4 •0018923	001	6 •0018553	7	8 •0018191	56
00	.0017835	.0017660	.0017486	5	.0017145	597	5	664	01648	01631
6300	.0016159	.0016000	.0015843	.0015687	•0015533	∞ .	.0015230	508	493	478
6400	.0014640	.0014496	.0014354	5	.0014074	.0013935	.0013798	366	25	.0013396
6500	.0013264	.0013134	.0013005	.0012877	.0012751	.0012626	.0012502	.0012379		233
0099	.0012018	.0011900	.0011783	.0011667	.0011553	.0011439	.0011327	.0011215	.0011105	660
6700	.0010888	.0010781	.0010675	.0010571	.0010467	.0010364	•0010262	.0010161	•	.0009963
6800	.0009865	.0009768	.0009672	.0009577	.0009483	93	•0009298	2	911	.0009027
0069	.0008938	.0008850	.0008763	2	.0008592	0	•0008424	34	00825	œ
7000	.0008098	.0008018	.0007940	.0007862	.0007784	.0007708	.0007632	55	4	.0007410
7100	.0007337	.0007265	.0007193	.0007123	.0007053	•0006984	•0006915	.0006847	578	.0006713
7200	.0006647	.0006582	.0006517	.0006453	.0006390	.0006327	•0006265	•0006204	.0006143	.0006082
7300	.0006023	.0005963	•0002905	.0005847	.0005789	.0005733	•0005676	•0005621	00556	.0005511
7400	.0005457	.0005403	.0005350	.0005297	.0005245	.0005194	.0005143	.0005092	4	.0004993
7500	*0004944	.0004895	.0004847	•0004799	.0004752	•0004706	•0004659	191	.0004568	.0004524
7600	•0004479	.0004435	.0004392	.0004348	.0004306	.0004263	.0004222	.0004180	m	.0004098
1700	.0004058	.0004018	.0003979	.0003940	.0003901	.0003863	.0003825	37	.0003750	.0003713
7800	.0003677	.0003641	.0003605	•0003569	.0003534	.0003500	.0003465	.0003431	.0003398	.0003364
1900	.0003331	.0003298	•0003266	.0003234	•0003202	.0003171	.0003140	.0003109	.0003078	.0003048
8000	.0003018	.0002988	•0002959	.0002930	.0002901	.0002873	.0002845	.0002817	.0002789	.0002762
8100	.0002734	.0002708	.0002681	• 0002655	•0002629	•0002603	.0002577	• 0002552	.0002527	•0002505
8200	.0002477	.0002453	•0002429	.0002405	.0002382	.0002358	.0002335	.0002312	.0002289	.0002267
A300	.0002245	.0002223	.0002201	.0002179	.0002158	.0002137	.0002116	.0002095	.0002074	.0002054
8400	.0002034	.0002014	.0001994	.0001974	.0001955	.0001936	.0001917	.0001898	.0001879	.0001861
8500	.0001843	.0001824	.0001807	.0001789	.0001771	2	.0001737	.0001720	.0001703	.0001686
8600	.0001669	.0001653	.0001637	.0001621	.0001605	•0001589	.0001573	.0001558	.0001543	.0001528
8700	.0001513	.0001498	.0001483	.0001468	.0001454	4	.0001426	.0001412	.0001398	00138
8800	.0001370	.0001357	.0001344	.0001330	.0001317	00130	.0001292	.0001279	.0001266	.0001254
8900	.0001242	•0001229	.0001217	•0001205	.0001194	.0001182	.0001170	.0001159	.0001147	3
0006	.0001125	.0001114	.0001103	.0001092	.0001081	101	.0001060	.0001050	8	.0001029
9100	•101000	.0001000	6660000	•00000	0098	00	.0000961	•	960	.0000933
9200	.0000923	.0000914	•000000	•0000896	.0000888	.0000879	.00000870	.0000862	985	.0000845
9300	.0000837	.0000828	.0000820	.0000812	0	.0000796	.0000788	.0000781	770	.0000765
9400	.0000758	.0000751	.0000743	.0000736	.0000729	.0000721	.0000114	.0000000	.0000000	900
9500	.0000687	.0000680	8	.0000667	•000000	•0000654	.0000647	.0000641	.0000635	.0000628
0096	.0000622	.0000616	.0000610	•000000		•0000592	.0000586	.0000581	.0000575	.0000569
9700	.0000564	.0000558	.0000553	.0000547	.0000542	~	.0000531	.0000526	.0000521	.00000
0086	.0000511	•000000	.000000	•0000496	.0000491	.0000486	.0000481	.0000477	.0000472	.0000467
0066	.0000463	.0000458	8	.0000449	.0000445	.0000440	.0000436	.0000432	.0000428	.0000423
0000	.0000419									

$$R = 1 - \frac{1}{12} \left(\frac{L^2}{\alpha t}\right) + \frac{8}{\pi^4} \left(\frac{L^2}{\alpha t}\right) \sum_{n=1,3,S}^{n=\infty} \frac{e^{-n^2 \pi^2} \left(\frac{\alpha t}{L^2}\right)}{n^4}$$

This function is useful for estimating return flows from irrigation or stream depletions due to distributed pumping. Examples of its use are given in Chapter 10. The factors described are well adapted to digital computer evaluations of return flows or pump depletions.

305

٥	.2073818 .2562075	207	9470	8 0 4	7050	242	0 0 0 0	033	~ 0	יור מי			63913	22159 52159	.6735177	.6838085	.6935767	.7028553	7200444	.7280495	,7356551	.7429039	7564145	.7627145	.7687343	.7744899			515	9446	4249	.8085244	55.72 57.72	374	1027	27543	ē i	.8373421
σ.	200	329570 329570	91867	.4192363	468232	90301	1980T 30356	548575	565718	90106	22	•622020	391	650043 661661	589	682803	525	701948	719917	.7272685	010	4	755768	94	768144	3925	/ / 4550 84750	323	.7946810	341	03812	808104	22210	0001	23668	27197	30597	.8370324
-	₩ Φ Φ Φ	261 590	800	25	4659	881	284	468	.5640504	50000	• •	.6237270	.6366395	04840 64040	.6713937	68179	.6916632	70007	71244	.7264836	.7341632	.7414816	74840	.7614774	76755	.7733592	7842315	78932	2	9888	0337	20 C	7.00	9627	233	2685	30263	.8367217
•	.1165385 .1903066 .2425941	t 10 4	6126	3931 9679	63637	86004	.5265753	5020	.5623721	100	3651	23395	5380	• 64 / 6659 6 F 6 2 6 6 2	6703233	.6807777	.690693	.7001213	717501	7256949	.7334119	74076	75446	7608547	.7669568	. 7727901	7837101	8882	372	342	263	.8072500	7	925	294	2650	2992	.8364098
ហ	1063 1842 2378 2816	319122	219	.4112490	.4613158	w	.5246670	3	.5606837			.6210569	.6341141	4246464	.6692471	.6797568	.6897300	9002669*	* / UBCUU4	.7249023	.7326569	.7400458	76470907	.7602290	.7663589	5	7831865	·	.7932446	J.	0	.8068331	2810218	.8188738	.8225829	"		.8360968
4	0951 1780 2330 2330	15561	80287	.4346809 .4346809	458978	81649		1422	.5589851	591110	5819	.6197105	.6328409	64526 <i>2</i> 457622	.6681653	678730	.6887558	-6982752	715921	.7241059	.7318983	. 7393227	/46401 753154	.7596005	765758	.7716442	7826605	.7878191	. 1927617	.7975002	<u>چ</u>	1004 100	4 4 6	4	222	∞ .	29252	.8357827
e	08240 17154 22816 27330	1958 1958 5262	7331	5820 2154	999	479450	814 814	39606	ις u	5895927	.6043864	.6183565	.6315605	.6440507 4878781	9220199	.6776990	7776	.6973451	יי פיי פיי	. ~	1136	.7385962	7524929	.7589692	.7651551	.7710674	FCF1011.	.7873131	. 1922768	.7970352	01599	780	1422B	18114	21852	44	28913	.8354674
~	.0672835	.3083135	.3743489	.4296087	.4542551	.4772372	.5188698	.5377789	.5555572	5880657	.6029450	.6169949	.6302731	.6428324	.6659842	.6766619	.6867923	.6964103	. / USS481	.7225012	.7303700	.7378662	7518286	.7583349	.7645491	.7704880	7816118	.7868050	.7917899	.7965683	.8011515	.8055501 7577009	מובמבום.	.8177329	.8214851	.8250960	.8285729	.8351509
~	.0475766 .1577939 .2180236	304624 304624 339687	1340	306	45186	0	.5169129	4	.5538276	א מ מ	149	362	897	64160 65356	498	67561	380	69547	/ U405 41238	0	.7296002	.7371327	7511612	7576977	.7639403	6	100	8629	9130	66096	00701	5811508	יות היות	817349	1116	741	2823	.8348334
, °	0.0000000 0.1504506 .2127692	300888	58305	37516 24461	444	2765	.5149436	534090	55208	584984 584984	37	48	576	640375 652395	739	674571	8	9	757507	.7208806	9	Š Š	7504007	57	763328	769321	7 6	78578	0809	628	0220	804684	A1303	6964	820746	84	827887	.8345146
$\left(\frac{a}{\lfloor^2\right)}\right)$	0000	2 0 C	0	9 0	90	8	1200	.1300	.1400	o c	1700	80	0	9 6	.2200	.2300	.2400	.2500	0000	.2800	.2900	3000	0016	3300	40	.3500	> C	8	0	0	.4100	0024	40	50	0	0	000	õ

Table 12. R. 306

Φ	0378 3308 6138 8872	854069 854069 858933 861250 863494 865669	40000000000000000000000000000000000000		5444 55211 55211 5521
60	4007 4302 4585 4860 5125	6 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	77541 77541 77541 77541 7753 7753 7751 7951		00210 00210 00210 00210 00210 10210 10210 10220 10220
۴	9779 2730 5580 8332 0993	85356 85356 85356 86305 86305 86523	1414 1414 1414 1414 1414 1414 1414 141	0.04694009000000000000000000000000000000	64995 64995 61110 61113 61113 6110 6420 6420 6420
v	44 44 40 43 43	853312 853312 853224 860563 862828 865023	25 1 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	88868888888888888888888888888888888888	138 138 138 138 138 138 138 138 138
ហ	39176 42148 45018 47789 50468	853057 853057 853562 860332 862605 866941	71019 71019 72968 76698 76698 80220 81907	•► 0.0 ~ D B C C D F O C 4 D C	10000000000000000000000000000000000000
4	3873 1856 4735 7517	852802 852315 857747 860101 862381 866731	2000 2000 2000 2000 2000 3000 3000	3 4 4 M M M M M M M A A A A A A A A A A A	04649 04669 06746 06749 07828 07828 11731 11731 11731 11731 11731 11768
ю	8569 1562 4452 7243 9939	855566 855069 8575069 859869 862156	2	88653 88653 88693 88693 8962 8962 8963 90011	00000000000000000000000000000000000000
~	3826 4126 4416 4696 4967	55229 52481 53481 53963 6415	7045 7045 7463 7463 7463 830 830 830 830	~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	00000000000000000000000000000000000000
-	795 097 388 669 940	.8520324 .8545705 .8570262 .8594031 .8617049 .8660960	84702 8702 8702 8702 8703 8812 8812	. 8905540 . 89105540 . 8913464 . 8933464 . 8946904 . 8972800 . 8972800 . 9009359	9043420 9054261 9054261 9075227 9095267 9104995 9114498 9132913 9132913
6	00000	.8517739 .8543205 .8567842 .8591689 .8614780 .8658829	.870034 .8720034 .8739219 .8757865 .8757865 .8757863		9065260 9065388 9065388 9065388 9065388 9113557 9113557 911499713 9166709
$(\frac{\alpha t}{\lfloor^2})$. 5 2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	2 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	6500 6500 6500 6500 6500 7000 7100	7300 7400 7400 7700 7700 7700 8200 8200 8400 8500	9800 9900 9900 9900 9900 9500 9500 9700

Values of the integral:

$$\int_{x}^{\infty} e^{-u^2 - \frac{m^2}{u^2}} du$$

This table was prepared by Simpson's rule integration. The Simpson's rule evaluation of the integral is approximate but an exact evaluation can be obtained in the form:

$$\int_{x}^{\infty} e^{-u^{2} - \frac{m^{2}}{u^{2}}} du = \frac{\sqrt{\pi}}{4} \left[e^{2m} + e^{-2m} \right] - \frac{\sqrt{\pi}}{4} \left[e^{2m} \frac{2}{\sqrt{\pi}} \int_{0}^{(x + \frac{m}{x})} e^{-u^{2}} du \right]$$

$$+ e^{-2m} \frac{2}{\sqrt{\pi}} \int_{0}^{(x - \frac{m}{x})} e^{-u^{2}} du$$

The integral from zero to infinity is:

$$\int_{0}^{\infty} e^{-u^{2} - \frac{m^{2}}{u^{2}}} du = \frac{\sqrt{\pi}}{2} e^{-2m}$$

The integral from zero to x is:

$$\int_{0}^{x} e^{-u^{2} - \frac{m^{2}}{u^{2}}} du = \frac{\sqrt{\pi}}{4} \left[e^{2m} \frac{2}{\sqrt{\pi}} \int_{0}^{(x + \frac{m}{x})} e^{-u^{2}} du + e^{-2m} \frac{2}{\sqrt{\pi}} \int_{0}^{(x - \frac{m}{x})} e^{-u^{2}} du \right]$$
$$- \frac{\sqrt{\pi}}{4} \left[e^{2m} - e^{-2m} \right]$$

A check of the tabulation can be obtained by comparing values computed from these integrals with the tabular values. Such comparisons are shown below:

m	$\frac{\sqrt{\pi}}{2} e^{-2m}$	Tabular Values
0.001	0.88446	0.88448
0.002	0.88269	0.88264
0.003	0.88093	0.88091
0.004	0.87917	0.87915
0.005	0.87741	0.87739
0.006	0.87566	0.87564
0.007	0.87391	0.87389
0.008	0.87216	0.87214
0.009	0.87042	0.87040
0.010	0.86868	0.86866
0.020	0.85148	0.85146
0.030	0.83462	0.83460
0.040	0.81809	0.81807
0.050	0.80189	0.80187
0.060	0.78601	0.78599
0.070	0.77045	0.77043
0.080	0.75519	0.75517
0.090	0.74024	0.74022
0.100	0.72558	0.72556
0.200	0.59406	0.59404
0.300	0.48637	0.48635
0.400	0.39821	0.39819
0.500	0.32602	0.32601
0.600	0.26693	0.26691
0.700	0.21854	0.21852
0.800	0.17893	0.17891
0.900	0.14649	0.14647
1.000	0.11994	0.11992

Since the Simpson's rule integration was made from x = 3.0 backward toward x = 0 a check between the integral and tabular values in the above comparison provides, for each value of m, a check of all tabular values between those limits.

Some spot checks for m = 0.001 are the following:

	$\int_{0}^{\infty} -u^2 - \frac{m^2}{u^2}$	
x) e du x	Tabular Values
0.1	0.78655	0.78653
0.2	0.68886	0.68884
0.3	0.59499	0.59497
0.4	0.50657	0.50655
0.5	0.42495	0.42493
1.0	0.13940	0.13938
1.5	0.03004	0.03002
2.0	0.00415	0.00413

The approximate integration is seen to be within about two units in the fifth place. Values shown are rounded to the nearest figure in the fifth place.

Table	13.	e ^{-u²} u²	du.
.09 .79644 .69847 .60413	. 23275 . 35807 . 29169 . 29385 . 18465 . 16916 . 06136	.02173 .02173 .01091 .01005	.00275 .00275 .001071 .00005 .000070 .000005
.08 .80637 .70813 .61335	. 44066 . 36517 . 29795 . 18902 . 11689 . 11224 . 08432	.02254 .02254 .01550 .01046	.00006 .00000 .00000 .00000 .00000 .00000
.07 .81631 .71783 .62263	. 19967 . 19967 . 19967 . 19967 . 11539 . 06683	.0233 .0233 .01610 .01089	. 00040 . 00040 . 00040 . 00023 . 000023 . 00000
.06 .82626 .72756 .63195	. 45669 . 37963 . 31071 . 25031 . 19841 . 11861 . 08940 . 06624	.03450 .02450 .01673 .01133	.00492 .00492 .00121 .00073 .00024 .00006 .00006
. 05 . 83623 . 73732 . 64132		.03570 .02513 .011737 .01179	.00514 .00330 .00207 .00077 .00045 .000046 .00007
.04 .84621 .74711 .65073	. 47303 . 39440 . 32382 . 26170 . 20812 . 16281 . 12525 . 09473	. 03694 . 02605 . 01804 . 01227	.00345 .00345 .00217 .00134 .00081 .00027 .00007
.03 .85618 .75693 .66020	. 48130 . 40192 . 33050 . 26753 . 21310 . 12867 . 07261	. 03821 . 02700 . 01873 . 01276	.00560 .00361 .00361 .00128 .00016 .00016 .00008
.02 .86616 .76677 .66970		. 01946 . 01946 . 01927 . 01927	.00585 .00377 .00239 .00148 .00053 .00017 .00009
.01 .87611 .77664 .67925	. 49807 . 41718 . 34412 . 27944 . 22331 . 17556 . 10320 . 07712	.00088 .02898 .02018 .01380	.00610 .00395 .00395 .00250 .00056 .00032 .00009 .00009
.00 .88448 .78653 .68884	. 2045 . 50655 . 42493 . 35105 . 28552 . 2854 . 17997 . 13938 . 10615	.04227 .04227 .03002 .01435	.00637 .00262 .00262 .00163 .00034 .00010 .00005
80-0r	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	. 4 & 0 ~ 0	0.987887809

Table 13. 311

	60.	.79641	.69845	.60413	.51511	.43275	.35807	.29169	.23385	.18445	.14310	10916	.08186	.06033	.04369	.03109	.02173	.01491	.01005	.00665	.00431	.00274	.00171	.00105	• 00062	•00036	.00020	.00011	.00005	.00002	00000
	• 08	.80633	.70812	.61335	. 52373	.44065	.36517	.29794	.23925	.18902	• 14689	11224	.08432	.06225	•04516	•03219	.02254	.01550	•01046	.00693	.00451	.00287	.00180	01100	• 00066	• 00038	.00021	.00011	90000	• 00005	00000
	.07	.81627	.71781	.62262	.53242	.44863	.37236	.30428	.24474	.19367	.15075	.11539	.08683	.06422	.04667	.03333	.02337	01910	.01089	.00723	.00471	.00301	.00188	•00116	69000*	.00040	.00023	-00012	• 00000	•00005	00000
	90•	.82622	.72754	.63194	.54117	.45669	.37962	.31071	.25031	.19841	.15469	.11861	.08940	•06624	.04823	.03450	.02424	.01673	.01133	•00754	.00492	.00315	• 00198	.00121	.00073	.00043	•0005	•00013	• 00000	• 00003	.00001
	• 05	.83618	.73730	.64131	.54998	•46482	.38697	.31722	.25596	.20322	.15871	.12189	.09204	.06831	.04982	.03570	.02513	.01737	.01179	.00786	•00514	.00330	.00207	.00128	.00077	.00045	•00056	• 00014	.00007	.00003	.00001
	• 0	.84614	.74709	.65073	.55886	.47302	.39440	.32382	.26170	.20812	.16281	.12525	.09473	.07043	.05146	.03694	.02605	.01804	.01227	.00819	.00537	.00345	.00217	.00134	.00081	.00048	.00027	.00015	.00007	.00003	.00001
	• 03	.85609	.75691	•66019	.56780	.48130	.40191	.33050	.26753	.21310	•16698	.12867	• 09749	.07261	.05314	.03821	.02700	.01873	.01276	.00854	09500	.00361	.00228	.00141	• 00085	• 000050	• 0005	• 00016	• 00008	. 00004	• 00001
	•02	.86602	.76675	69699•	.57680	.48965	.40950	.33727	.27344	.21816	.17123	.13217	.10031	.07484	.05487	.03953	.02797	.01944	.01327	•00889	•00585	.00377	•00239	•00148	06000.	.00053	• 00030	.00017	60000	*0000*	.00001
	.01	.87582	.77662	.67924	.58585	.49806	.41717	.34412	.27944	.22331	.17556	.13574	.10320	•07712	•05664	.04088	.02898	.02018	.01380	• 00926	.00610	.00395	.00250	.00155	•0000•	• 00056	•00032	*1000	•0000•	•0000•	.0000
• 002	00•	.88264	.78651	.68883	.59496	.50655	.42492	.35105	.28552	.22854	.17997	.13938	.10615	•07946	.05846	.04227	•03002	•0209	.01435	• 00965	.00637	.00413	*00262	.00163	66000*	• 00059	•00034	•1000•	.00010	• 00005	.00002
E E	×	•	٠,	.2	۴,	4.	٠,	9.	٠.	8	6.	1.0	1:1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2	2.3	5.4	2.5	2.6	2.7	2.8	2.9 3.0

M= .003

3.

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	60.	.79630	.69841	.60410	.51509	.43274	.35806	.29169	.23385	.18445	.14310	10916	.08186	.06033	.04369	.03109	.02173	.01491	.01005	• 00665	.00431	.00274	.00171	.00105	• 00062	•00036	.00020	11000	.00005	.00002	00000
	• 08	.80620	. 70807	.61332	.52371	.44064	.36517	.29794	.23925	.18902	.14689	.11224	.08432	.06225	.04516	.03219	.02254	.01550	•01046	.00693	.00451	.00287	.00180	•00110	99000	.00038	.00021	.0001	90000	*0000	00000
	.07	.81612	.71776	•62259	.53240	.44862	.37235	.30428	.24473	.19367	.15075	.11539	.08683	•06422	.04667	.03333	.02337	01910	•01089	.00723	.00471	.00301	.00188	•00116	• 0000	.00040	.00023	•00012	90000	*0000	00000
	90•	.82604	.72749	16169*	.54115	.45668	.37962	.31071	.25030	.19840	.15469	11861	.08940	•06624	.04822	.03450	.02424	.01673	.01133	•00754	.00492	.00315	•00198	.00121	• 000 13	.00043	• 00054	•00013	• 00000	.00003	10000
	• 05	.83596	.13724	.64128	.54997	.46481	.38697	.31722	.25596	.20322	.15871	.12189	•09504	.06831	.04982	.03570	.02513	.01737	.01179	.00786	.00514	.00330	.00200	.00128	.00077	.00045	•00056	•00014	.00007	.00003	10000
	•0•	.84586	.74703	. 65069	.55884	.47301	.39440	.32381	.26170	.20811	.16280	.12525	.09473	.07043	.05146	•03694	•02605	.01804	.01227	• 00819	.00537	.00345	.00217	.00134	.00081	.00048	.00027	.00015	.00007	• 00003	.00001
	.03	.85571	.75684	.66015	.56778	.48129	16107	.33050	.26752	•21309	.16698	.12867	.09749	.07261	.05314	.03821	.02700	.01873	.01276	.00854	.00560	.00361	.00228	.00141	.00085	.00050	• 00059	91000*	.00008	• 0000	.00001
	•05	.86544	.76667	99699•	.57678	.48963	.40950	.33726	.27344	.21816	.17123	.13217	.10031	.07484	.05487	.03953	.02797	.01944	.01327	.00889	.00585	.00377	•00239	.00148	06000	.00053	.00030	.00017	•0000•	,0000	10000
	.01	. 87468	.77653	.67920	.58583	.49805	.41717	.34411	.27943	.22330	.17556	.13574	.10320	.07712	•05664	.04088	•02898	.02018	.01380	•00926	.00610	• 00395	.00250	.00155	*0000	• 00056	•00035	• 00018	60000	*0000	10000
,004	00•	.87915	.78641	.68879	. 59494	.50654	.42491	.35105	.28552	.22853	17996	.13938	.10614	•07946	.05846	.04227	•03002	•0209	.01435	.00965	.00637	.00413	•00262	.00163	66000*	•0000	•00034	•00019	•00010	•0000•	.00000
M= .004	×	•	.1	~	63	4.	°.	•	۲.	₩.	٥.	1.0	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2	2.3	2.4	2.5	2.6	2.7	2.8	2.9 3.0

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Table	13.	الم	e^{u-u^2}	du.
.79621	. 51508 . 43273 . 35806 . 29168 . 23385	.14310 .10916 .08186 .06033	.01691 .01491 .01005 .00665 .00431	.00171 .00105 .00005 .00036 .00001 .00005
.08 .80611 .70803	.51330 .44064 .36516 .29794 .23925	.14689 .11224 .08431 .06225	.02254 .01550 .01056 .00693 .00651	.00180 .00110 .00066 .00038 .000011 .00006
.07 .81601 .71772	.54239 .44862 .37235 .30428 .24473	.15075 .11539 .08683 .06422	.01610 .01610 .01089 .00723	.00188 .00116 .00069 .000023 .000023
.06 .82591 .72745	.54114 .54114 .37667 .31070 .25030	.15469 .11861 .08940 .06624 .04822	.02424 .02424 .01633 .00754 .00492	.00198 .00121 .00073 .00043 .000124 .00006
. 83579 . 73720	. 54460 . 544995 . 386496 . 31721 . 25596	.15871 .12189 .09204 .06831 .04982	. 02513 . 01737 . 01179 . 00786 . 00330	.00207 .00128 .00077 .00045 .00014 .00007
.04 .84565 .74698	. 55883 . 47300 . 39439 . 32381 . 26170	. 16280 . 12525 . 09473 . 07043	.02605 .018605 .01827 .00819 .00537	.00217 .00134 .00081 .00048 .000127 .00007 .00003
. 03 . 85543 . 75678	. 56777 . 56777 . 46128 . 33049 . 26752	. 16697 . 12867 . 09749 . 07261	. 02521 . 02700 . 01873 . 01276 . 00854 . 00560	.00228 .00141 .00085 .00050 .000129 .00004
.02 .86502 .76661	. 57676 . 57676 . 409462 . 33726 . 27343	.17122 .13217 .10031 .07484	. 02797 . 01944 . 01327 . 00889 . 00585	.00239 .00148 .00090 .00053 .00003 .00009
.01 .87385 .77646	.58581 .58581 .49804 .41716 .34411 .27943	.17555 .13574 .10320 .07712 .05664	. 02898 . 02018 . 01380 . 00926 . 00610	.00250 .00155 .00034 .00032 .00003 .00004
.00 .87739 .78633	.59492 .50492 .50653 .42491 .35104 .28551	.17996 .13938 .10614 .07946	.04624 .02094 .01435 .00637 .00637	.00262 .00163 .00099 .00099 .00010 .000059
% o::	, , , , , , , , , , , , , , , , , , ,		7	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0

Table 13 315

	•00	.79611	.69834	.60407	.51507	.43273	.35806	.29168	.23385	.18445	.14310	.10916	.08186	.06033	.04369	.03109	.02173	.01491	•01005	• 00665	.00431	.00274	.00171	•00100	*0000	• 00036	.00000	.00011	.00005	• 00002	00000
	•08	.80599	. 70799	.61328	. 52369	.44063	.36516	.29793	.23924	.18902	.14689	.11224	.08431	.06225	.04516	.03219	.02254	.01550	.01046	.00693	.00451	.00287	.00180	.00110	99000•	.00038	.00021	.00011	90000	• 00005	00000*
	.07	.81587	.71768	.62255	.53237	.44861	.37234	.30427	.24473	.19367	.15075	.11539	.08683	.06422	.04667	.03333	.02337	01910	.01089	.00723	.00471	.00301	•00188	•00116	69000	.00040	.00023	• 00012	90000	• 00005	00000•
	•00	.82574	.72740	.63187	.54112	.45666	.37961	.31070	.25030	.19840	.15469	.11861	.08940	•06624	.04822	.03450	.02424	.01673	.01133	-00754	.00492	•00315	.00198	.00121	• 00073	.00043	•00054	.00013	90000	.00003	•00001
	• 05	.83559	.73714	.64123	.54994	.46479	.38695	.31721	.25595	.20321	.15871	.12189	.09204	.06831	.04982	.03570	.02513	.01737	.01179	•00786	.00514	.00330	.00207	.00128	.00077	.00045	•00056	•00014	. 00007	.00003	10000
	•0•	.84539	.74692	.65064	.55881	.47299	.39438	.32381	.26169	.20811	.16280	.12525	.09473	.07043	.05146	.03694	.02605	.01804	.01227	.00819	.00537	.00345	-00217	.00134	.00081	.00048	.00027	.00015	.00007	.00003	.00001
	•03	.85509	.75672	01099	.56775	.48127	.40189	.33049	.26752	.21309	.16697	.12867	.09749	.07261	.05314	.03821	.02700	.01873	.01276	.00854	.00560	.00361	•00228	.00141	.00085	.00050	•00059	• 00016	00000	,0000	.00001
	•02	.86450	.76654	09699•	. 57674	.48961	. 40948	.33725	.27343	.21815	.17122	.13217	10001	-07484	.05487	.03953	.02797	.01944	.01327	*00889	.00588	.00377	.00239	.00148	06000.	.00053	• 00030	.00017	60000	•0000•	.00001
	.01	.87287	.77638	.67914	.58579	.49803	.41715	.34410	.27943	.22330	.17555	.13574	.10319	.07712	•05664	.04088	.02898	.02018	•01380	•00926	01900	• 00395	.00250	.00155	* 6000 *	• 00056	• 00032	•00018	•0000	,0000	.00001
• 000	00•	.87564	.78624	.68872	. 59490	.50651	•42490	.35104	.28551	.22853	17996	.13938	10614	•07946	.05846	.04227	*03002	*0204	.01435	• 00965	.00637	.00413	•00262	.00163	66000*	•0000	•00034	•00019	.00010	• 00005	• 00000
1	ĸ	•	٠,	•2	m.	4.	٠.	9•	٠.	₩,	6.	0.1	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2	2•3	2.4	2.5	5.6	2.7	2.8	8°8 3°0

	٦	a	bl	е	1:	3									6	- (ኢ-		U	2	dl	L.	,									
	60°	.79599	.69829	•0000	.51505	.43272	.35805	.29168	.23384	.18445	.14309	10916	.08186	€6090.	₹ 69840.	.03109	.02173	.01491	.01005	.00665	.00431	.00274	.00171	• 00102	. 00062	•00036	.00020	.00011	• 00005	• 00005	00000	
	90•	.80585	. 70794	.61325	.52367	.44062	.36515	.29793	.23924	.18902	.14688	.11224	.08431	•06225	.04516	•03219	.02254	.01550	.01046	.00693	.00451	.00287	• 00180	.00110	99000	•00038	.00021	.00011	90000•	• 00005	00000	
	.07	.81571	.71762	.62252	.53236	.44860	.37233	.30427	.24473	.19367	.15075	.11539	.08683	.06422	.04667	.03333	.02337	01910	•01089	.00723	.00471	.00301	.00188	•00116	69000•	.00040	.00023	.00012	90000	20000	00000	
	•00	.82555	.72734	.63183	.54111	.45665	.37960	.31069	.25030	.19840	.15469	.11860	.08940	.06624	.04822	.03450	.02424	.01673	.01133	.00754	*00492	.00315	.00198	.00121	.00073	.00043	•00054	.00013	90000	•0000	10000	
	•05	.83536	.73708	.64120	.54992	.46478	.38695	.31721	.25595	.20321	.15871	.12189	•09504	.06831	.04982	.03570	.02513	.01737	•01179	.00786	.00514	.00330	.00207	.00128	.0007	.00045	•00056	.0001	-0000	.00003	.00001	
	•04	.84509	.74685	.65061	.55879	.47298	.39438	.32380	.26169	.20811	.16280	.12524	.09473	.07043	.05146	.03694	• 02605	.01804	.01227	• 00819	.00537	.00345	.00217	.00134	.00081	.00048	.00027	.00015	10000	• 00003	.00001	
	.03	.85468	. 75664	90099•	.56773	.48125	.40189	.33048	.26752	.21309	.16697	.12867	.09749	.07260	.05314	.03821	.02700	.01873	.01276	.00854	.00560	.00361	.00228	.00141	• 00085	.00050	• 00059	• 00016	• 00008	*0000	10000	
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	• 05	.83508	.73700	.64116	.54990	.46477	.38694	.31720	.25595	.20321	.15870	.12189	.09204	.06831	.04982	.03570	.02513	.01737	.01179	•00786	.00514	.00330	.00207	.00128	.0007	.00045	• 00056	•00014	• 00007	• 00003	.00001	
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M= .010	ĸ	•	٦.	~	•3	4.	٠,	9.	۲.	Φ.	6.	1.0	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2	2.3	2.4	2.5	5.6	2.7	2.8	5.9	3.0

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	.07	.73784	.68513	.60528	.52211	.44215	.36815	.30151	.24290	.19245	.14994	.11485	.08648	•06399	.04653	.03323	.02331	•01606	.01087	•00722	.00470	.00300	•00188	.00115	69000*	.00040	.00023	-00012	90000	• 00002	00000
	•0•	.73930	.69235	.61359	.53035	16675	.37524	.30782	.24839	.19713	.15385	.11805	•0890	00990•	.04807	.03440	.02418	•0100	.01131	.00752	.00491	.00314	.00197	.00121	.00073	.00043	.00024	.00013	•0000•	.00003	10000*
	• 05	.74000	.69932	.62186	.53862	.45773	.38240	.31421	.25396	.20189	.15783	.12131	• 09165	• 06806	99670*	.03560	•02506	•01733	.01177	.00784	.00513	•00329	.00200	.00127	.00077	.00045	• 00056	• 00014	.0000	• 00003	10000
	•0•	.74020	.70598	• 63009	.54691	.46561	.38963	.32068	.25962	.20673	.16189	.12464	.09433	.07017	.05129	.03683	.02598	.01800	.01224	.00817	•00536	.00344	.00217	.00134	.00081	.00048	.00027	• 00015	10000	.00003	.00001
	• 03	.74022	.71227	.63826	. 55523	.47353	.39693	. 32723	.26536	.21165	.16602	.12804	.09707	.07233	.05296	.03810	• 02692	•01869	.01273	.00852	• 00559	• 00360	.00227	.00141	.00085	.00050	• 00059	.00016	• 00008	,0000	.00001
	• 05	.74022	.71813	.64636	.56356	.48152	.40430	.33386	.27118	.21666	.17023	13151	98660.	.07455	.05468	.03941	.02790	.01940	.01324	.00888	.00584	.00377	.00238	.00148	06000	• 00053	• 00030	.00017	•0000	•0000•	.00001
	•01	.74022	.72347	.65437	.57190	.48955	.41174	.34056	.27709	.22174	.17452	.13505	.10274	.07682	.05645	.04075	.02890	.02013	.01377	.00925	•0900•	.00394	•00250	•00155	*6000	•00026	•00032	•00018	•0000	*0000	.00001
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.08 .72247 .67099 .59322		.01545 .01043 .006692 .00179 .000179 .00038 .00001 .00006
.07 .72416 .67798 .60134	.44065 .36718 .36718 .19216 .14975 .11473 .06394 .0649	.01605 .01086 .001721 .00721 .00300 .00188 .00015 .00012 .00002
.06 .72510 .68472 .60943	. 34422 . 34424 . 34424 . 196834 . 11792 . 06895 . 06895 . 03437	.01668 .01130 .00752 .00491 .00121 .00073 .00013 .00006
.05 .72548 .69116 .61746		.01732 .01176 .00784 .00513 .00507 .00045 .000046 .00007
.04 .72556 .69724 .62543		.01799 .01224 .01224 .00535 .00217 .000134 .000048 .000015
.03 .72556 .70291 .63333	.32647 .32648 .32647 .26686 .16579 .07227 .05292	.01868 .001273 .00851 .00185 .00181 .00185 .000185 .00018
.02 .72556 .70810 .64113	. 47964 . 40309 . 23307 . 21631 . 131000 . 13135 . 07448 . 05464 . 05464	.01939 .01324 .01324 .00584 .00238 .000148 .000148 .00017
.01 .72556 .71273 .64862 .56871	.48758 .41048 .27654 .22138 .17427 .10263 .07675 .05640	.02012 .01376 .00394 .00609 .00396 .000155 .000156 .00018
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	80	.39819	.39809	.39288	.37237	.33767	.29518	.25040	-20702	.16726	.13228	.10251	.07789	.05805	.04245	.03046	.02144	.01481	.01004	.00668	.00436	.00279	•00175	.00107	*9000*	-00037	.00021	•00011	• 00005	• 00002	00000
	.07	.39819	.39814	.39400	.37515	.34160	.29961	.25486	.21123	.17104	.13555	.10525	.08013	.05983	.04383	.03151	.02223	.01539	.01045	96900*	.00455	.00292	.00183	.00112	.00068	•0003	•00022	•00012	90000	• 00002	00000
	90•	.39819	.39817	.39496	.37778	.34544	.30401	.25934	.21546	.17486	.13887	• 10805	.08242	•06166	•04526	.03260	.02304	.01598	.01087	.00726	.00475	• 00305	.00192	.00118	.00071	.00042	•00054	.00013	90000	.00003	.00001
	• 05	.39819	.39818	.39576	.38026	.34919	.30838	.26382	.21973	.17873	.14224	.11089	.08475	.06353	.04672	.03371	.02387	.01658	.01131	• 00756	• 00496	• 00319	.00201	•00124	• 00075	*000*	• 00025	• 00014	-00007	.00003	.00001
	•0•	.39819	.39819	.39641	.38258	.35285	.31272	.26831	.22404	.18264	.14566	.11379	.08714	.06545	.04822	.03486	.02473	.01721	.01176	.00788	.00518	.00334	.00211	.00130	• 000 4	• 00046	.00027	.00014	.00007	.00003	.00001
	•03	.39819	.39819	.39694	.38472	.35640	.31701	.27281	.22837	.18660	.14914	.11674	.08958	.06740	.04976	•03604	.02562	.01786	.01223	.00821	.00541	.00349	.00221	.00137	.00083	• 00049	• 00028	• 00015	• 00008	,0000	.0000
	• 02	.39819	.39819	.39734	.38670	.35985	.32126	.27730	.23273	.19060	.15266	.11974	• 09206	.06941	.05134	.03725	.02653	.01854	.01271	.00855	.00564	• 00365	•00232	•00144	.00087	•00052	• 00030	• 00016	• 00008	•0000•	.00001
	•01	.39819	.39819	.39764	.38851	.36317	.32546	.28179	.23712	.19465	.15624	.12280	09460	.07146	.05295	.03850	.02747	.01923	.01321	06800*	•00589	.00382	.00243	.00151	• 00092	•00055	.00031	.00017	60000	•0000•	.00001
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60•	.32601	.32436	.31398	.29149	.25999	• 55409	.18756	.15297	.12186	₹ 96760.	₹ 94210.	.05417	.03971	.02854	.02012	.01391	.00943	.00627	.00409	.00261	.00164	.00100	09000	.00035	•00019	.00010	• 00005	*0000	00000	
• 08	.32601	.32479	.31558	.29422	.26341	.22776	.19116	.15629	.12479	•09745	.07451	.05582	.04100	.02952	.02085	.01444	.00981	•00654	.00427	.00274	.00172	.00105	• 9000	.00037	.00021	.0001	• 00005	-00005	00000	
.07	.32601	.32513	.31705	.29685	.26679	.23142	.19478	.15965	.12776	86660*	.07661	.05751	.04232	.03053	.02161	.01500	.01021	.00682	•00446	• 00286	.00180	.00111	19000	•0003	*00052	•00012	• 00000	*0000	00000	
90•	.32601	.32539	.31839	• 29939	.27011	-23507	.19841	.16304	.13077	.10257	.07875	.05924	.04368	.03158	.02239	.01557	•01062	•00710	• 00466	• 00300	•00189	.00116	.00070	.00041	.00023	.00013	90000*	.00003	.0000	
.05	.32601	.32559	.31960	.30182	.27337	.23871	•20206	.16646	.13383	•10519	.08093	.06100	.04507	•03265	•02319	•01616	.01104	.00740	.00487	.00314	.00198	.00122	. 00074	.00043	• 00025	.00013	10000	.00003	10000	
40.	.32601	.32574	.32069	.30414	.27657	.24233	.20571	.16991	.13692	.10786	.08316	.06281	.04649	.03374	.02402	.01677	.01148	.00771	.00508	.00328	.00200	.00128	.00078	• 00046	.00026	.00014	20000	.00003	.00001	
.03	.32601	.32584	.32165	.30635	.27971	.24592	.20938	.17339	.14005	.11057	.08543	•06466	• 04796	.03487	.02487	.01740	.01193	.00803	.00530	.00343	.00217	.00135	.00082	.00048	.00028	.00015	• 00008	.00003	.00001	
-02	.32601	.32591	.32249	.30844	.28277	.24949	.21306	.17689	.14323	.11333	.08775	.06654	•04946	•03604	.02575	.01804	.01240	•00836	.00553	•00358	.00228	.00142	• 00086	.00051	•00059	•00016	*0000	*0000*	.00001	
.01	.32601	.32596	.32322	.31041	.28576	.25303	.21673	.18042	.14644	.11613	.09011	.06847	.05099	.03723	.02665	.01871	.01289	.00871	.00577	.00375	•00239	.00149	.00091	.00054	.00031	.00017	•0000•	*0000*	.00001	
00	.32601	.32598	.32384	.31226	.28867	.25653	.22041	.18398	.14969	.11897	.09251	•0704	.05256	.03845	.02758	.01940	.01339	90600*	• 00602	.00392	.00250	.00156	\$6000.	.00057	.00033	.00018	.00010	.00005	*0000	00000
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	60.	.26691	.26691	.26657	.26238	.24955	.22795	.20053	.17068	.14110	.11365	•08936	69890.	.05168	.03808	.02749	.01945	.01349	.00917	.00612	.00400	.00256	.00161	86000.	• 0000	.00034	•1000•	01000.	.00000	.00002	00000	
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	.07	.26691	.26691	.26676	.26381	.25288	.23285	.20631	.17670	.14690	.11891	•09394	.07253	.05480	.04054	.02939	.02087	.01454	26600*	• 00664	.00436	.00280	.00177	•00100•	• 00065	€000	•00022	.00012	90000	-00005	00000	
	90•	.26691	.26691	.26682	.26440	.25441	.23520	•20916	.17971	.14982	.12159	.09628	.07451	.05641	.04182	.03037	.02162	.01509	.01032	26900*	.00455	.00293	.00185	.00114	69000*	.00040	.00023	.00012	90000	.00003	.00001	
	• 05	.26691	.26691	.26685	.26490	.25584	.23749	.21197	.18272	.15276	.12429	99860.	.07652	•05806	.04313	•03139	.02239	.01565	.01073	.00721	.00475	.00307	.00194	.00120	•00073	.00043	•00054	.00013	.00007	.00003	.00001	
	•0•	.26691	.26691	.26688	.26533	.25717	.23970	.21475	.18572	.15572	.12703	10101	•07856	. 05974	.04447	.03243	.02318	.01624	.01115	.00751	• 00496	.00321	.00203	.00126	• 00076	.00045	•00056	• 1000	.0000	• 00003	.00001	
	•03	.26691	.26691	•26689	.26570	.25840	.24183	.21748	.18871	.15869	.12979	.10352	.08065	.06146	.04585	.03350	•02399	.01684	•01159	.00782	.00517	• 00335	.00213	.00132	08000	.00048	.00027	.00015	• 00008	• 00003	.00001	
	•02	.26691	.26691	.26690	.26599	.25954	.24389	.22018	.19169	.16167	.13258	.10600	.08277	.06321	.04726	.03460	.02483	.01746	.01204	.00814	•00539	.00350	.00223	.00139	.00085	.00050	•00059	• 00016	.00008	,0000	10000	
	.01	.26691	.26691	.26690	.26624	.26058	.24586	.22282	.19466	.16467	.13540	.10852	.08493	.06500	.04870	.03573	•02569	.01810	.01251	.00847	.00563	•00366	.00234	.00146	• 0008	.00053	.00031	.00017	•0000•	•0000•	.00001	
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B 32	×	0.	٠,	•5	۴,	4.	٠. در	9.	٠.	₩.	6.	1.0	1:1	1.2	1.3	1.4	1.5	1.6	1.7	. 1.8	1.9	2.0	2.1	2.2	2.3	5.4	2.5	5.6	2.7	2.8	5.9	3.0

Ta	ble	13.											r.	ē	·u	; —	U	2	d	U.	,					
.21852	.21847	.21034	.17652	.15300	.10473	.08321	.06452	.04889	.03624	.02630	.01869	10210	.00888	•00294	.00389	.00250	.00157	96000.	• 00058	-00034	61000°	.00010	• 00002	*0000	00000	
.21852	.21849 .21731	.21133	.17873	.15543 .13086	.10702	.08524	• 06625	.05031	.03738	.02718	• 01636	01320	• 00923	• 00619	• 00400	.00261	.00164	.00101	.00061	• 00036	.00020	.0001	• 00005	-00005	00000	
.21852	.21850 .21850 .21757	.21224	.18089	.15786	.10933	.08730	.06801	.05177	.03854	•02808	•0200	.01401	09600*	*00644	*00424	.00273	•00172	• 00100	• 00064	• 00038	.00021	.00011	90000	• 00005	00000	
.21852	.21852 .21851 .21779	21307	.18301	.16027	.11166	•08839	.06981	.05326	•03973	20620	.02075	.01454	86600.	.00671	•00442	.00286	.00181	.00112	89000	.00040	.00023	• 00012	90000	• 00003	.00001	
.21852	.21852 .21852 .21798	.21384	.18509	.16267	.11401	.09151	.07163	.05477	• 04096	.02997	.02148	.01508	•01037	66900*	.00462	•00599	•00189	.00117	.00071	.00042	•00024	.00013	10000	• 00003	.00001	
.21852	.21852 .21852 .21812	.21453	.18712	.16504	.11637	.09365	.07349	.05632	.04220	•03095	.02223	.01563	.01077	.00728	.00482	.00312	• 00198	.00123	.00075	• 00044	.00025	• 00014	.0000	.00003	.00001	
.21852	.21852 .21852 .21823	.21515	.18910	.16739	.11875	.09582	.07537	.05790	.04348	.03196	•05299	.01621	.01119	.00758	.00502	•00326	•00208	•00129	• 0000	.00047	.00027	.00015	• 00008	•0000	.00001	
.21852	.21852 .21852 .21832	.21570	.19103	.16972	.12115	.09801	.07729	.05951	.04479	•03299	.02379	.01680	.01163	.00788	• 00524	.00341	.00218	• 00136	• 00083	• 00049	•00028	.00016	• 00008	• 00004	.00001	
.21852	.21852 .21852 .21839	.21619	16261	.17202	.12356	.10023	.07923	.06115	.04613	.03404	.02460	-01741	.01207	.00820	•00546	.00356	.00228	.00143	.00087	•00052	.00030	.00017	60000	,0000	.00001	
.21852	.21852 .21852 .21843	.21662	.19473	.17429	.12598	.10247	.08121	.06282	.04749	.03513	.02544	.01804	.01254	.00854	.00570	.00372	.00238	.00150	.00092	•00055	.00032	.00018	60000	,0000	•00005	• 00000
% o.∙	1.5.	u	. •	۲. 8	6.	1.0	1.1	1.2	1.3	1.4	1.5	9•1	1.7	1.8	1.9	2.0	2.1	2.2	2.3	2.4	2.5	2.6	2.7	2.8	5.9	3.0

	60.	17891	17891	.17890	.17844	.17530	.16697	.15313	.13524	.11539	.09541	.07668	*0090*	.04587	.03423	.02499	.01785	.01248	.00855	.00573	.00377	.00242	.00153	*6000*	• 00026	.00033	* 00018	.00010	• 00005	-00005	00000	
	.08	.17891	.17891	.17890	.17856	.17582	.16807	.15472	.13715	.11740	.09737	.07847	•06159	.04717	•03529	.02581	.01848	.01295	• 00889	.00597	.00393	•00254	•00160	66000*	• 00029	•00035	• 000020	.00010	• 00005	*0000	00000	
	.07	.17891	.17891	.17891	.17865	.17629	.16911	.15628	.13904	.11942	•09934	.08028	.06318	.04850	•03636	•02666	*01912	.01343	•00924	• 00622	• 00410	• 00265	•00168	.00104	.00063	.00037	.00021	.00011	90000	*0000	00000	
	• 00	.17891	.17891	.17891	.17872	.17671	.17008	.15779	.14091	.12143	.10132	.08211	.06478	•04985	.03746	.02753	•01979	.01393	09600	.00648	.00428	.00277	• 00176	60100	99000	.00039	*00052	.00012	• 0000	20000	.00001	
	• 05	.17891	.17891	.17891	.17878	.17708	.17100	.15926	.14275	.12344	.10332	• 08396	.06641	.05123	.03859	.02842	.02047	.01444	16600°	. 00674	.00447	.00290	.00184	.00114	69000	.00041	.00023	.00013	• 00000	.00003	.00001	
	•04	.17891	17891	.17891	.17882	.17740	.17187	.16067	.14456	.12543	.10531	.08583	.06807	.05263	.03974	.02933	.02118	.01497	.01036	.00702	.00466	• 00303	.00193	.00120	.00073	.00043	• 00025	•00014	.00007	.00003	.00001	
	•03	.17891	.17891	.17891	.17885	.17768	.17267	.16204	.14634	.12742	.10732	.08771	*1690	• 05406	.04091	•03026	.02190	.01551	.01075	.00730	.00486	.00317	.00202	•00126	.00077	• 00046	• 00026	.00014	.00007	.00003	.00001	
	•02	.17891	.17891	.17891	.17887	.17793	.17341	.16335	.14809	.12940	.10933	.08961	•07145	•05552	.04211	.03122	.02264	.01607	.01117	.00760	.00507	.00331	.00212	.00132	.00081	.00048	•00028	.00015	00000	*0000	.00001	
	.01	.17891	.17891	17891	.17888	.17813	.17410	.16462	.14981	.13136	.11135	.09153	.07317	.05700	.04334	.03220	.02340	.01664	•01159	.00791	.00528	•00346	.00221	•00139	.00085	.00051	•00059	•00016	•0000	*0000	10000	
800	00.	.17891	17891	.17891	.17889	.17830	.17473	.16582	.15149	.13331	.11337	•09346	.07492	.05851	.04459	•03321	.02419	.01724	.01203	.00822	.00550	.00361	.00232	.00146	•00089	.00054	.00031	.00017	60000	*0000	*0000	00000
M= .800	×	•		•2	٠,	4.	٠.	9.	٠.	۰.	6.	1.0	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2	2.3	2.4	2.5	5.6	2.7	2.8	5.9	3.0

Ī	M=1.000									
×	00•	.01	•02	.03	•	• 05	•00	.07	.08	60.
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٦.	.11992	11992	11992	.11992	.11992	.11992	.11992	11992	11992	•11992
7.	.11992	.11992	.11992	.11992	.11992	11992	.11992	11992	.11992	-11992
۴,	11992	.11992	11992	.11992	-11992	.11992	16611.	.11991	.11990	.11989
4.	.11988	11986	.11983	.11980	.11976	.11970	.11964	.11956	11946	.11935
'n.	.11922	.11907	.11889	.11869	.11846	.11820	.11792	.11760	.11725	.11687
9.	.11645	• 11600	.11551	.11499	.11443	.11383	.11320	.11253	.11183	.11108
۲.	11031	• 10949	.10864	.10776	•10685	.10590	.10492	.10391	.10287	.10181
φ.	.10072	09660*	• 09846	.09729	.09611	.09490	.09367	.09243	.09117	.08990
6.	.08861	.08731	.08600	.08468	.08335	.09201	.08067	.07932	.07797	.07662
0.1	.07527	•07391	.07256	.07121	98690.	•06852	•06718	•06585	.06453	.06321
1.1	.06190	09090•	•05931	.05803	.05676	•05550	.05426	.05303	.05181	.05060
1.2	.04941	.04824	.04708	.04593	.04480	.04369	.04259	.04151	•04045	.03940
1.3	.03837	.03736	.03636	•03239	.03443	.03348	.03256	•03165	•03076	.02989
1.4	•0530	•02820	*02738	.02658	.02579	.02502	.02427	.02354	.02282	.02212
1.5	.02144	.02077	•02012	.01948	.01886	.01826	.01767	.01710	.01654	•01200
1.6	.01546	•01495	.01444	•01395	.01348	.01302	.01257	.01213	.01171	•01130
1.7	•01090	.01051	•01010	.00977	*00945	80600*	• 00874	.00842	.00811	.00781
1. 8	•00752	•00723	96900.	69900.	*00644	• 1900•	• 00595	•00572	.00550	.00528
1.9	.00507	.00487	.00467	• 00449	.00431	.00413	•00396	•00380	• 00 364	.00349
2.0	.00335	.00321	.00307	•00294	•00282	.00270	•00258	.00247	•00236	.00226
2.1	•00216	•00207	•00198	.00189	.00180	.00172	•00165	.00157	.00150	.00143
2.2	.00137	.00130	•00124	•00118	.00113	•00108	.00103	86000	.00093	.00089
2.3	.00084	00000	92000.	.00073	69000	99000.	• 00062	•00029	• 00056	.00053
2.4	.00091	9,000	94000	.00043	.00041	•0003	.00037	• 00035	.00033	.00031
2.5	•00030	•00028	• 00056	• 00025	• 00054	•00052	.00021	•00050	61000°	. 00018
5.6	.00017	91000*	•00015	.00014	• 00013	.00012	•00011	•00011	.00010	60000
2.7	60000*	90000	*0000	.00007	.00007	90000	• 0000	• 00005	• 00005	-00005
2.8	*0000	• 0000 •	• 00003	• 00003	.00003	• 00003	• 00005	20000	•00005	.0000
2.9	10000	.00001	•00001	.00001	.00001	.00001	00000	00000	00000	00000
3 ° 0	00000									

Table 14

Values of:

$$\int_{x}^{\infty} \frac{e^{-u^2 - \frac{m^2}{u^2}}}{u} \frac{du}{u}$$

This table was prepared by Simpson's rule integration. Since this is an approximate numerical process it is desirable to have some means of evaluating the degree of approximation obtained.

When $(m^2/u^2) \ll 1$ the integral reduces approximately to

$$\int_{x}^{\infty} \frac{e^{-u^2} du}{u}$$

The substitution $v = u^2$ will reduce this to a form of the Exponential Integral which is a tabulated function. Then

$$\int_{v}^{\infty} \frac{e^{-u^{2}} du}{u} = \frac{1}{2} \int_{v^{2}}^{\infty} \frac{e^{-v} dv}{v} = \frac{1}{2} E_{1}(x^{2})$$

When m = 0.001 x = 1.0 $(m^2/u^2) < 0.000001$ $e^{-\frac{m^2}{u^2}} > 0.999999$

 $\frac{1}{2} E_1(x)^2 = 0.1096919.$ The tabular value is 0.10969. The check is satisfactory when m = 0.001 x = 0.1 $(m^2/u^2) < 0.0001$ $e^{-\frac{m^2}{u^2}} > = 0.999900$

 $\frac{1}{2}E_1(x^2) = 2.0189648$. The tabular value is 2.01891. The check is good to the fourth decimal place which is all that could be expected of the approximation.

Another check can be obtained from the integral:

$$\int_{0}^{\infty} \frac{-u^{2} - \frac{m^{2}}{u^{2}}}{u} = K_{0} (2m). \quad \text{If } m > 0.$$

A comparison of values computed by Simpson's rule with values of the above integral is shown below:

m	K ₀ (2m)	Values computed by Simpson's rule
0.001	6.33055	6.42824
0.002	5.63742	5.61213
0.003	5.23198	5.23290
0.004	4.94434	4.94644
0.005	4.72124	4.72140
0.006	4.53898	4.53863
0.007	4.38489	4.38476
0.008	4.25143	4.25146
0.009	4.13373	4.13377
0.010	4.02846	4.02846
0.020	3.33654	3.33654
0.030	2.93288	2.93287
0.040	2.64749	2.64748
0.050	2.42707	2.42706
0.060	2.24786	2.24785
0.070	2.09717	2.09716
0.080	1.96742	1.96741
0.090	1.85371	1.85371
0.100	1.75270	1.75270

Agreement is observed for the m = 0.010 and larger values of m . In this range the table can be accepted since the integration was carried backward from x = 3.0. The Simpson's rule procedure makes every value dependent upon the preceding one and a check of the value for x = 0 checks the entire table for that value of m .

Discrepancies are noted in the range m=0.001 to m=0.009. In this range the integrand drops sharply toward zero as x approaches zero and the shape of the curve may here be ill adapted to evaluation by the Simpson's rule procedure. The checks obtained for x=1.0 and x=0.1, when

m = 0.001, lend support to this presumption. To evaluate the integral in the area near zero make use of the relations

$$e^{-u^2} = 1 - u^2 + \frac{u^4}{2!} - \frac{u^6}{3!} + \dots$$

and

$$\int_{x}^{\infty} \frac{e^{-u^{2} - \frac{m^{2}}{u^{2}}}}{u} = \int_{0}^{\infty} \frac{e^{-u^{2} - \frac{m^{2}}{u^{2}}}}{u} - \int_{0}^{x} \frac{e^{-u^{2} - \frac{m^{2}}{u^{2}}}}{u} du$$

Then the last integral above can be expressed approximately as

$$\int_{0}^{x} \frac{e^{-u^{2}} - \frac{m^{2}}{u^{2}}}{u} du = \int_{0}^{x} \frac{e^{-\frac{m^{2}}{u^{2}}} du}{u} - \int_{0}^{x} \frac{u^{2} e^{-\frac{m^{2}}{u^{2}}} du}{u} + \frac{1}{2} \int_{0}^{x} \frac{u^{4} e^{-\frac{m^{2}}{u^{2}}} du}{u} - \dots$$

Let $\frac{m}{u} = v$ $u = \frac{m}{v}$ $du = \frac{-mdv}{v^2}$ $\frac{1}{u} = \frac{v}{m}$. Then, by substitution

$$\int_{0}^{x} \frac{e^{-\frac{m^{2}}{u^{2}}} du}{u} = \int_{(\frac{m}{x})}^{\infty} \frac{e^{-v^{2}} dv}{v}$$

By similar substitutions and integration by parts the remaining two integrals can be evaluated and the integral approximated as

$$\int_{0}^{x} \frac{e^{-u^{2} - \frac{m^{2}}{u^{2}}}}{u} \frac{du}{u} = \left(-\frac{x^{2}}{2} + \frac{x^{4}}{8} - \frac{m^{2}x^{2}}{8}\right) e^{-\left(\frac{m^{2}}{x^{2}}\right)} + \left(1 + m^{2} + \frac{m^{4}}{4}\right) \int_{\left(\frac{m}{x}\right)}^{\infty} \frac{e^{-v^{2}}}{v} dv$$

(Approximate if $m \ll 1 \times \ll 1$)

As noted previously, the integral which appears here can be expressed in terms of the exponential integral as

$$\int_{\left(\frac{m}{x}\right)}^{\infty} \frac{e^{-v^2} dv}{v} = \frac{1}{2} \int_{\left(\frac{m^2}{x}\right)}^{\infty} \frac{e^{-u} du}{u} = -\frac{1}{2} \operatorname{Ei} \left(-\frac{m^2}{x^2}\right) = \frac{1}{2} \operatorname{E}_1\left(\frac{m^2}{x^2}\right) .$$

The results of computations made by this approximation are

m	x = 0.01	x = 0.02
0.001	4.31163	3.62237
0.002	4.29683	

Since the approximation values for m = .001 x = 0.02 and for m = 0.002 x = 0.01 agree well with the tabular values the presumption is confirmed. The tabular values affected by this difficulty have been replaced by computed values.

Tal	ole	14	ŀ.						_	ē '	u- -	u	u	2	d	u	-										
.09 2.12331 1.38999	. 99044	.53795	.39902	29491	.15753	.11342	.08071	.05670	.03931	.02686	.01809	.01200	.00783	.00503	.00318	.00198	.00121	.00072	.00043	•00054	*1000	.0000	• 0000 •	-00005	.00001	00000	
.08 2.24023 1.44224	1.02279	. 55424	.41116	.30405	.16269	.11727	.08355	.05878	.04080	.02792	.01883	.01251	.00818	.00527	.00333	• 00208	.00127	• 00016	.00045	• 00056	.00015	.00008	• 00004	*0000	.00001	00000	
.07 2.37299 1.49767	1.05651	.57105	. 42366	970EC	.16801	.12123	.08648	•06092	.04234	*0530	.01960	.01304	.00854	.00551	• 00349	.00218	.00133	.00080	.00047	.00027	• 00015	• 00008	• 0000	*0000	.00001	00000	
.06 2.52646 1.55667	1.09169	.58837	.43652	01626.	.17348	.12532	.08950	.06313	•04394	•03016	.02040	•01359	• 00892	• 00576	•00366	.00228	.00140	.00085	.00050	• 00059	• 00016	60000	.00005	20000	10000	00000	
.05 2.70817 1.61967	1-12844	.60624	.44976	• 55504 • 58504	.17911	.12952	.09261	.06541	.04559	.03134	.02123	.01417	.00931	•00602	.00383	.00240	.00147	• 00089	.00053	.00031	.00017	.00010	.00005	*0000	.00001	00000	
.04 2.93075 1.68723	1.16689	.62467	.46340	2526	.18491	.13386	.09582	•06776	•04729	.03255	•0220•	•01476	• 00972	• 00629	.00401	.00251	.00155	*6000*	•00056	• 00032	.00018	.00010	• 00005	.00003	10000	00000	
.03 3.21784 1.75999	1.20716	.64370	.47743	.35379	19081	.13832	.09913	•07019	.04905	.03381	.02297	.01538	.01014	•00658	.00420	•00264	.00163	66000•	• 00029	•00034	• 00019	.0001	90000	.00003	.00001	00000	
.02 3.62236 1.83879	1.24942	.66334	.49189	.36462	.19701	.14291	.10254	.07270	.05087	.03512	.02390	.01602	.01058	.00687	.00439	•00276	.00171	.00104	• 00062	96000.	.00021	11000.	90000*	.00003	.00001	00000	
.01 4.31163 1.92465	1.29383	.68363	. 50679	.37576	20332	•14764	.10606	.07529	.05276	.03647	.02485	•01668	.01103	.00718	.00460	• 00290	•00179	•00100	• 0000	•0003	• 00022	.00012	-0000	.00003	.00001	00000	
.00 6.33055 2.01891	1,34061	.70459	.52213	28722	20982	.15251	.10969	.07795	.05470	.03786	.02584	.01737	.01151	.00750	.00481	.00304	.00188	.00115	69000*	.00040	•00053	.00013	-0000	,0000	*0000	.00001	00000
×°.	7 5	14	ي	۰۰	- œ	٥.	1.0	1. 1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2	2.3	2.4	2.5	2.6	2.7	2.8	2.9	3.0

Ma .001

Table 14. 343

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90•	2-23662	1.44159	1.02256	.74853	.55419	.41113	.30403	.22337	• 16269	.11726	.08355	.05878	.04080	.02792	.01883	.01251	.00818	.00527	• 00333	.00208	.00127	• 00076	.00045	• 00056	• 00015	*0000	•0000•	• 00005	.00001	00000	
.07	2.36825	1.49694	1.05625	.77170	.57099	.42362	.31342	.23045	.16800	.12123	.08647	*06092	.04234	•02902	.01960	.01304	.00854	.00551	.00349	.00218	.00133	• 00000	.00047	.00027	• 00015	.0000R	,0000	*0000	.00001	00000	
90•	2.51996	1.55583	1.09141	.79567	.58831	.43648	*32308	.23773	.17347	.12531	•08949	.06312	*04394	.03016	.02040	.01359	.00892	•00576	•00366	.00228	• 00140	.00085	.00050	•00059	•00016	•0000•	.00005	*0000	10000	00000	
• 05	2.69877	1.61871	1.12814	.82050	.60617	.44972	.33302	.24522	.17910	.12952	.09261	.06541	.04559	.03133	.02123	.01417	.00931	*0000	.00383	•00240	.00147	• 00089	•00053	.00031	-00017	•00010	• 00005	*0000	.00001	00000	
*0	2.91603	1.68611	1.16655	.84622	.62460	.46336	.34325	.25293	.18490	.13385	.09582	0.06776	.04729	.03255	.02209	.01476	.00971	• 00629	.00401	.00251	.00155	*6000	• 00026	.00032	.00018	.00010	• 00005	• 00003	10000	00000	
.03	3.19172	1.75868	1.20679	.87289	.64362	.47739	.35377	.26085	.19086	.13831	.09913	.07019	.04905	.03381	.02297	.01538	.01014	.00658	.00420	.00264	.00163	66000.	• 00059	•00034	• 1000•	.00011	90000•	• 00003	.00001	00000	
202	3.56439	1.83724	1.24900	.90057	.66326	.49185	.36460	.26901	.19700	.14291	.10254	.07270	.05087	.03512	.02390	.01602	.01058	.00687	.00439	.00276	.00171	•00104	•00062	•00036	.00021	.00011	• 0000	.00003	.00001	00000	
10.	4.09886	1.92279	1.29337	.92930	.68354	.50674	.37573	.27739	.20331	.14764	10606	.07528	.05275	.03647	.02485	.01668	.01103	.00718	.00460	•00290	.00179	•00100	• 00065	•00038	•00022	.00012	• 00001	• 00003	.00001	00000	
00	4.38489	2.01663	1.34010	.95916	.70449	.52208	.38720	-28602	.20981	.15251	•10968	.07795	.05470	.03786	.02584	.01737	.01151	.00750	•00481	•00304	.00188	•00115	69000	.00040	.00023	• 00013	.00007	,0000	-00002	.00001	00000
સ	. 0		-5		4		9		8	6.	1.0		1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2	2.3	2.4	2.5	2.6	2.7	2.8	2.9	3.0

M= .007

Table 14. 349

	9	600	00611.2	1.58924	• 99016	.72611	.53788	.39898	.29489	.21649	.15752	.11342	.08070	.05670	.03931	.02686	.01809	.01200	.00783	.00503	.00318	.00198	.00121	•00072	.00043	.00024	.00014	.00007	*0000*	*00005	10000	00000	
	ě	900	000000	1.44139	1.02248	.74850	.55417	.41112	.30402	.22337	.16269	.11726	.08354	.05878	.04080	.02792	.01883	.01251	•00818	.00527	.00333	•00208	.00127	• 00016	• 00045	• 00056	• 00015	• 00008	* 0000	*0000	10000	00000	
	70	7 3,477	7 2001	1.064-1	1.05618	.77166	. 57097	.42361	,31341	.23045	.16800	.12123	.08647	.06091	.04234	•02905	09610*	.01304	.00854	.00551	• 00349	.00218	.00133	.00080	.00047	.00027	•00015	•0000	• 0000	*0000	.00001	00000	
	4	2 51704	******	1.55557	1.09132	.79563	.58829	.43647	.32308	.23773	.17347	.12531	.08949	.06312	•04394	•03016	•02040	.01359	*00892	• 00576	-00366	•00228	.00140	.00085	• 000050	• 00059	• 1000	60000°	• 00005	-00005	.0000	• 00000	
	3	10000	70707	1+810-1	1.12804	.82045	.60615	.44971	•33302	.24522	.17910	.12952	.09261	.06540	•04559	.03133	.02123	.01417	16600.	• 00602	.00383	.00240	.00147	• 00089	.00053	•00031	.00017	.00010	.00005	*0000	.00001	00000	
	ð	0 0 0	04116.7	1.06276	1.16644	.84617	.62458	.46334	.34324	.25292	.18489	.13385	.09582	• 06776	.04729	.03255	•0220	.01476	.00971	.00629	• 00401	•00251	.00155	•0000•	• 00056	•00032	.00018	.00010	.00005	.00003	.00001	00000	
	6	2 10310	0.100.0	12861-1	1.20667	.87284	.64360	.47738	.35376	.26085	.19086	.13831	.09913	•01010	•04905	.03381	.02297	.01538	•01010	• 00658	.00420	•00264	.00163	66000.	• 00029	•00034	•1000•	.0001	90000	•00003	.00001	• 00000	
	60	205	0.04040	1.63673	1.24887	.90051	.66323	.49184	.36459	.26900	.19699	.14290	.10254	.07270	.05087	.03512	.02390	.01602	.01058	.00687	•00439	•00276	.00171	•00104	• 00062	• 00036	.00021	11000	• 00000	.00003	.00001	00000	
	5	10.	10110.4	12226-1	٠	.92925	.68351	. 50673	.37573	.27739	.20331	.14764	• 10606	.07528	.05275	.03647	•02485	•01668	.01103	.00718	.00460	• 00290	.00179	00100	• 00005	•00038	.00022	• 00012	.0000	.00003	10000	00000	
•008	S	24/190 1/	4.67147	24610.2	1.33994	.95910	.70446	.52207	.38719	-28602	.20981	.15251	.10968	.07795	.05470	.03786	.02584	.01737	.01151	.00750	.00481	•00304	.00188	.00115	69000*	.00040	.00023	•00013	-00007	. 0000	• 00005	.00001	00000
#	ĸ	c	•	•	.2	۴.	4.	ı,	9.	٠,	æ	6.	1.0	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2	2.3	5.4	2.5	2.6	2.7	2.8	5.9	3.0

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Table	14.	$e^{-u^2 \frac{m^2}{u^2}}$	du
2.11860 1.38904 .99008	. 57500 . 39897 . 29489 . 15752 . 11341 . 08070	. 03930 . 02686 . 01809 . 01200 . 00783	.00198 .00198 .000124 .000024 .000014 .000007
2.23422 1.44116 1.02240	. 55415 . 41111 . 30402 . 22337 . 16268 . 11726	.04080 .02792 .01883 .01251 .00818	.00333 .00238 .00127 .00026 .00026 .00009 .00000
2.36509 1.49645 1.05608		.04234 .02902 .01960 .01304 .00854	.00349 .00218 .00013 .00027 .00004 .00006
.06 2.51565 1.55527 1.09123	. 58826 . 58826 . 32307 . 17347 . 12531 . 08312	.04394 .03016 .02040 .01359 .00892	00000 00000 00000 00000 00000 00000 0000
.05 2.69256 1.61807 1.12793	.00041 .44913 .33301 .24521 .17910 .12951	.04559 .03133 .02123 .01417 .00931	.00240 .00240 .00240 .00089 .000031 .000017 .00002
.04 2.90634 1.68536 1.16632		.04729 .03255 .02209 .01476 .00971	
.03 3.17469 1.75781 1.20654		.03381 .03381 .02297 .01538	
.02 3.52759 1.83620 1.24873		.05087 .03512 .02390 .01602 .01058	.00024 .00024 .000171 .000171 .000036 .00001 .00001
.01 3.98121 1.92155 1.29307	.68348 .50634 .50634 .27338 .20331 .16463	.05275 .03647 .02485 .01668 .01103	.00065 .000179 .000179 .000038 .000012 .000013
.00 4.13373 2.01512 1.33976	.70443 .52205 .38718 .28601 .20980 .15251 .10968	.05470 .03786 .02584 .01737 .01151	.00481 .00304 .00188 .00069 .00069 .00007 .00007
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Table 14. 351

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Table 14.
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80.	2.00355	1.39508	1.00548	.74061	. 55006	.40883	.30269	.22258	.16221	.11697	.08337	.05867	.04073	.02788	.01881	.01250	.00817	.00526	.00333	.00207	.00127	• 00076	.00045	•00056	• 00015	.00008	*0000*	-00002	.00001	00000	
203	2.07332	1.44436	1.03763	.76319	•56659	.42119	.31201	.22962	.16750	.12092	•08629	.06080	.04227	.02898	.01958	.01303	•00853	.00550	.00349	.00218	.00133	.00080	.00047	.00027	•00015	•0000	*0000	• 00005	.00001	00000	
90*	2.13843	1.49604	1.07105	.78654	.58363	.43391	.32160	.23685	.17294	.12499	.08930	.06301	.04386	.03011	.02037	.01358	.00891	.00575	.00365	.00228	.00140	.00084	• 000050	• 00059	•00016	60000	.00005	• 00002	.00001	00000	
• 05	2.19325	1.55026	1.10582	.81067	•1109•	.44699	•33146	.24429	.17855	.12918	.09240	.06528	.04551	•03129	.02120	.01415	.00930	.00601	.00383	.00239	.00147	68000	.00053	.00031	. 00017	•00010	.00005	• 00002	.00001	00000•	
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• 02	2.24784	1.72942	1.21911	.88824	.65722	.48860	.36275	.26792	. 19635	.14251	.10230	.07255	.05078	.03506	.02386	.01600	.01056	.00687	.00439	•00276	.00171	.00104	• 00062	•00036	.00021	.00011	• 00000	•00003	.00001	00000	
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.03	1.96736	1.15913	.85264	.63354	.47193	.35066	.25902	.18976	.13765	.09872	*6690*	.04890	.03372	.02292	.01535	.01012	.00657	• 1000	.00263	.00162	00008	• 00029	•00034	61000.	.00011	90000•	.00003	.00001	00000	
• 02	1.96741	1-19636	.87869	.65250	•48606	.36131	.26707	.19584	.14221	10212	.07244	.05072	•03502	.02384	.01599	.01056	• 00686	• 00439	• 00276	.00171	.00104	-00062	•00036	.00021	.00011	90000•	• 00003	.0000	00000•	
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•00	.77752	.76530	.68533	.56671	.44957	.34804	.26470	.19829	.14644	.10663	•07655	.05416	.03776	.02593	.01753	.01166	• 00764	.00492	.00312	.00194	•00119	. 00071	.00042	.00024	*00014	-0000	• 00004	*0000	.00001	00000	
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.07	.77752	.77200	.70627	.59113	•47196	.36689	.27995	.21033	.15577	.11374	.08189	.05811	.04063	.02798	.01898	.01267	.00832	•00538	•00342	•00214	.00131	• 00019	-00047	.00027	• 00015	•0000	+0000•	*0000	10000	00000	
90•	.17752	.77417	.71603	•60333	.48338	.37659	.28783	.21657	.16062	.11745	.08468	.06018	.04214	.02907	.01974	.01320	• 00869	• 00562	.00358	•00224	•00138	•00083	• 00049	• 00059	•00016	•0000•	.00005	• 00005	.00001	00000	
• 05	.77752	.77566	.72523	.61548	.49495	.38648	.29589	.22297	.16559	.12126	.08755	.06231	.04370	•03019	.02054	.01375	90600*	.00588	.00375	.00235	.00145	*0008	.00052	•00030	.00017	60000	• 00005	*0000	.00001	00000	
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• 03	.17752	.77713	.74170	.63953	.51848	.40680	.31255	.23623	.17594	.12920	.09355	.06677	.04697	.03255	.02221	.01492	18600	• 00642	.00411	• 00258	.00160	16000.	.00058	.00034	.00019	.0001	90000•	.00003	10000	00000	
•02	.77752	.77738	.74884	.65133	.53041	.41723	.32115	.24310	.18132	.13334	89960	.06911	.04868	.03378	.02309	.01554	•01059	.00670	.00430	.00271	•00168	•00102	.00061	•00036	.00020	.00011	90000	• 00003	.00001	00000	
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	90•	.31850	.31850	.31813	.31069	.28667	.24917	. 20649	.16498	.12802	*6960	.07183	.05218	.03720	.02604	•01190	•01200	.00803	.00524	.00335	.00211	.00130	• 00019	.00047	.00027	• 00015	60000	,0000	*0000	10000	00000	
	• 05	.31850	.31850	.31828	.31212	.28981	.25329	.21079	.16896	.13146	.09977	.07409	.05392	.03851	.02700	.01860	.01259	.00837	.00547	19600.	.00221	.00137	.00083	• 000050	•00059	• 00016	.00009	.00005	.00002	10000	00000	
	•04	.31850	.31850	.31837	.31337	.29280	.25734	.21509	.17299	.13496	.10267	.07640	.05571	•03986	.02800	.01932	.01310	.00873	.00571	.00367	• 00232	•00144	.00088	• 00052	.00031	1000.	01000.	• 00005	.00003	.0000	00000	
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009•	00	31850	31850	31850	31675	30316	27282	23230	18950	14954	11686	08620	05336	04547	04240	.02246	01534	01029	00678	00439	00279	.00175	-00107	49000	00038	0000	00012	20000	.00003	.00002	.00001	00000
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• 08	.11389	.11389	.11389	.11383	.11286	• 10875	•1001•	•08796	.07387	.05967	.04661	.03534	.02609	.01879	.01323	*00012	• 00615	.00407	.00263	.00167	.00104	*0000	• 00038	.00022	.00013	. 00007	*0000	20000	10000*	• 00000	
.07	.11389	.11389	.11389	.11385	.11306	.10935	.10124	.08930	.07531	•06106	.04784	.03637	*02692	.01944	•01372	.00947	.00640	.00424	.00275	.00175	•00100•	19000	• 00040	.00023	• 00013	.00007	*0000	*0000	.00001	00000	
90•	.11389	.11389	.11389	.11387	.11323	10601.	.10225	*09062	.07675	.06245	•04400	.03743	•02778	.02010	.01422	*8600*	.00667	.00442	.00288	.00184	.00115	.00070	.00042	• 00025	•0001	• 00008	*0000	*0000	10000	00000	
• 05	.11389	.11389	.11389	.11387	.11337	.11043	10321	•09192	.07818	.06386	•05036	.03851	.02865	•02079	.01473	.01022	*00694	.00461	10600	•00192	.00120	* 000.	*000*	• 00026	•00015	•0000	*0000*	• 00002	.00001	00000	
*0 *	.11389	.11389	.11389	.11388	.11349	.11090	.10414	•1660•	.07961	.06527	.05164	.03961	.02954	•02149	.01527	.01061	.00722	.00481	•00314	.00201	•00126	.00078	.00047	.00028	• 00016	60000	\$0000	*0000	.00001	00000	
.03	.11389	.11389	.11389	.11388	.11358	.11133	10502	.09443	.08103	69990*	.05295	.04073	.03046	.02221	.01581	.0110	.00751	.00501	.00328	.00210	.00132	.00082	. 00049	• 00059	.00017	60000°	• 00005	• 00003	10000	00000	
•05	.11389	.11389	.11389	.11389	.11366	.11171	.10586	.09565	.08244	•06812	.05426	.04186	•03139	.02295	.01638	.01143	.00781	*00522	.00343	.00220	•00139	• 00086	*00052	.00031	• 00018	• 00010	• 00005	• 00003	10000	00000	
.01	.11389	.11389	.11389	.11389	.11372	.11205	.10665	•09684	*08384	.06955	•05559	•04302	.03235	.02370	.01696	.01186	•00812	• 00544	.00358	.00230	.00145	06000	•00055	•00032	• 1000	.00011	• 00000	• 00003	0000	00000	
00	.11389	.11389	.11389	.11389	11377	.11236	•10739	66160	.08523	.07099	.05694	.04420	.03332	.02448	.01755	.01230	•00844	.00567	.00373	.00241	.00152	•0000	.00057	•00034	•00050	11000	90000	•00003	10000	.00001	20000
×	0•		~	· ~	4.	5	9.	.,	8	6•	1.0	1.1	1.2	1.3	1.4	1.5	1.6	1.7	1.8	1.9	2.0	2.1	2.2	2,3	2.4	2.5	2.6	2.7	2.8	2.9	2.0

Table 15

Values of:

$$\int_{x}^{\infty} \frac{e^{-u^2} - \frac{m^2}{u^2}}{u^2} du$$

This table was prepared by Simpson's rule integration. It is to be expected that the difficulties met in the preparation of Table 14 would be present here also and that some of the values near zero would have to be replaced. The integral from zero to infinity is:

$$\int_{0}^{\infty} \frac{e^{-u^{2} - \frac{m^{2}}{u^{2}}}}{u^{2}} = \frac{\sqrt{\pi} e^{-2m}}{2m} \qquad \text{If } m > 0 .$$

A comparison of some computed values and values computed by Simpson's rule is shown below:

	$\int_{0}^{\infty} e^{-u^2 - \frac{m^2}{u^2}} du$	
m	$\int_{0}^{\infty} \frac{u^{2}}{u^{2}}$	Values computed by Simpson's rule
0.001	884.45624	1015.64397
0.002	441.34455	425.84105
0.003	293.64183	292.38882
0.004	219.79135	220.65825
0.005	175.48176	175.69769
0.01	86.86785	86.87255
0.02	42.57387	42.57386
0.03	27.82057	27.82057
0.04	20.45226	20.45226
0.05	16.03782	16.03782
0.1	7.25581	7.25581
0.2	2.97028	2.97028
0.3	1.62124	1.62124
0.4	0.99552	0.99552
0.5	0.65205	0.65205

From this comparison it can be concluded that the table can be accepted for the m values from 0.02 to 1.00 inclusive and that corrections will be needed for some tabular values in the range of m between 0.001 and 0.01 inclusive.

To obtain these corrections make use of the expression

$$e^{-u^2} = 1 - u^2 + \frac{u^4}{2!} - \frac{u^6}{3!} + \dots$$

And write the approximation

$$\int_{0}^{x} \frac{e^{-u^{2} - \frac{m^{2}}{u^{2}}} du}{v^{2}} = \int_{0}^{x} \frac{e^{-\frac{m^{2}}{u^{2}}} du}{u^{2}} - \int_{0}^{x} \frac{u^{2} e^{-\frac{m^{2}}{u^{2}}} du}{u^{2}} + \frac{1}{2} \int_{0}^{x} \frac{u^{4} e^{-\frac{m^{2}}{u^{2}}} du}{u^{2}} - \dots$$

For these purposes this approximation will only be used for values of x which are small compared to unity. Let

$$\frac{m}{u} = v$$
 $u = \frac{m}{v}$ $du = -\frac{mdv}{v^2}$

Then, by substitution:

$$\int_{0}^{x} \frac{e^{-\frac{m^{2}}{u^{2}}}}{u^{2}} = \frac{\sqrt{\pi}}{2m} \frac{2}{\sqrt{\pi}} \int_{(\frac{m}{x})}^{\infty} e^{-u^{2}} du .$$

$$\int_{0}^{x} \frac{u^{2} e^{-\frac{m^{2}}{u^{2}}} du}{u^{2}} = m \int_{(\frac{m}{x})}^{\infty} \frac{e^{-v^{2}} dv}{v^{2}}$$

$$\frac{1}{2} \int_{0}^{x} \frac{u^{4} e^{-\frac{m^{2}}{u^{2}}} du}{u^{2}} = \frac{m^{3}}{2} \int_{(\frac{m}{x})}^{\infty} \frac{e^{-v^{2}} dv}{v^{4}}$$

When v is small compared to unity this last integral will always be less than

$$\frac{m^3}{2} \int_{\left(\frac{m}{X}\right)}^{\infty} \frac{dv}{v^4} = -\frac{m^3}{6v^3} \right]_{\left(\frac{m}{X}\right)}^{\infty} = \frac{x^3}{6}$$

Since this integral must be smaller than the two preceding ones it appears that the approximation will be valid to five places of decimals out to x = .03. To obtain a value for the integral from x to infinity, as in the table, these integral evaluations must be subtracted, algebraically, from the value of the integral from 0 to ∞ . The second integral can be evaluated by use of Table 9.

As an example, compute the tabular value for m = 0.001 and x = 0.02 (m/x) = 0.05. Then

$$\int_{0}^{\infty} \frac{e^{-u^{2}} - \frac{m^{2}}{u^{2}}}{u^{2}} = 884.45624$$

$$-\frac{\sqrt{\pi}}{2m} \frac{2}{\sqrt{\pi}} \int_{0.05}^{\infty} e^{-u^{2}} du = -(886.22693) (1 - 0.056371978)$$

$$= -836.26856$$

$$+ m \int_{0.05}^{\infty} \frac{e^{-u^{2}}}{u^{2}} du = +(0.001) (18.27753) = +0.01828$$

$$-\frac{0.02^{3}}{6} = Sum \frac{0.00000}{48.20596}$$

The tabular value is 48.20600

Replacement values have been computed by this procedure until the replacement and tabular values agree. The replacement values have been incorporated into the tables. Since the integration was made by working from

x=3 back toward x=0 an agreement between a replacement value and a tabular value at some x value constitutes a check of all tabular values between that point and x=3.

Table	15.	$\frac{e^{-u^2}}{u^2}\frac{111^4}{u^2}du$	
.09 9.42809 3.67952 1.96181	. 73965 . 48047 . 31687 . 21041 . 13992 . 09283 . 06127	.02608 .01677 .01067 .00670 .00416 .00254 .00153	.00031 .00017 .00010 .00003 .00001 .00001
.08 10.80682 3.96208 2.07536	. 50128 . 50128 . 33020 . 21917 . 14575 . 09678	.02724 .01754 .01117 .00702 .00436 .00267	.00032 .00018 .00000 .000003 .000001
.07 12.58224 4.27902 2.19800 1.29203	. 52296 . 52296 . 34412 . 22831 . 15183 . 16661	.02846 .01834 .01169 .00736 .00458 .00170 .00101	.00034 .00011 .00001 .00003 .000002 .000001
.06 14.95265 4.63679 2.33079	. 94599 . 54573 . 35865 . 23782 . 15815 . 10503	.02972 .01917 .0123 .00771 .00480 .00295 .00179	. 00036 . 00012 . 00006 . 00003 . 00000 . 00000
.05 18.27488 5.04356 2.47495	. 48517 . 56959 . 37383 . 24774 . 16474 . 10943 . 07240	.03103 .02004 .01280 .00504 .00504 .00310 .00112	. 00038 . 00012 . 00007 . 000004 . 000000 . 00000
.04 23.26235 5.50983 2.63191	.90291 .90461 .90461 .38969 .17160 .11402 .07547	. 03240 . 02094 . 01339 . 00847 . 00529 . 00126 . 00118	.00041 .00023 .00013 .00004 .00004 .00000
.03 31.57857 6.04934 2.80334	. 62085 . 62085 . 40626 . 26887 . 17874 . 11879	.03383 .02189 .01401 .00887 .00554 .00264 .00208	.00043 .00025 .00014 .00004 .00002 .00000
.02 48.20596 6.68040 2.99121	1.006/10 1.01657 .64839 .42358 .28012 .18618 .08200	.02287 .02287 .01466 .00581 .00581 .00219	.00008 .00008 .00008 .000004 .0000015
.01 97.90531 7.42799 3.19789	1.06545 1.06545 .67732 .44170 .29186 .19393 .12893	.03686 .02390 .01534 .00973 .00810 .00377 .00230	.00027 .00027 .00015 .00009 .00002 .00001
.00 884,45624 8.32705 3.42618		.03847 .02497 .01604 .01019 .00539 .00396 .00242	.00029 .00016 .00009 .00009 .00001
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	•05	18,10936	5.03769	2.47380	1.42733	.88502	.56952	.37379	.24773	.16473	.10943	.07240	•04759	.03103	•0200	.01280	•00808	•00500	.00310	•00188	.00112	99000*	• 00038	• 00022	•00012	10000	•0000•	• 00005	.00001	00000	00000•
	•0•	22,93968	5.50256	2.63059	1.50190	.92644	.59453	.38965	.25807	.17159	11401	.07547	.04965	.03240	.02094	.01339	.00847	.00529	.00326	.00198	.00118	.00070	.00041	.00023	.00013	.00007	• 00004	• 00002	.00001	00000	00000•
	•03	30.81936	6.04021	2.80182	1.58152	.97017	.62077	.40622	.26885	.17873	.11879	.07867	.05179	.03383	.02189	.01401	.00887	.00554	•00342	•00208	.00125	• 00074	• 00043	• 00025	.00014	• 00008	,0000	• 00005	.00001	00000	00000•
	• 02	45.70721	6.66872	2.98946	1.66668	1.01637	.64831	.42354	.28010	.18617	.12375	•08200	• 05402	.03531	.02287	•01466	•00929	.00581	•00359	.00219	.00131	• 00078	.00045	•00056	.00015	• 00008	*0000	• 00005	10000	00000	00000
	.01	80.45200	7.41274	3.19585	1.75790	1.06524	.67722	.44165	.29183	16261.	•12892	.08546	.05634	•03686	.02390	.01533	.00973	.00610	.00377	.00230	.00138	*0008	• 00048	.00027	.00015	60000	• 00005	• 00002	.0000	.00001	00000
• 008	00•	109.02002	8.30665	3.42381	1.85581	1.11697	.70761	.46060	.30407	.20199	.13431	.08907	.05875	.03847	.02497	.01604	.01019	• 00639	•00396	•00242	.00146	•00086	.00051	•00059	•00016	60000	• 00005	•00003	.00001	.00001	00000
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60.	9.39241	3.67597	1.96091	1.17173	.73951	.48041	.31684	.21039	13991	.09282	.06127	.04015	.02608	.01677	.01067	.00670	J	~			•					.00003	.00001	.00001	00000	00000	
90.	10.75584	3.95787	2.07435	1.22988	.77311	.50115	•33016	.21915	-14574	.09673	•06388	.04190	.02724	.01754	.01117	•00702	.00436	.00267	.00161	96000*	•00026	•00032	.00018	.00010	90000	•00003	.00001	.00001	00000	00000	
.07	12.50593	4.27398	2.19686	1.29164	.80847	.52288	.34408	.22829	.15182	.10079	.06661	.04372	.02845	.01834	.01169	.00736	.00458	.00281	.00170	.00101	• 00029	•00034	•00019	.00011	90000*	.00003	• 00005	.00001	00000	00000	
90.	14.83127	4.63070	2.32950	1.35731	.84572	.54564	.35861	.23780	.15814	.10502	.06945	.04562	.02972	.01917	.01223	.00771	.00480	• 00295	.00179	.00107	.00063	•00036	.00021	.00012	90000	•0000	• 00002	•00001	00000	00000	
• 05	18.06512	5.03611	2.47348	1.42723	.88498	.56950	.37378	.24772	.16473	.10943	.07240	•04759	.03103	•0200•	.01280	.00808	.00504	.00310	.00188	•00112	• 00066	.00038	.00022	.00012	.00007	•0000•	.00002	.00001	00000	00000	
•0•	22.85390	5.50061	2.63023	1.50179	.92639	.59451	.38964	.25806	.17158	.11401	.07547	.04965	.03240	•0209	.01339	.00847	.00529	.00326	.00198	.00118	.00070	.00041	.00023	.00013	.00007	*0000*	• 00002	.00001	00000	00000	
•03	30.61987	6.03775	2.80141	1.58140	.97012	.62075	.40621	.26885	.17872	.11878	.07867	.05179	.03383	.02189	.01401	.00887	.00554	.00342	.00208	.00125	. 00074	.00043	.00025	.00014	• 00008	• 0000 •	• 00002	.00001	00000	00000	
.02	45.07190	6.66558	2.98899	1.66654	1.01632	.64828	.42353	.28009	.18616	.12375	.08200	• 05402	.03531	.02287	.01466	.00929	.00581	•00359	•00219	.00131	.00078	.00045	• 00056	.00015	• 00008	•0000•	*0000	.00001	00000	00000	
.01	76.71589	7.40863	3.19530	1.75775	1.06518	.67720	.44164	.29183	.19391	.12892	.08546	.05634	.03686	.02390	.01533	.00973	.00610	.00377	.00230	.00138	.00082	.00048	.00027	.00015	60000	• 00005	• 00002	.00001	.00001	00000	
00•	96.71306	8,30116	3.42317	1.85564	1.11691	.70758	.46058	.30406	.20198	.13430	.08907	.05875	.03847	.02497	.01604	•01010	•00639	•00396	.00242	•00146	•00086	.00051	.00029	• 00016	•0000•	•00005	• 00003	.00001	.00001	00000	00000
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•00	9.25227	3.66186	1.95733	1.17044	.73896	.48014	.31670	.21032	.13987	.09280	.06126	.04014	• 02608	.01677	•01066	.00670	.00416	• 00254	.00153	• 00001	• 00053	.00031	.00017	•00010	• 00005	• 00003	.00001	.00001	00000	00000	
• 08	10.55635	3.94114	2.07032	1.22846	.77251	.50087	*33005	.21908	.14570	0.09670	.06387	.04189	.02724	.01754	.01116	• 00702	.00436	.00267	.00161	96000*	• 00026	•00032	• 00018	•00010	90000	.00003	10000	.00001	00000	00000	
.07	12.20899	4.25397	2.19231	1.29008	.80782	.52258	.34393	.22820	.15177	.10077	.06660	.04371	.02845	.01833	•0110•	.00736	.00458	.00281	.00170	.00101	• 000 59	.00034	61000*	.00011	90000*	•0000	• 00002	.00001	00000	00000	
90•	14.36302	4.60652	2.32435	1.35560	.84501	.54532	.35845	.23772	.15809	.10500	.06943	.04561	.02971	.01917	.01223	.00771	.00480	• 00295	•00179	.00107	.00063	• 00036	.00021	.00012	90000	.00003	*0000	.00001	00000	00000	
• 05	17.26750	2.00657	2.46763	1.42534	.88421	.56915	.37361	.24763	.16468	.10940	.07238	.04758	.03103	.02004	.01280	.00808	.00504	.00310	.00188	•00112	99000*	• 00038	• 00022	.00012	.0000	•0000•	• 00005	.00001	00000	00000	
•0•	21.34072	5.46405	2.62355	1.49970	.92556	.59413	.38945	.25797	.17153	.11398	.07545	•04964	.03239	.02094	.01339	.00847	.00529	•00326	• 00198	.00118	.00070	.00041	.00023	.00013	20000	*0000*	-00002	.00001	00000	00000	
.03	27.25893	5.99185	2.79374	1.57908	.96921	.62034	.40601	.26875	.17867	.11875	.07865	.05178	.03382	.02188	.01401	.00887	.00554	.00342	•00208	.00125	* 2000.	.00043	.00025	• 1000	•0000	*0000*	.00002	.00001	00000	00000	
• 02	35.60550	6.60694	2.98014	1.66397	1.01533	.64784	.42331	.27998	.18610	.12372	.08198	.05401	.03531	.02287	•01466	•00929	.00581	.00359	.00219	.00131	• 00078	.00045	• 00056	.00015	*0000	,0000	.00002	.00001	00000	00000	
.01	42.36671	7.33221	3.18503	1.75489	1.06411	.67672	14145	.29171	.19385	.12889	.08544	.05633	.03685	.02389	.01533	.00973	01900	.00377	.00230	•00138	.00082	.00048	.00027	.00015	•0000•	• 00005	• 00005	.00001	.00001	00000	
00•	42.57386	8.19915	3.41117	1.85244	1.11573	.70707	.46034	30394	.20192	.13427	.08905	•05874	.03847	.02496	.01604	•01010	•00639	•00396	•00242	.00146	.00086	.00051	•00059	•00016	•0000	• 00005	• 00003	.00001	.00001	00000	00000•
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	.07	10.51460	4.12554	2.16270	1.27991	.80357	• 52059	.34292	.22768	.15149	10001	.06651	•04366	.02842	•01832	.01168	•00736	*00457	.00281	.00170	.00100	•0000	•00034	• 00019	.00011	90000	• 00003	• 00005	.00001	00000	00000	
	•00	11.81768	4.45187	2.29085	1.34440	.84040	.54318	.35738	•23716	.15779	.10483	•06934	.04556	.02968	•01915	.01222	.00771	.00480	• 00295	.00179	.00107	• 0000	•00036	.00021	•00012	90000	• 00003	-00005	• 0000	00000	00000	
	• 05	13.25422	4.81836	2.42957	1.41299	.87920	.56684	.37247	.24703	.16435	.10922	.07228	.04753	•03099	• 02002	.01279	.00808	•00204	.00310	.00188	•00112	99000•	•00038	•00052	.00012	.0000	•0000	*0000	.00001	00000	00000	
	•0•	14.67281	5.23225	2.58012	1.48605	.92012	.59165	.38823	.25733	.17119	.11380	.07535	.04958	.03236	-02092	•01338	• 00846	.00528	.00325	.00198	•00118	.00070	.00041	.00023	• 00013	.00007	+0000•	• 00002	.00001	00000	00000	
	.03	15.71153	5.70243	2.74395	1.56396	.96329	.61767	.40470	.26807	.17831	.11855	.07854	.05172	.03378	•02186	.01400	.00886	.00554	.00342	• 00208	•00125	. 00074	.00043	.00024	.00014	.00008	• 0000 •	.00002	.00001	00000	00000	
	•02	16.03061	6.23991	2.92277	1.64717	1.00887	96449.	.42191	•27926	.18572	.12351	.08186	•05394	.03527	.02285	.01465	•00928	.00581	• 00329	•00219	.00131	.00078	.00045	•00056	• 00015	*0000	*0000	*0000	.00001	00000	00000	
	.01	16.03782	6.85828	3,11855	1.73619	1.05705	.67362	.43991	.29094	.19344	.12866	.08532	.05626	.03681	.02387	.01532	.00972	60900*	.00377	.00230	.00138	• 00082	.00048	.00027	•00015	60000	• 00005	• 00005	.00001	.00001	00000	
•050	00.	16.03782	7.57419	3.33365	1.83156	1.10802	.70371	.45873	.30312	.20148	.13403	.08891	.05867	.03842	.02494	•01602	.01018	•00639	96600.	.00242	.00146	• 00086	•000050	•00059	• 00016	60000	•0000•	•00003	.00001	.00001	00000	00000
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60*	8.01379	3.52441	1.15760	.73344	.47751	.31536	.20961	.13948	.09258	.06113	.04008	.02604	.01675	.01065	69900*	.00415	.00254	.00153	.00091	.00053	.00031	.00017	.00010	.00005	.00003	.00001	.00001	00000	00000	
.08	8.84868	3.7.878	1.21439	.76653	.49805	.32859	.21832	.14529	.09648	.06374	.04182	.02720	.01751	.01115	.00702	.00436	.00267	.00161	96000.	• 00056	.00032	• 00018	.00010	90000	.00003	.00001	.00001	00000	00000	
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• 00	10.78217	4.5/589	1.33858	.83799	.54206	.35682	.23686	.15763	-10474	• 06959	.04553	.02967	.01914	.01222	.00770	.00480	.00295	.00179	.00107	.00063	• 00036	.00021	-00012	•0000	.00003	• 00005	.00001	00000	00000	
-05	11.77781	4.12395	1.40658	.87659	.56564	.37187	.24672	.16419	.10913	.07223	.04750	.03098	.02001	.01278	.00807	•00203	•00310	•00188	.00112	99000*	.00038	• 00022	.00012	-0000	*0000	*0000	.00001	00000	00000	
• 04	12.60018	3.11008	1-47896	.91728	.59036	.38759	.25700	10171.	.11370	.07529	.04955	.03234	.02091	.01338	.00846	.00528	.00325	.00198	.00118	.00070	.00041	.00023	.00013	.00007	•0000•	-00002	.00001	00000	00000	
•03	13.03117	7.03424	1.55611	.96020	.61627	.40401	.26771	.17811	.11845	.07848	.05168	.03377	.02185	.01399	• 00886	.00554	.00342	•00208	.00125	• 00074	.00043	.00024	•00014	.00008	*0000	• 00005	.00001	00000	00000	
• 02	13.09988	0.0000	1.63846	1.00551	•64346	.42118	.27888	.18552	.12340	.08180	.05391	.03525	.02284	.01464	•00928	.00581	•00329	.00219	.00131	.00078	• 00045	• 00056	• 00015	*0000	• 0000 •	• 00005	.00001	00000	00000	
.01	13.10021	2 00660	1.72649	1.05338	.67199	.43912	. 29053	.19322	.12855	.08525	.05622	.03679	.02386	.01531	.00972	60900*	.00377	.00230	.00138	•00082	*000	.00027	• 00015	60000	• 00005	• 00002	.00001	.00001	00000	
00	13.10021	4 20604	1.82075	1.10401	.70196	.45788	.30269	.20125	.13390	.08884	.05863	.03840	.02493	•01602	.01017	• 00638	•00396	•00242	.00146	•00086	000020	• 00059	.00016	60000	.00005	•00003	.00001	.00001	00000	00000
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	.07	9.02116	3.98557	2.12951	1.26841	.79874	.51832	.34178	.22708	15116	.10043	.06641	.04361	.02839	.01830	.01167	• 00735	.00457	.00280	.00170	.00101	• 00029	•00034	•1000	.00011	90000*	• 00003	• 00002	.00001	00000	00000•
	90.	9.75663	4.28431	2.25335	1.33175	.83516	.54074	.35616	.23652	.15744	.10464	.06923	.04550	• 02965	.01913	.01221	.00770	.00480	• 00295	• 00179	-00107	• 0000	•00036	.00021	.00012	90000	.00003	.00002	.00001	00000	00000•
	• 05	10.40344	4.61589	2,38704	1.39905	.87352	.56422	.37116	.24635	•16399	.10902	•07217	•04746	• 03096	.02000	.01278	.00807	•00203	.00310	.00188	.00112	99000	.00038	• 00022	.00012	.00007	,0000	*0000	.00001	00000	00000•
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	.03	10.99417	5.39703	2.68854	1.54690	.95656	.61463	.40321	.26729	.17789	.11833	.07841	.05164	.03374	.02184	.01399	• 00885	.00554	.00342	•00208	.00125	. 00074	.00043	•00054	.00014	.00008	*0000	.00002	.00001	00000	00000
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	• 02	8.22488	5.40283	2.77711	1.60345	.99189	.63736	.41820	.27734	.18469	.12294	.08155	.05376	.03517	.02279	.01461	• 00926	.00580	.00358	.00218	.00131	. 00078	• 00045	•00056	• 00015	*0000	• 0000	-00005	.00001	00000	00000	
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~ About this Book ~

Transient Ground Water Hydraulics has been cited or referenced in numerous papers, journals, and publications. It addresses developments, formulas, and methods that engineers have found useful when making quantitative evaluations of ground water flow. WRP is pleased to include this popular book in its Classic Resource Edition.



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